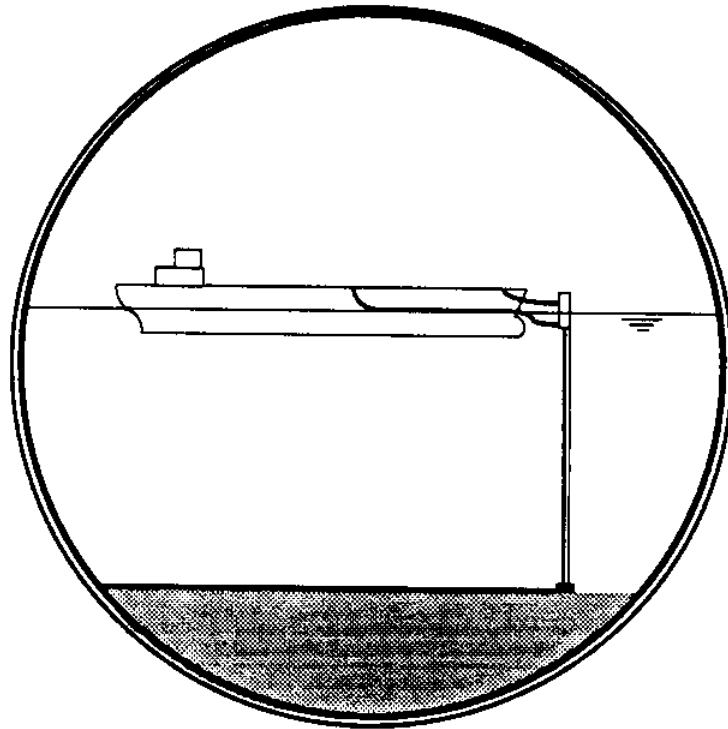


Offshore Single Point Mooring Systems for Import of Hazardous Liquid Cargoes

SEA GRANT PROJECT R/OE-26



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ABSTRACT

The goal of this project was the determination of feasibility of single point mooring systems (SPMS) for use as deepwater ports for the import of hazardous liquid cargoes offshore southern California. The use of deepwater ports is advocated because it has been determined by the U.S. Coast Guard that they represent the least risky form of crude oil import, lessening the likelihood of occurrence and environmental impact severity of accidents. Two configurations of SPMS were examined as deepwater ports in this project : catenary anchor leg mooring (CALM) and single anchor leg mooring (SALM). Two sites for these systems were chosen offshore southern California by the California State Lands commission : El Segundo and Morro Bay. The project examined the environmental conditions at both sites, developed analytical models with which to evaluate the suitability of SPMS to these environmental conditions, determined the reliability of the systems by use of state-of-the-art reliability methods, and evaluated the feasibility of the systems by comparing reliability to system costs.

The results of this project indicate that SPMS for offshore southern California conditions are feasible and do not require major technological developments to allow such systems to be designed, constructed, and operated. Use of these systems should lower the number of accidents due to hazardous liquid cargo import, as well as reduce the impact of those accidents which do occur.

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List of Symbols

A	Projected area
A_S	Side surface area of pile
B	Stress range model error parameter
B_B	Beam
B_K	Bias on K
C_{USS}	Undrained shear strength
C_B	Coefficient of variation of B
C_D	Drag coefficient
C_{Df}	Drift coefficient
C_{Drift}	Average drift coefficient
C_K	Coefficient of variation of K
C_S	Wind shape coefficient
C_{WS}	Wetted surface area coefficient
C_Δ	Coefficient of variation of Δ
D_{Buoy}	Buoy diameter
D_p	Pile diameter
f_{CO}	Friction coefficient, chain and ocean bottom
f_{SC}	Unit skin friction capacity
f_0	Average stress frequency
$F_{Bow, drift}$	Mean wave drift force, bow-on
$F_{Current, buoy}$	Current force on buoy
$F_{Current, Ship}$	Current force on ship, bow-on
$F_{Mean drift}$	Mean wave drift force
F_{Wind}	Steady wind force
FS	Factor of safety
FSF	Fatigue safety life factor

LIST OF SYMBOLS

H_D	Design wave height
H_{max}	Maximum wave height
H_S	Significant wave height
K	Log life intercept of S-N curve
L	Length
L_C	Length of chain in contact with ocean bottom
m	Negative slope of S-N curve
N	Number of elements
N_T	Stress cycles
P_C	Chain holding power
P_{conn}	Probability tanker is present at facility
P_f	Annual probability of failure
$P_{sea\ state}$	Probability of sea state being maximum annual sea state
Q_P	Ultimate pull-out capacity
R_{50}	Mean capacity
S	Wetted surface area
S_{fD}	Fatigue design stress range
S_m	Largest expected stress range
S_{50}	Mean load
t	Wind gust duration
T_{DP}	Design time period
T_{fL}	Mean fatigue life
T_S	Significant wave period
T_{SF}	Design time period, with safety factor
T_{SL}	Service life
T_{WT}	Minimum pile wall thickness
V_C	Current velocity
V_R	Coefficient of variation of resistance
V_S	Coefficient of variation of load
V_t	Wind gust velocity, duration t
$V_{t,z}$	Wind gust velocity, modified by elevation and duration
V_X	Coefficient of variation of X
V_Z	Wind velocity at centroid elevation

LIST OF SYMBOLS

x

$V_{1\ min}$	Wind gust velocity, 1 minute duration
V_{10}	Wind velocity at 10 meter elevation
V_{II}	Coefficient of variation of type II variances
WC	Submerged unit weight of chain
WP	Pile weight
X	Mean value of X
X_{50}	2-year force
X_{99}	100-year force
Z	Centroid elevation
α	Wave height exponent
α_P	Dimensionless pile factor
β	Safety index
β_E	Element safety index
β_S	System safety index
Δ	Accumulated fatigue damage
ΔVol	Displaced volume
ϵ	Stress range parameter
Φ	Standard cumulative normal distribution
Γ	Gamma function
$\lambda (m)$	Rainflow correction factor
ρ_{Air}	Air density
ρ_E	Correlation of elements
ρ_{FM}	Failure mode correlation
ρ_{RS}	Correlation coefficient, load to capacity
ρ_{SW}	Salt water density
$\sigma_{ln K}$	Standard deviation log K
$\sigma_{ln R}$	Standard deviation log R
$\sigma_{ln S}$	Standard deviation log S
$\sigma_{ln T}$	Standard deviation log T
$\sigma_{ln X}$	Standard deviation log X
σ_X	Standard deviation X
Ω	Stress range parameter

Chapter 1

Introduction

1.1 Project Overview

The purpose of this project (Sea Grant project R/OE-26) was to perform an evaluation of the reliability and feasibility of deepwater ports, specifically those consisting of a single point mooring system (SPMS) and support equipment for tanker discharge, for offshore southern California. These deepwater ports could serve as discharge ports for tankers delivering crude oil from Valdez, Alaska, or other supply points, to southern California. Two locations along the California coast, El Segundo and Morro Bay, were studied as potential locations for these facilities. These two locations were specified by the California State Lands Commission. The water depth for both facilities was proposed as one thousand feet.

Direction in this project was provided by Professor Robert Bea (principal investigator) and Professor William Webster (co-principal investigator). The research was performed by Mr. Aaron Salancy and Mr. Wei Ma. Mr. Salancy was responsible for the work presented in this report, consisting of the systems engineering of the facilities. Mr. Ma was responsible for the development of the analytical models used in this project [Ma, 1994].

1.2 Project Background

This project investigated the feasibility of deepwater ports at the two locations proposed by the California State Lands Commission. This investigation required examining the existing types of SPMS, evaluating how they would function in the offshore southern California environment, determining what configurations would adequately withstand the environmental conditions while performing satisfactorily, and establishing the financial and technical feasibility of the resulting configurations. The determination of feasibility was

based on an assessment of the reliability characteristics of each proposed system and the costs to build and operate the system.

Two facility systems were chosen for detailed examination. Each facility was to be capable of servicing tankers up to roughly very large crude carrier (VLCC) size (loosely defined as 200,000 to 275,000 DWT), with the tankers requiring no major modification to use the facility. Each facility would also meet all major relevant requirements and guidelines for an offshore installation of this type. It was desirable that the facilities should not require major leaps in technology from that currently existing in any component, or in installation, maintenance, or regular operation. This was considered necessary to insure that reasonable reliability, feasibility, and cost estimates could be obtained. Above all else, the facility should be capable of rapid, safe disconnection in deteriorating sea states and be capable of surviving intact the 100-year storm and seismic conditions with a sufficiently high probability of success. Only once these requirements were met by a system would the financial feasibility analysis be conducted for that system.

The scope of this project includes : the SPMS, the tankers which are expected to use the facility, all equipment necessary for connection and discharge of the tankers, and the piping system to transfer oil from the facility to the shore. The pipeline to shore itself is not a main focus of this project, however, and the shoreside facilities have not been examined. These aspects of the systems should be given attention in the future, as they will heavily impact system feasibility by their effect on facility cost.

1.3 Deepwater Ports

Deepwater ports, defined as ports several miles offshore which can service VLCC's and ULCC's (ultra-large crude carriers, loosely defined as 275,000 DWT and up), have several obvious advantages over other methods of crude oil delivery. They lessen the impact of accidents due to their distance from shore and reduce the probability of accidents by keeping tankers from entering congested ports. However, until recently, no quantifiable evidence proving the worth of deepwater ports existed. This changed when the U.S. Coast Guard declared deepwater ports to be the least environmentally risky form of crude oil import in their report "USCG Deepwater Ports Study" [U.S. Department of Transportation, 1993].

In the report, deepwater ports were compared with three other methods of crude oil import : direct vessel delivery (tanker enters port and discharges at a terminal), offshore lightering (tanker off-loads to a smaller tanker or barge offshore, and this second vessel then transports oil to the port terminal) and offshore mooring delivery (tankers less than VLCC size discharge through pipeline to shore at a shallow water facility). Offshore moorings are defined as being within 1 to 2 miles offshore in the study. Although the Louisiana Offshore Oil Port (LOOP) facility was the only deepwater port examined, approximately 14% of

all foreign-source crude oil imported to the U.S. has gone through LOOP in recent years, making it significant [U.S. Department of Transportation, 1993].

The determination of environmental risk in this report was based upon historical frequency of spills, average spill size, and an environmental impact coefficient for each different environmental area entered or transited by tankers for each method of delivery. This produced an average environmental impact for each method of delivery. Deepwater ports were found to pose the lowest environmental risk primarily because : the transfers of crude oil occur offshore, where environmental impact is lower; the crude oil is delivered into port by means of a pipeline, which is a very safe means of transportation; and no ships are exposed to through-port transit dangers. Deepwater ports also allow for the pre-positioning of spill response equipment at the port site. However, for "worst case" spills, the study found all methods of import to pose roughly equal environmental risk, due to the disastrous consequences of complete loss of a tanker's cargo [U.S. Department of Transportation, 1993].

Of course, deepwater ports have their drawbacks, and these should be mentioned. They require enormous capital expenditure as well as efforts to obtain state and federal permits for construction and operation.

1.4 Single Point Mooring Systems

The first parameter in this project was the use of a SPMS as a deepwater port. A SPMS is a mooring which allows a ship to weather vane around the mooring, thus minimizing the environmental loads on the system by allowing the moored ship to head into the prevailing weather. In this case, the SPMS also provides the interface between the tanker and the pipeline for the discharge of crude oil.

1.4.1 SPMS Around the World

SPMS have been used successfully in many applications around the world in many different conditions. The challenge posed in this project for the use of SPMS is the specified water depth of one thousand feet and the offshore California oceanographic and seismic conditions. This is not an unprecedented depth for the use of SPMS, as the Marlim Field catenary anchor leg mooring (CALM) off the coast of Brazil is located in approximately 1312 feet of water [Hwang and Bensimon, 1990]. However, the environmental conditions offshore California, including seismic activity, are more severe than those encountered by most SPMS. Even so, the design of a SPMS for use in one thousand feet of water off the California coast should require no major breakthrough developments.

Single point mooring systems are in operation in many locations around the globe. Appendix 1 gives a representative list of approximately 400 SPMS, their locations and their installation date. There are currently SPMS in California, but these existing systems are in significantly shallower water.

SPMS have been designed in many different configurations, some of which are discussed in Chapter 3, System Configurations. Two specific systems which best meet the needs of this project are chosen for analysis in Chapter 3.

1.4.2 LOOP Facility

In the course of developing background for this project, we visited the Louisiana Offshore Oil Port (LOOP) facility in Louisiana. This facility was considered to be highly relevant to the project, as it is currently the only deepwater facility in the U.S. and makes use of three SPMS. The LOOP facility is owned and operated by LOOP Inc., and is governed by the laws of the United States in the same manner as if the port were an area of exclusive federal jurisdiction located within a state. The United States Coast Guard's Marine Safety Office has governmental authority over LOOP [LOOP Operations Manual, 1992]. We conducted several interviews at LOOP, and the findings were very helpful in many aspects of this project.

The LOOP facility encompasses (offshore) three SPMS of the single anchor leg mooring (SALM) type and a platform complex consisting of a pumping platform and a control platform. The LOOP offshore pumping facility is located at 28 degrees, 53.2 minutes North latitude, 90 degrees, 1.5 minutes West longitude. The SALM's are located in a radial pattern from the platform at a distance of 8150 feet, and were built to accommodate vessels of up to 700,000 DWT. LOOP began operations in 1986 and over recent years has received approximately 14% of all foreign-source US import crude oil. It has been visited by tankers ranging in size from 80,000 DWT to 556,000 DWT [US Department of Transportation, 1993].

The major physical difference between LOOP and the facilities proposed in this project is the water depth. Although LOOP is considered a deepwater port -- because it is capable of servicing VLCC's and ULCC's -- it is located in approximately 115 feet of water. The environmental conditions are also milder in the Gulf of Mexico than along the southern California coast. LOOP is located approximately 18 nautical miles offshore Louisiana, a greater distance than the facilities in this project. The LOOP facility was designed to support a much greater amount of tanker traffic than the facility in this project.

Bearing these differences in mind, there is still much to be learned from LOOP. All components at or near the water surface will be very similar to those in this project, as will operational procedure. Installation and maintenance will have the largest differences due to factors related to water depth.

1.5 Report Structure

This report examines environmental conditions, SPMS types, analytical models, reliability, and feasibility of SPMS. Chapter 2 presents background on the sites chosen by the California State Lands Commission and the environmental conditions encountered at these sites. The main environmental components examined are wind, wave, current, and seismic activity. The ocean floor and soil conditions at each site are also examined.

In Chapter 3 the various types of SPMS configurations are examined, and the configurations deemed most suitable for this project, CALM and SALM, are detailed. The supporting components of these systems are also examined. These include the tankers visiting the facility, the pipeline from the facility to the shore, and all tending vessels required for facility operation.

Chapter 4 discusses the analytical models used to determine the effects of environmental components on the facilities. Environmental loadings and environmental-induced motions are modeled for their effect on the horizontal offset of the SPM buoy, from which line tensions in the SPMS anchor legs can be determined. The environmental loadings consist of steady forces (wind, current, and mean wave drift force), oscillating motions (first order wave motions and second order wave motions) and seismic loadings. The restoring force of both the CALM and SALM systems are modeled.

Reliability is examined in Chapter 5. The reliability of each facility is measured by its annual probability of failure. The probability of failure is divided into four relatively independent components : failure due to storm loadings, failure due to seismic loadings, failure due to fatigue, and failure attributable to human and organizational error (HOE). Each of these components is examined. The annual probability of failure of each facility for each component is examined and calculated.

Chapter 6 discusses the feasibility of the proposed facilities. This is done by comparing the cost of each facility with its reliability. The financial analysis includes the cost of the system components, system development and engineering, construction and transportation, installation, operation and maintenance, and permitting. The feasibility of the systems is the result of this analysis.

Chapter 7 presents a summary of this work, as well as recommendations for future work on this topic.

Chapter 2

Environmental Conditions

2.1 Introduction

The goal of this chapter is the description of the environmental conditions which have an impact on the design of the SPMS systems at the two chosen sites. A SPMS is always subject to forces from the surrounding ocean and atmosphere, in the form of wind, wave and current. In a location such as southern California, seismic events must be considered as well. The topography of the ocean floor needs to be considered, as well as the nature of the soil with regards to anchor and anchor pile holding power. Therefore, the conditions examined in this chapter include wind, waves, current, seismic activity, ocean floor topography, and soil type.

2.2 Project Sites

Two sites along the southern California coast were identified by California Sea Grant as prospective SPMS facility sites : El Segundo and Morro Bay. El Segundo is located at 33 degrees, 55 minutes North latitude and 118 degrees, 25 minutes West longitude, while Morro Bay is located at 35 degrees, 22 minutes North latitude and 120 degrees, 53 minutes West longitude (Figure 2.1). The specified water depth of 1000 feet gives a distance offshore ranging from 6 to 12 miles at both sites, for a variety of specific locations (Appendix 2).

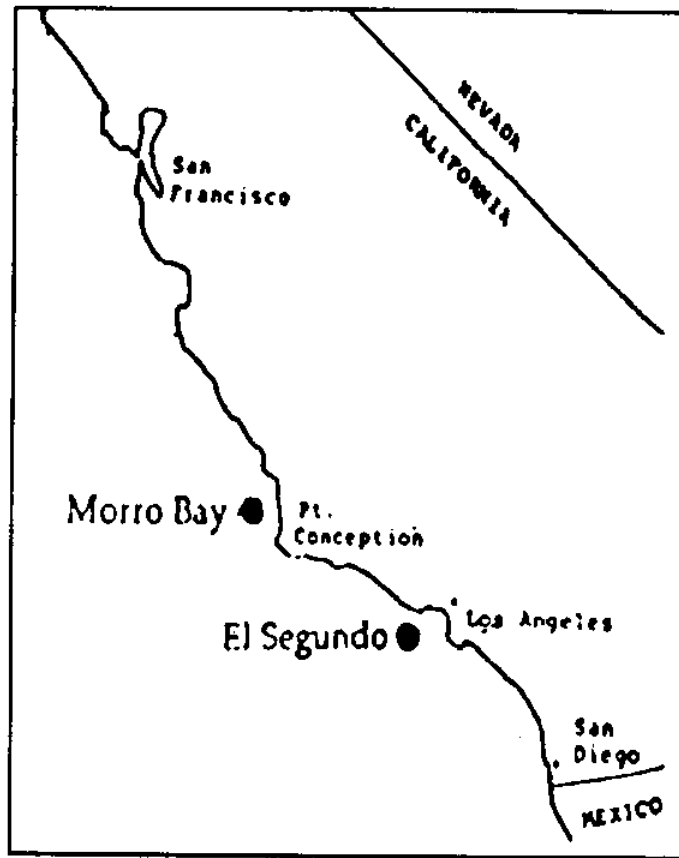


Figure 2.1 : Site Locations

2.3 Environmental Conditions

The weather along the southern California coast is primarily a product of extra-tropical storms originating in the Northeast Pacific during winter [Stevens, 1977]. Other weather phenomena such as tropical storms, thunderstorms, tornadoes and waterspouts are rare at best.

The main environmental components of wind, waves, current and seismic activity are discussed in the following sections and summarized in Table 2.1. The environmental conditions are presented in a probability framework, which will allow for rapid integration with the methods used to determine probability of failure in Chapter 5, "Reliability". This method is based on the concept of "return period". The return period is the mean elapsed time expected between occurrences of an event. For example, if a thirty-foot wave is expressed as the 50-year return period wave, this means that a wave of thirty foot height is expected to occur once every fifty years. These values are based on past statistical data and extrapolation.

It is customary to assume that all of the environmental components considered here follow a log normal distribution, and this has been done. This type of distribution will be discussed more fully in Chapter 5.

Return Period (years)	Wind Velocity (@ 10m) (knots)	Surface Current Velocity (knots)	Expected Maximum Wave Height (feet)	Peak Vertical Ground Acceleration (gravities)
2	39	1.2	29	0.01
10	55	1.7	37	0.05
100	72	2.2	46	0.18
1000	88	2.7	54	0.40

Table 2.1 : Environmental Conditions by Return Period

Another environmental component, visibility, should be noted. Heavy fog is possible in these areas, and visibility is typically reduced to under one mile for approximately 2.5% of the year, varying by location [Stevens, 1977]. This may have an adverse effect on operations in a less direct manner than other environmental components.

2.3.1 Wind

Wind in this region is primarily from the northwest, circulating around the Pacific High, and varies from winter to summer. The most notable exception to this trend are the Santa Ana winds, generated inland and blowing out over the coast. However, given the distance offshore of these facilities, the effects of this type of wind can be ignored. Another phenomenon of this region is the Catalina Eddy. This eddy causes recurvature of winds locally near the coast, but, for this project, it can also be considered insignificant [Stevens, 1977].

The wind speed can be expected to vary throughout the region under consideration, but estimates for the area should prove sufficient for this project. Values for wind speed were researched from several sources [Intersea Research Corporation, 1974; Stevens, 1977] and are given by return period in Table 2.1.

2.3.2 Waves

The primary wave direction offshore southern California is north to northwesterly, with little seasonal variation. Values for maximum wave height were researched from several sources [Intersea Research Corporation, 1974; Department of Navigation and Ocean Development, 1977; Stevens, 1977; API, 1989] and are given by return period in Table 2.1. It should be noted that for these values, maximum wave height is related to significant wave height by Equation 2.1 [Stevens, 1977].

$$H_{max} = 1.86 H_G \quad (2.1)$$

2.3.3 Current

Currents in this region are usually relatively small. It is normal practice to estimate current speed by a combination of two components. The first component is shear force imparted by wind. The second component consists of tidal flows and currents arising from the topography of the ocean floor. Surface currents are usually wind-driven, while subsurface currents are driven by geostrophic factors and tides.

In this project, the surface current speed will be taken as 4% of the steady wind speed, subsurface currents at mid-depth will be taken as one-half surface current speed, and near-bottom currents will be taken as one-third surface current speed [Stevens, 1977]. This approximation includes the small effect of tides. The resulting values for current speed correlate well with those found in other sources [Intersea Research Corporation, 1974]. Values for maximum current are given by return period in Table 2.1.

2.3.4 Weather Directionality

For the analyses done later in this paper involving environmental loads, it is necessary to know the primary direction of wind, wave and current. Since only extreme conditions are examined, directionality need only be known for these conditions.

In this region, storms are usually from the northwest. In a large storm, wind and wave direction will tend to coincide, especially when short wind gusts are discounted. Surface current will also follow the direction of the wind, as discussed in the previous section. Therefore, the environmental components will be assumed to act unidirectionally. There are some flaws to this assumption, as some directional spreading is inevitable and subsurface currents are ignored. However, directional spreading can be addressed, and it is proven in Chapter 4 that subsurface currents have little effect on the analytical models. Therefore, unidirectionality of wind, wave and currents will be assumed.

2.3.5 Seismic Activity

The southern California area has a relatively high degree of seismic activity. Offshore structures are usually analyzed for seismic safety by examining the effects of local activity as well as distant, more severe activity. The seismicity of a specific region, however, is highly variable, depending on local and distant fault positions. Since the area of interest in this project is located in deep water, fault positions are relatively unknown, and an accurate portrayal of seismicity in the region is not possible. Therefore, values

from a study of the entire offshore southern California area have been used for peak ground accelerations by return period [Bea, 1992]. These values are given in Table 2.1.

2.3.6 Ocean Floor Topography

The features of the ocean floor of both sites were evaluated by examination of nautical charts prepared by the National Ocean Service [U.S. Department of Commerce, 1991]. The purpose of this examination was to determine the slope of the floor in these locations and discover which locations would be unfavorable due to excessive slope, which may indicate a likelihood of sliding.

At El Segundo, the slope of the floor was found to vary from 12.8 degrees to 2.2 degrees at 1000 feet depth, with the slopes growing steeper to the south (Appendix 2). The only feature of note in this area is the Santa Monica Canyon, which is not especially deep. The slopes at Morro Bay were found to be less severe, ranging from 1.9 degrees to 0.9 degrees (Appendix 2).

2.3.7 Soils

Soils were investigated for the purpose of calculating anchor and anchor pile holding power, as well as determining the possibility of scour, sliding and other ocean floor phenomena which may have an effect on the facilities. The soils in this region consist of "clayey silt" to a depth of approximately 15 feet below the sea floor, and "stiff, silty clay" below this level to a depth of approximately 200 feet. The former type of soil has an undrained shear strength of approximately 1.5 kilopounds per square foot, while the latter soil type has an undrained shear strength of approximately 2.0 kilopounds per square foot [Woodward-Clyde, 1984].

Scour, slumping and sliding are all possible in this area, but risks should be lower in areas with shallower floor slopes [Intersea Research Corporation, 1974]. Silt soils, such as those at the sea floor, have a low resistance to scour. Clay soils have a lower susceptibility to scour, however, so while some scour may occur, it will not be severe, due to the presence of the stiff clay soils below the silt soils [Woodward-Clyde, 1983].

Liquefaction is probably a greater concern. This phenomenon involves the sudden loss of soil strength due to ground shaking or wave effects, and could have a severe effect on the holding power of pile anchors. The top soil type, "clayey silts" will be expected to experience some loss in strength, on the order of 15%, but soil strength loss below 50 feet should be negligible [Woodward-Clyde, 1978]. Therefore, if piles extend more than 50 feet below the sea floor, they should be relatively safe from liquefaction risks.

Chapter 3

System Configurations

3.1 Introduction

The purpose of this chapter is the examination of various configurations of SPMS in operation around the world, and the selection from these of two configurations which best meet the specific requirements of this project. All equipment necessary for tanker discharge, such as hawser lines, pipelines, transfer hoses and product risers, is also discussed. However, the main thrust of this project is the design of the SPMS, so only components directly related to the operation of the SPMS are examined in detail. Onshore facilities and offshore pipeline pumping stations are considered to be outside the scope of this project.

In the course of the project, systems were designed and then tested against the analytical models, reliability framework and feasibility criteria of later chapters. The designs were then reiterated as necessary to produce systems which met all applicable guidelines and rules. The trials of this iterative procedure are not repeated here; for the sake of conciseness only the final designs are presented.

3.2 Existing SPMS Types

Many types of SPMS are in use around the world, as can be seen in Appendix 1. The following is a non-exhaustive list of configurations of SPMS in operation : catenary anchor leg mooring (CALM), single anchor leg mooring (SALM), turret mooring, and fixed, articulated loading or catenary articulated tower. This list covers the major systems implemented to date. These systems are illustrated in Figures 3.1 through 3.4. Systems which use dynamic positioning can also be considered SPMS, although these were not considered in this project due to the cost associated with a purpose-built vessel of this type.

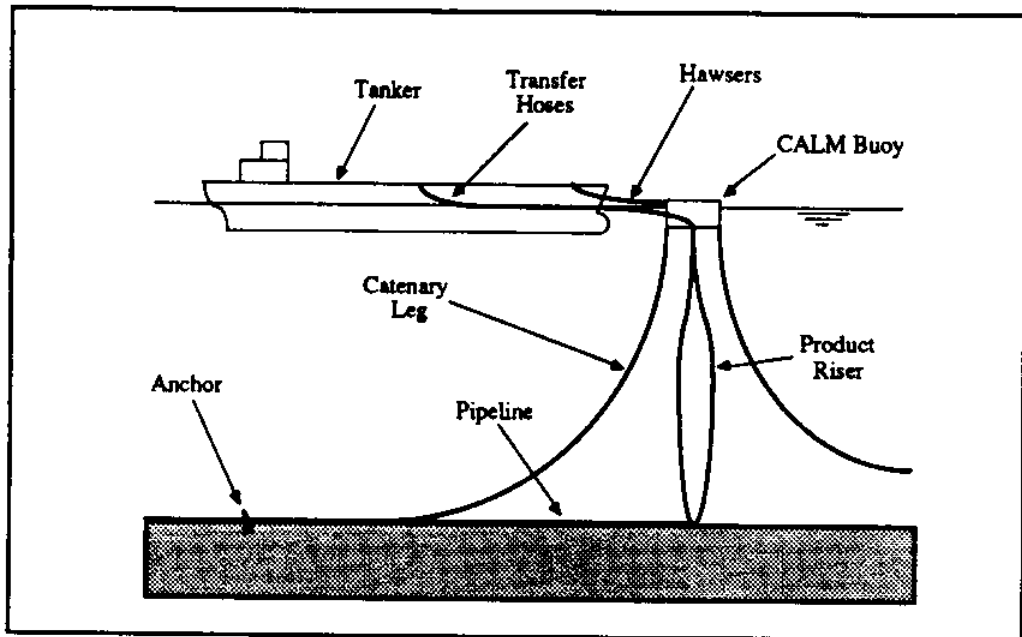


Figure 3.1 : Typical Catenary Anchor Leg Mooring Schematic

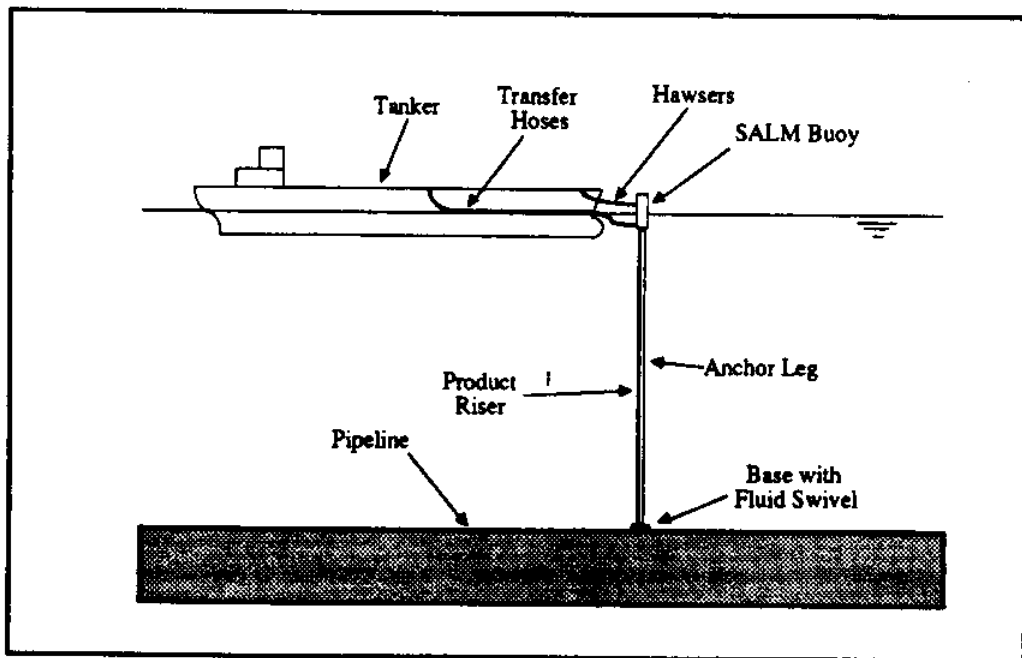


Figure 3.2 : Typical Single Anchor Leg Mooring Schematic

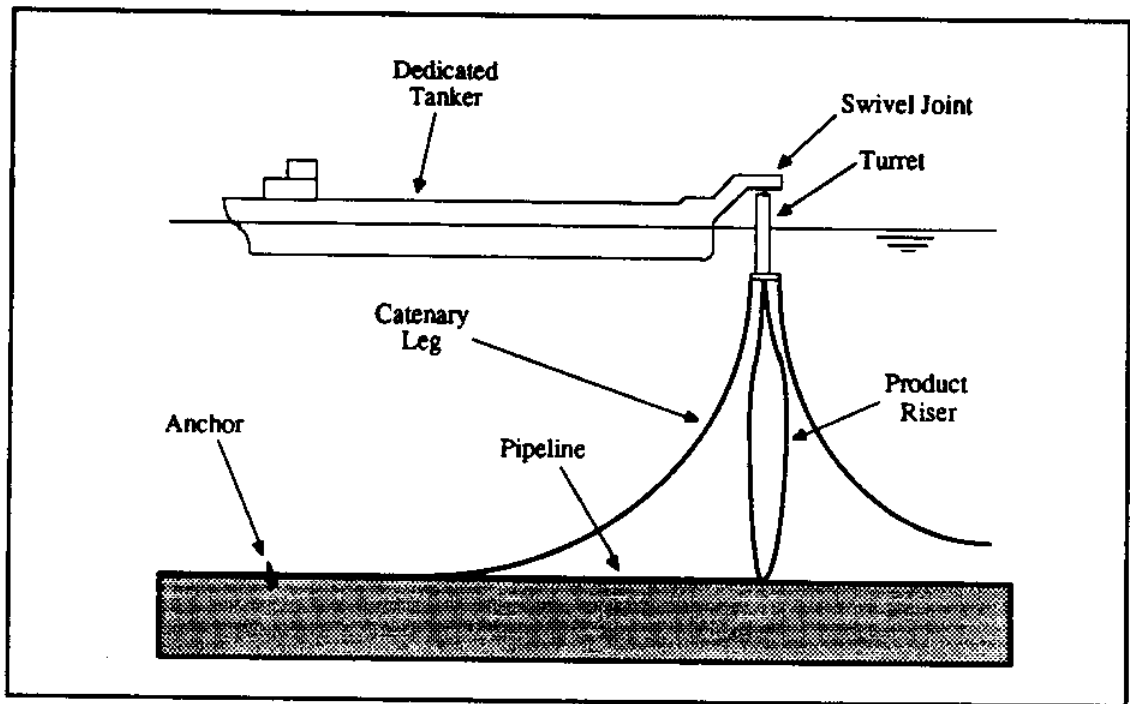


Figure 3.3 : Typical Turret Mooring Schematic

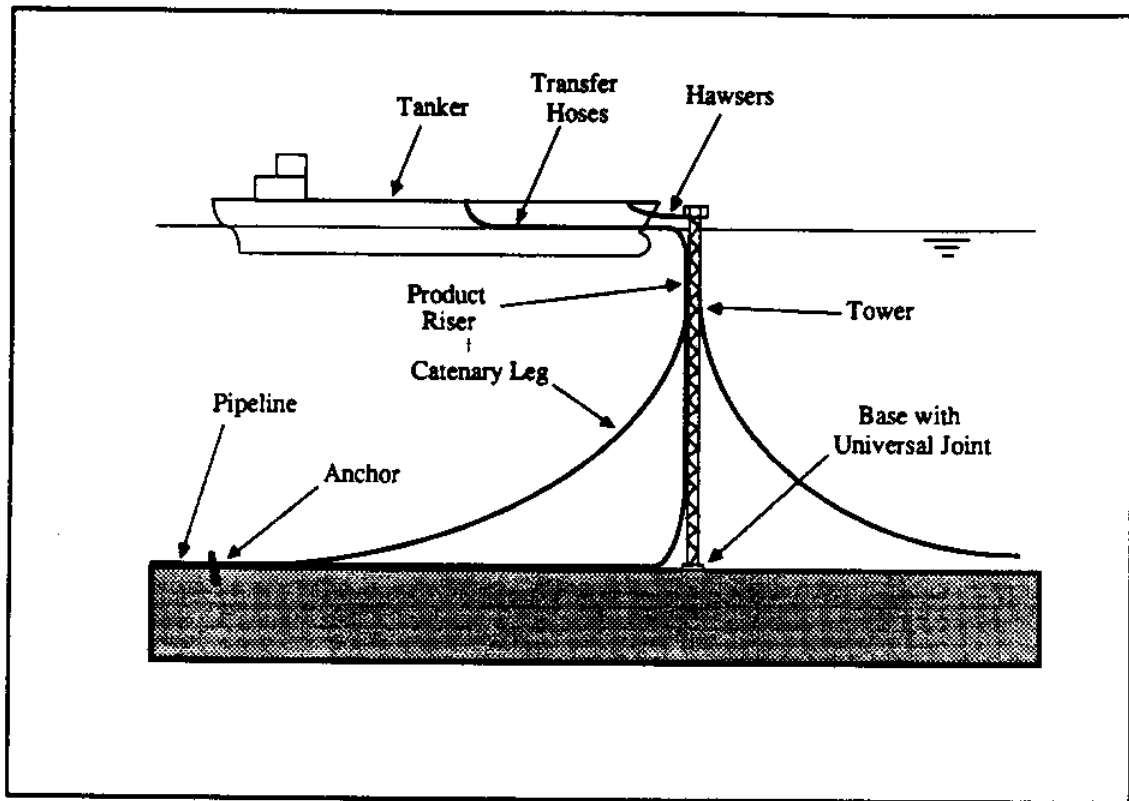


Figure 3.4 : Typical Tower Mooring Schematic

There is an even greater diversity of systems once the nature of major components is considered. The attachment between SPMS and tanker can be made by hawser, soft yoke or hard yoke (Figure 3.5). A SALM system can have either a flexible riser or a rigid riser, and the rigid riser may be articulated. The anchor legs of a CALM system can be made of chain, wire rope, or a combination of the two, with or without spring buoys. The CALM can use either drag-embedment anchors or pile anchors. Turret moorings can be internal or external to the moored vessel. Facilities can employ a permanent, dedicated tanker.

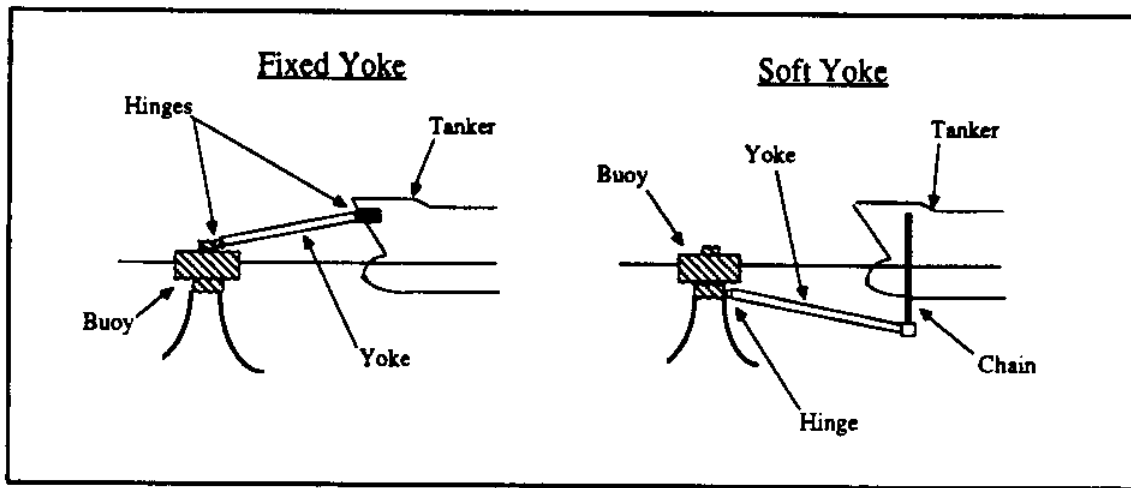


Figure 3.5 : SPMS Connection Type

Most of these decisions do not need to be made until the detailed design phase of the project. However, the type of systems to be examined and the nature of the connection from the SPMS to the tanker are decisions which must be made initially. The requirements already imposed upon this project limit the choices available for system configuration. Of the systems mentioned, towers are not suitable for unprotected waters or use in deep water depths. Turret moorings require either significant vessel modification or a permanently moored tanker. A permanently moored tanker was considered too costly to pursue, while vessel modification was prohibited in the project definition. Therefore, towers and turrets were discounted as inappropriate for this project.

The connection type was also decided based upon project requirements. While a hawser system is the simplest form of connection, it leaves motions of the tanker and buoy completely uncoupled. This is a drawback, as it can be a liability in withstanding harsh environments. This problem can be ameliorated by the use of a rigid yoke, ensuring that the tanker and buoy will have strongly coupled motions, or a soft yoke, causing some degree of coupling (Figure 3.5). However, use of either type of yoke requires vessel modification, and is therefore inappropriate for this project.

Therefore, due to the restrictions on vessel modification and the specified water depth, only the following systems remain as viable possibilities for this project : CALM with hawser connection and SALM with hawser connection. The CALM is the oldest and most common type of SPMS (Appendix 1), is relatively simple, and can be considered a baseline SPMS case. SALM systems are also well-proven, but have not been employed in this water depth to date.

The specific nature of the systems, such as the components of the CALM anchor legs, as well as the number of anchor legs and their layout, the type of anchor, and the type of anchor leg for the SALM system, are discussed in later sections.

3.3 Applicable Requirements

An initial criteria for this project was the requirement that both facilities meet all relevant rules and guidelines governing safety. In this project, the primary guidelines are the ABS factors of safety on mooring lines, anchors and fatigue, and the API "watch circle" guideline governing maximum buoy offset based on product riser type [Jones, 1992; API, 1991].

A factor of safety (F.S.) of 2.0 is required by ABS on mooring lines for floating production systems examined by quasi-static analysis in the intact condition [Jones, 1992]. The factor of safety is defined as the ratio of the capacity to the 100-year load, as calculated by either quasi-static or dynamic analysis. This factor of safety drops to 1.6 for the damaged condition, which refers to one mooring line broken. These and all other applicable ABS Factors of Safety are given in Table 3.1 [Jones, 1992]. Appendix 12 enumerates SPMS that have been classified by the ABS criteria.

Mooring Component / Method of Analysis	Holding Power	F.S. : Intact Condition	F.S.: Damaged Condition
Mooring Lines			
Quasi-static	Tension	2.0	1.6
Dynamic	Tension	1.67	1.33
Foundation			
Anchors			
Quasi-static	Tension	2.0	1.5
Dynamic	Tension	1.5	1.1
Piles			
Quasi-static	Vertical Load	2.0	1.5
	Horizontal Load	1.5	1.3
Dynamic	Vertical Load	1.5	1.25
	Horizontal Load	1.5	1.1
Fatigue			
Quasi-static	Tension	3.0	NA
Dynamic	Tension	3.0	NA

Table 3.1 : Applicable ABS Factor of Safety Requirements

As Table 3.1 shows, factors of safety for dynamic analysis are lower than those for quasi-static. The methods of analysis employed in some parts of this project (line tensions and anchor loads for the CALM system) are considered dynamic, while the remainder of the analyses are considered quasi-static.

The watch circle specified by API states that the maximum horizontal buoy offset from calm water position under the maximum design conditions (taken as the 100-year storm in this project) for systems employing flexible production risers in deep water (2000 feet to 3000 feet) must not exceed 15% of the water depth. For shallow water (below 300 feet) the criteria is 15% to 25% offset [API, 1991]. In this case, "flexible product riser" refers to any hose or pipe falling from a buoy in a catenary shape or attached to a vertical leg system such as a SALM. Therefore, a maximum offset of 15% of water depth (150 feet in this project) will be used as a design guideline.

Limits on operational sea states are another requirement that will be placed on these facilities. Requirements on operational sea states for the facility will be set by the Facility Manager, in conjunction with the Coast Guard. Establishing these operational constraints is necessary because both systems are disconnectable, and require disconnection in order to be adequately resilient to severe storm conditions. In operation, the tanker captain and the facility pilot must decide when to stop cargo transfer and disconnect from the SPMS due to adverse or expected adverse environmental conditions. Details of operation and disconnection are covered in Section 3.7.

3.4 Catenary Anchor Leg Mooring

A catenary anchor leg mooring (CALM) system consists of a set of anchored catenary legs arranged in a radial pattern around a large buoy, with some type of flow line from the sea floor to the buoy for transporting liquids (Figure 3.1). This is the most common type of SPMS (Appendix 1). A CALM system derives its restoring force to offset from the tension in the anchor legs due to the weight of the legs and an initial pretension [Ma, 1994].

The disadvantages of these systems are : disconnection requires substantial amounts of time (except for emergency disconnection) while weather may be deteriorating; operation is limited by the sea-keeping ability of the vessels assisting in line recovery; and tankers and other vessels can come into contact with the catenary legs, causing damage to the legs. A problem which this system shares with all other systems not involving dedicated vessels is the presence of floating hoses on the water surface. Two sections of approximately 1000 foot long hose will be floating on the surface when the facility is not in operation. These hoses are vulnerable to damage from passing vessels. This is why it is necessary to establish zones for the facility that are free of most maritime traffic and are constantly monitored for stray vessels (see Section 3.7, Operations, and [LOOP, 1992]).

The final design of the CALM for this facility is illustrated in Figure 3.6. This design is the result of iteration between the analytical models of Chapter 4 and the reliability and feasibility analysis of Chapters 5 and 6. The two main components of this system are the buoy and the anchor legs. The buoy is

60' in diameter, 25' in depth and weighs 271 LT. There are 8 anchor legs, arranged in a 45 degree spread. Each leg has three components : an upper chain section, a wire rope section and a lower chain section. The upper section is made of chain for tensioning and wear purposes, while the lower section is chain for bottom-abrasion purposes. The lower section also adds to the system holding power. Wire rope is used in the supported section of the leg because it gives a higher restoring force for a given pretension, due to its lower weight when compared with chain [API, 1991]. The other components of the CALM system are described in Section 3.6.

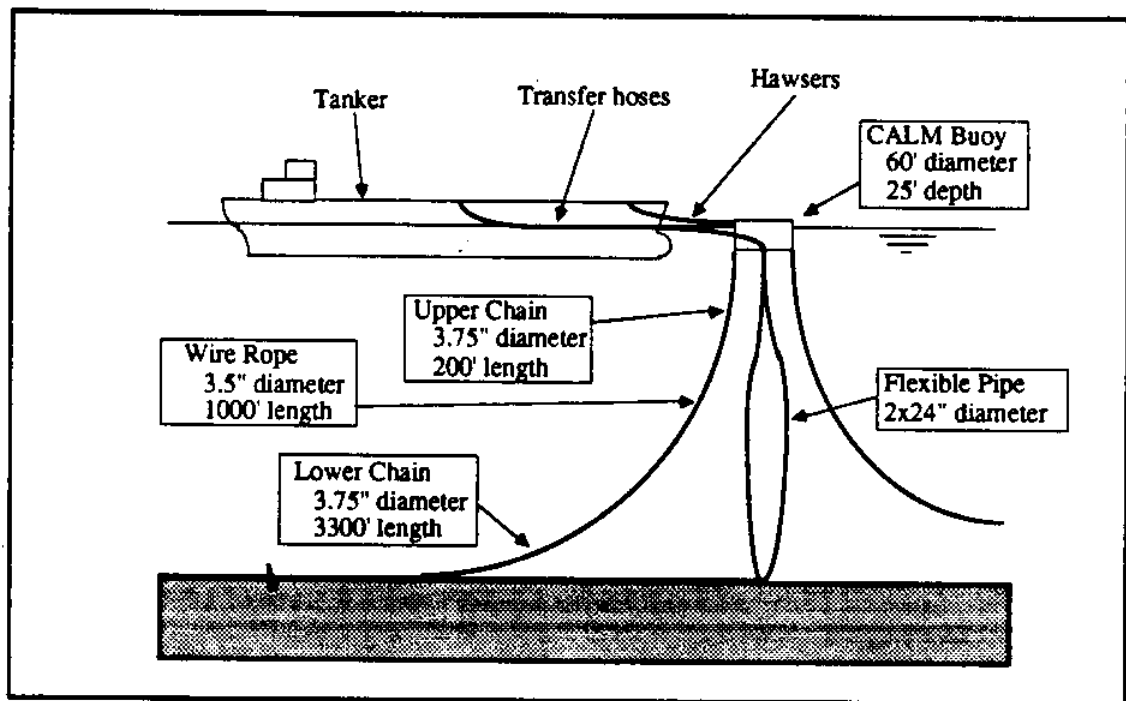


Figure 3.6 : Final CALM Design

3.5 Single Anchor Leg Mooring

A single anchor leg mooring (SALM) system differs from a CALM in that it has only one anchor leg, which is vertical and highly tensioned. A SALM derives its restoring force from this single tensioned leg by the horizontal component of the leg tension when angularly displaced, as well as the added buoyancy from the buoy resulting from horizontal displacement. A simple description of the physics of the system would be to describe it as an inverted pendulum (see Chapter 4, Figure 4.1). A SALM consists of a vertical buoyant riser, a foundation, a pretensioned leg from the sea floor to the riser, and a flow line from the sea

floor to the surface. A SALM has the same connection possibilities as a CALM system, which, in this project, means that only hawser connections are under consideration. The foundation is typically of the pile anchor type in deep water depths.

There is some variation in SALM designs in the nature of the tensioned leg employed : chains, wire rope, tubular risers and articulated risers have all been used, along with various combinations of these elements. In this project, wire rope was initially investigated as the anchor leg, but loadings and resulting tensions required switching to an articulated tubular riser.

A SALM has the advantages over a CALM of being more forgiving of collisions, due to the nature of its restoring force, and has a lower likelihood of contact and entanglement problems as the leg is located directly beneath the buoy.

A SALM system has many drawbacks, however. They are more complicated and expensive than a comparable CALM system. They have not yet been proven in this water depth. Maintenance can be a problem, as a SALM fluid swivel is considered to be a high-maintenance item, and is usually located at the sea floor. And, like the CALM, the floating hoses on the surface are vulnerable to damage.

The final SALM design for this project is illustrated in Figure 3.7. As with the CALM, this design is the result of iteration between the analytical models of Chapter 4 and the reliability and feasibility analysis of Chapters 5 and 6. The SALM buoy is 15' in diameter, 70' in depth and weighs 47.4 LT. The tubular riser is divided into three sections. The sections are, from top to bottom, 300', 350' and 320' in length. All four are four feet in outer diameter. The wall thickness varies by section due to the differences in hydrostatic pressures. The uppermost section has 0.5" thick walls, the middle section has 0.75" thick walls and the lower section has 1.0" thick walls. In this design, the fluid swivel is located in the bottom of the buoy, which makes it easily accessible. The articulated leg is divided into three sections, each of which is slightly buoyant prior to installation and made negatively buoyant during installation. The transfer hoses connect to the bottom of the SALM buoy.

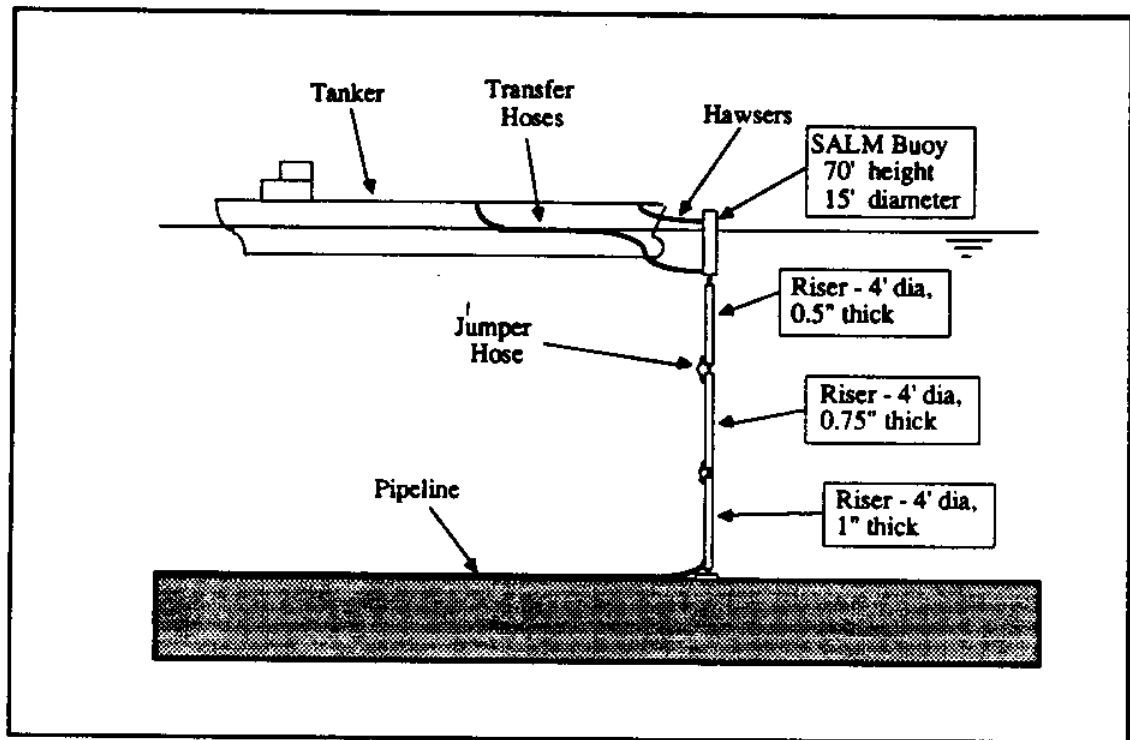


Figure 3.7 : Final SALM Design

3.6 Supporting Components and Systems

The elements of each facility under consideration in this project are : the SPMS, including buoy, anchor legs and anchors or anchor piles; the crude oil tanker discharging cargo to the facility; the pair of hawsers to hold the tanker in position while it is using the facility; two tending vessels to assist the tanker in the use of the facility; two transfer hoses for the transfer of crude oil from the tanker to the product riser(s); the product riser(s) to the sea floor; and the pipeline itself, running from the sea floor to the shore-side facility. It should be noted that some form of pumping stations will be required, but these are considered to be outside the scope of this project.

3.6.1 Tankers

The San Diego class of crude oil tanker was examined in this study, as this class is considered typical of the Valdez, Alaska to southern California trade. The particulars of this tanker class can be found in Table 3.2.

L. B. P. (feet)	Beam (feet)	Draft (feet)	Lightship (LT)	Full Load (LT)
915	166	59.3	30,000	188,500

Table 3.2 : San Diego Class Tanker Particulars

A typical rate of discharge for this size of tanker is 80,000 to 100,000 barrels per hour. This gives a time for discharge of approximately 17 hours for an entire cargo load of crude oil.

3.6.2 Product Risers and Pipeline

Product risers -- the flow line(s) used to transfer crude oil from the moored tanker to the ocean floor pipeline -- can be of three types : hoses, rigid pipe or flexible pipe. Hose will be used for the SALM system, as the product riser is attached to the anchor leg and need not support itself. Flexible pipe will be used for the CALM system. The flexible pipe will be free to hang in a catenary curve from the buoy to the sea floor.

Flexible pipe is preferred over rigid pipe because flexible pipe allows for larger relative motions and does not require heave compensation equipment. Flexible pipe consists of seven or more separate layers of material. These layers may be bonded or unbonded, although unbonded pipes are becoming the standard. The layers are typically (from inside out) : a steel carcass, a plastic sheet, a layer of wound steel wire, a flat steel carcass, an anti-friction plastic sheet, armor layers and an external plastic sheet. A typical value for minimum radius of curvature of flexible pipe is ten times pipe outer diameter [Sødahl, 1991].

Flexible pipe is well-proven in offshore applications. It has been used in water depths up to 850 meters. Over 2500 kilometers total of flexible pipe have been installed to date, with approximately half of that total presently over ten years old [Coutarel, 1992].

The design of the pipeline necessary for these facilities was considered to be outside the scope of this study. However, some important points concerning pipeline design for offshore southern California were found, and these are summarized here to give some feel for the difficulties involved in pipeline design.

Laying and servicing pipeline in 1,000 feet of water is within the reach of today's technology. However, the equipment required is not readily available on the West Coast, and would probably need to be delivered from the Gulf of Mexico, the North Sea, or Brazil. This would have a substantial impact on the installation and maintenance costs of the pipeline. Of the environmental conditions present at the locations examined in this study, only one significantly affects the design of sub-sea pipeline. This is the seismic activity of the southern California region [California Coastal Commission, 1978].

Seismic events can cause three possible actions : soil liquefaction, elastic ground waves, and inelastic, permanent ground movement. The issue of soil liquefaction has already been mentioned. Elastic ground waves typically have peak-to-peak lengths of several miles, and amplitudes of inches or fractions of

inches. These waves will not have a significant effect on ocean floor pipeline. Inelastic, permanent ground movement occurs along faults during seismic events, and may be either horizontal or vertical, with rupture regions up to several hundred feet in length. These ruptures are a major source of concern in pipeline design. Locating pipelines clear of faults is the best design solution, but this is often not feasible, as faults in deep water are difficult to locate. When it is known that a section of pipeline must cross a fault, the pipeline may be reinforced at this section or lifted off the ocean floor by bents. If the latter course of action is taken, the area must be prohibited to commercial fishing and any drag-type operations [California Coastal Commission, 1978].

Federal regulations specify that offshore pipelines for the transfer of liquids must be buried. Burial eliminates the possibility of damage from anchors and other ocean floor equipment. However, burial is very expensive, especially for large diameter pipelines such as those considered in this study. Of course, pipelines should not be buried in regions where they cross known faults. Burial can also be counter-productive in areas where liquefaction is likely [California Coastal Commission, 1978].

3.6.3 Tending Vessels and Connection Equipment

The following information on operations is based upon recommendations from Captain A. F. Fantauzzi of Chevron Shipping, the LOOP Operations Manual [LOOP, 1992] and interviews conducted at LOOP during this study.

Two vessels will be required for normal operations. One of these vessels will handle the hawsers during connection while the other retrieves the hoses. The primary mooring vessel must be sufficiently large and powerful to assist the tanker in adverse sea conditions. This vessel will be a standby/towing vessel of 60 to 80 meters length. It should be capable of operating in all weather conditions under which operation of the facility is to be conducted, with an added margin for emergencies. It will have hose handling capability of 10 meters by 22 meters with deck containment and drainage for crude oil. Wooden clad will be provided for servicing floating hoses. It must be highly maneuverable, with twin-ducted propellers, twin rudders and transverse thrust units. It should be equipped with two towing wires of approximate strength to the bollard pull, two towing winches (or a dual winch) and all necessary wire pennants and ancillary towing equipment. The required bollard pull will be based on the vessel being able to assist the tanker in the following environmental conditions : 35 knot wind, 1 knot adverse current and 3.5 meter significant wave height. This should give a required bollard pull of at least 60 tonnes. The vessel should also have fire-fighting capability. This vessel would be similar to the LOOP Responder, a 155 foot tractor tug with twin 7,300 hp engines, employing Voith Schneider propulsion.

The secondary mooring vessel is a line handling vessel which is smaller in size and does not require the same bollard pull capacity or sea-keeping capability. This vessel will pass the hawser messenger lines to the tanker in connection. This vessel will be similar to the LOOP Line and LOOP Loader. These vessels are launches of 85 foot length, with twin 1,200 hp engines.

Two hawsers will be used to connect the tanker to the SPMS buoy. The use of two hawsers for safety through redundancy is considered standard. The hawsers for use with both facilities examined in this project will be 200 foot long, 21 inch diameter nylon ropes with chafe chain attachments at both ends.

3.7 Operation

Figure 3.8 is a schematic of standard operational procedure with the two vessels assisting the tanker. It should be noted that a facility of this nature will require some type of vessel traffic control [LOOP, 1992]. This is considered to be outside the scope of this project, however.

A tanker using the facility will be guided to the SPMS by the vessel traffic controller (VTC) of the facility, and will be escorted by the primary mooring vessel, to insure against possible damage resulting from a loss of power by the tanker and subsequent drifting in the area of the facility. Once the tanker reaches the facility, the primary mooring vessel will be responsible for keeping the floating hoses clear of the tanker, while the secondary mooring vessel passes the hawser messenger lines to the tanker, connecting the tanker to the SPM buoy. The primary vessel then assists in connection with the floating hoses to the tanker manifold. Discharge can begin once the hoses are attached. The primary vessel maintains a stern tow on the tanker during operations to avoid sudden swings by the tanker which can result in very high transient loadings on the hawsers. Disconnection is handled in a similar fashion, removing the hoses from the manifold, lowering them into the water and then disconnecting the hawser lines from the tanker. Total time for connection, discharge and disconnection for both facilities is estimated to be 19 hours for San Diego class tankers.

Disconnection states must be determined for both systems. Disconnection will be carried out to avoid sea states which result in excessive loadings on the system for the attached condition. There will also be a margin on this disconnection sea state, to allow for worsening of weather during disconnection. Normal disconnection is estimated to take approximately one hour, including disconnection of the hoses and lowering the hoses into the water, but emergency disconnection should take no more than five minutes. This emergency disconnection involves the use of cargo pump emergency trips, and is not recommended for non-emergency use. Normally 20 to 30 minutes notice is recommended before hose disconnection is performed.

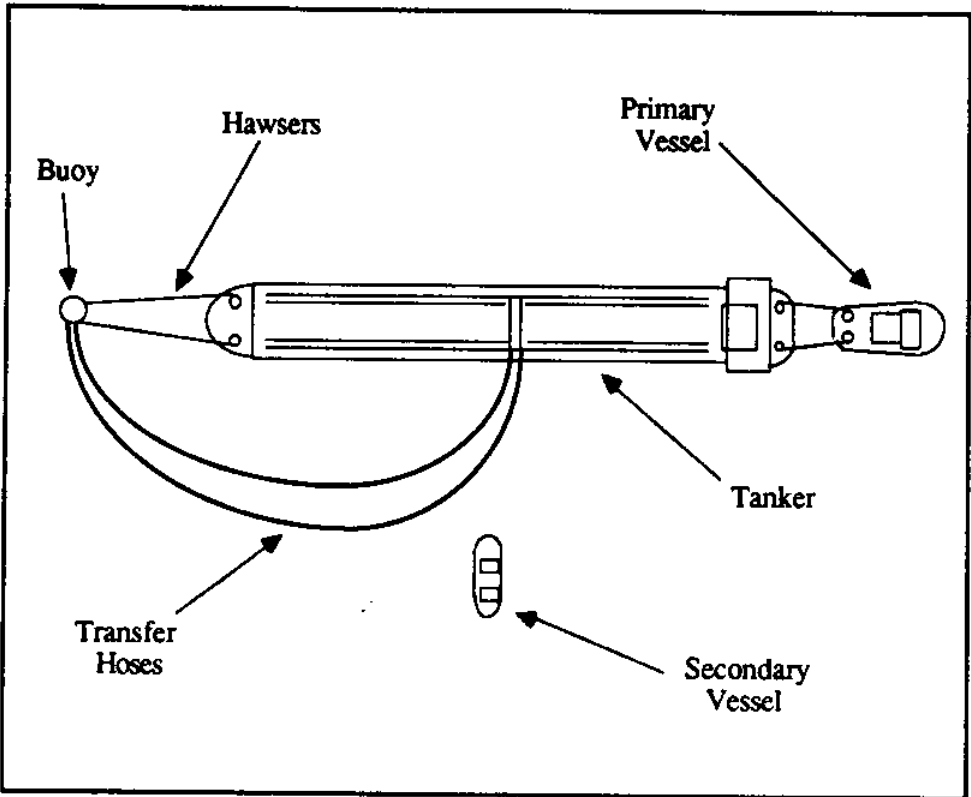


Figure 3.8 : Schematic of Operating Procedure

The determination of disconnection while in operation will be made by the captain of the tanker in conjunction with the pilot/facility manager. General guidelines for disconnection will be set by the facility in conjunction with the Coast Guard, as discussed in Section 3.3.

Chapter 4

Analytical Models

4.1 Introduction

The goal of this chapter is the examination of the effect of the environment on the proposed facilities in terms of line tensions, anchor/pile loadings and fatigue damage. This is done by first modeling the loads in the systems caused by the environmental components and then modeling the response of each facility to these loads. The environmental effects have been separated into four groups : steady forces, oscillating motions, seismic motions and fatigue. Which category each environmental component contributes to will be discussed in the following sections. The analytical models of each facility are also discussed, including line tensions and anchor loadings.

4.2 Steady Environmental Forces

Three environmental components act as relatively steady forces. These are : wind force, current force and mean wave drift force. Each is described in detail in the following sections. These forces are combined and applied to the facility to produce a steady offset, to which oscillatory offsets are added.

4.2.1 Wind

The effects of wind on the SPMS and other facility components have been evaluated by the conventional drag equation [Simiu, 1978]. Each above-water facility component (buoy, tanker hull and tanker superstructure) has been treated separately, with its own wind velocity based on centered elevation. Wind velocities were given in Chapter 2 for a 10 meter elevation above still water. These velocities must be adjusted to the component centered elevation velocity by Equation 4.1 for each component [Simiu, 1978].

$$V_z = V_{10} \left(\frac{Z}{10m} \right)^{0.125} \quad (4.1)$$

This velocity is then modified by gust duration as in Equation 4.2 [Bea, 1993]. The velocities given in Chapter 4 are for a one-minute duration gust. It was decided that a three minute gust duration is appropriate for calculating steady force, due to the period of low frequency motions, following the example of Hunter, et al, for evaluating wind loadings on moorings and vessels [Hunter, 1993].

$$V_r = V_{1min} \left[1 + \frac{1}{2} \ln \left(\frac{1min}{t} \right) (0.00535 + 0.00042 V_{1min})^{1/2} \right] \quad (4.2)$$

The steady wind force is then calculated from Equation 4.3 with this adjusted wind velocity.

$$F_{wind} = \frac{1}{2} \rho_{Air} C_S V_z |V_z| A \quad (4.3)$$

The wind shape coefficient is considered typical of marine systems with relatively solid shapes [Bea, 1993]. The total steady wind force can be found in Table 4.1, while the calculations are given in Appendix 4.

4.2.2 Mean Wave Drift

The effect of waves on the facility in question have been separated into three components : first order wave motions (wave frequency motions), second order wave motions (low frequency motions) and a mean wave drift force [API, 1987]. While model tests would be a superior measure, this approach is considered to be an adequate approximation. The mean wave drift force is considered to be a steady force component, while the first and second order motions are considered oscillating motions.

Mean wave drift force was calculated based on Equation 4.4 [Le Tirant, 1990] for the tanker in the head-seas condition. It can be seen from the equation that wave drift is roughly proportional to the square of the wave amplitude, and for a constant amplitude, the mean wave drift increases as wave period decreases.

$$F_{Bow, drift} = 0.13 C_{Drift} B^2 L H_s^2 \quad (4.4)$$

The average drift coefficient for tankers is 0.05 [Le Tirant, 1990]. Significant wave height is determined by Equation 2.1. For the buoy and the riser, the mean wave drift force is calculated by Equation 4.5 [Le Tirant, 1990]. The drift coefficient is taken as $1175 \text{ N s}^2 \text{ m}^{-4}$.

$$F_{Mean\ drift} = C_{Df} D_{Buoy}^2 \left(\frac{H_s}{T_s} \right)^2 \quad (4.5)$$

The resulting mean wave drift force can be found in Table 4.1, while the calculations are given in Appendix 4.

4.2.3 Current

The current forces have been evaluated by the drag equation for the buoys and risers [Bea, 1993] and by the API guidelines for the tanker [API, 1991] with depth effects linearized, as described in Section 2.2.3. Equation 4.6 gives the steady current force on the buoys and the risers.

$$F_{Current, buoy} = \frac{1}{2} \rho_{SW} C_D V_C |V_C| A \quad (4.6)$$

The drag coefficient was taken as 1.0 for the buoys and 0.9 for the risers [Bea, 1993]. The steady current force on a tanker for the head-seas condition is given by Equation 4.7 [API, 1991].

$$F_{Current, Ship} = 0.016 S V_C^2 \quad (4.7)$$

The wetted surface was calculated by Equation 4.8 [Lewis, 1988].

$$S = C_{WS} (\Delta_{Vol} L)^{1/2} \quad (4.8)$$

The wetted surface coefficient was taken as 2.7, a typical value for tankers of the San Diego class size [Lewis, 1988]. The current force can be found in Table 4.1, while the calculations are given in Appendix 4.

System Component	Wind Force (Long Tons)	Mean Wave Drift Force (Long Tons)	Current Force (Long Tons)
SALM buoy	0.60	0.33	1.18
CALM buoy	0.76	5.36	1.65
Tanker, lightship, superstructure	12.25	—	—
Tanker, full load, superstructure	11.53	—	—
Tanker hull, lightship	11.82	9.12	0.86
Tanker hull, full load	4.37	9.12	2.16
Flexible Pipe	—	0.01	1.24
Hoses	—	0.04	2.06

Table 4.1 : Steady Environmental Forces, Two-Year Return Period Conditions

4.3 Oscillating Environmental Motions

Two of the environmental components can be treated as oscillating components. These are first order (wave frequency) motions and second order (low frequency) motions, both due to wave action. For the connected condition, the tanker was considered to generate the governing motion, and buoy motions were ignored. The motions were combined by adding the maximum wave frequency motions to the significant low frequency motions [API, 1987].

4.3.1 First Order Wave Motions

Wave frequency motions have been determined by the use of a ship motions program, SEAWAY [Journée, 1992]. SEAWAY is a PC-based ship motions program using ordinary and modified strip theory method. It calculates wave-induced loads and motions with six degrees of freedom. SEAWAY can simulate moorings as well, by adding up to six linear springs to the model.

SEAWAY was used to model the San Diego class tanker and both the CALM and SALM buoys. Springs were then added to these models to simulate the mooring restoring force. Although the mooring restoring force is not perfectly linear, for small offsets this is a relatively good approximation. These models were tested against the given wave events for various return periods. The details of this program and the results of this analysis can be found in Appendix 5. The offsets due to first order motions are given in Table 4.2.

4.3.2 Second Order Wave Motions

Low frequency motions have been determined by use of the API curves [API, 1987]. These curves were generated for drill ships in the 400 foot to 540 foot range, but with the given correction factors for vessels outside this length range, these curves should be an adequate approximation for tankers. A separate set of curves for semisubmersible hulls was used for low frequency motions of the SPMS buoys. The calculations for these motions can be found in Appendix 5. The offsets due to second order motions are given in Table 4.2. Total motions were determined by adding the maximum wave frequency motions to the root mean square low frequency motions.

	Wave Frequency Motions (feet)	Low Frequency Motions (feet)
Tanker, full load	2.4	2.9
Tanker, lightship	2.7	2.9
CALM buoy	6.2	11.4
SALM buoy	7.2	8.6

Table 4.2 : Oscillating Motions, Two-Year Return Period Conditions

4.4 Seismic Activity

Seismic motions were examined for the SALM system only. This is because seismic motions will effect only systems with significant vertical stiffness. The SALM must have a high vertical stiffness due to its method of providing restoring force, but the CALM system does not require a high vertical stiffness.

The SALM system was modeled as a spring. PCNSPEC [Mahin, 1983], an earthquake analysis program, was used to determine peak motions. PCNSPEC is an inelastic response program for viscously damped single-degree-of-freedom systems. The SALM system was tested against the El Centro earthquake, scaled to have the peak ground accelerations given in Table 2.1 for the various return period events. The resulting offsets are given in Table 4.3. As the values in the table indicate, seismic motions are not large, especially considering the anchor leg's 1000 foot length.

The input and output of PCNSPEC are described more fully in Appendix 6.

Return Period	Maximum Relative Vertical Offset (feet)
2-year	0.0123
10-year	0.0617
100-year	0.2217
1000-year	0.4922

Table 4.3 : SALM Vertical Seismic Offsets

4.5 Fatigue

Fatigue is defined as the degradation of component characteristics (such as strength or stiffness) due to cyclic straining and stressing. In this application, the cycling is due to wave action. Cycling can also result from operations (cargo pumping, operation of other equipment), construction (installation, transport to installation, launching) or other environmental components (thermal changes, wind, current, earthquakes) [Bea, 1990]. However, wave cycling is considered to be the dominant source of cyclic loading in this project.

Fatigue effects are calculated for the chain, wire rope and connections in the CALM system, and for the riser and articulations in the SALM system. These components are considered to be the most likely components to fail due to fatigue.

The calculation of fatigue "load" is done in a different manner than for other environmental loads. Since fatigue failure is a result of cumulative damage over a (usually) long period of time, it is more appropriate to determine a mean fatigue life. This mean fatigue life is the expected life of the component or structure before failure due to cyclic fatigue occurs. The mean fatigue life is calculated by Equation 4.9 [Bea, 1990].

$$T_{\mu} = \frac{\Delta K}{B^n \Omega} \quad (4.9)$$

The accumulated fatigue damage parameter is set equal to 1.0 for fatigue failure. The stress range model error parameter is taken as 0.80, a standard value for the marine environment [Bea, 1990]. The values for the negative slope and the log life intercept are taken from the S-N curve for a particular component [API, 1989; API, 1991; Bea, 1990]. These values are given in Table 4.4.

Component	m	K (cycles)
Wire Rope	4.09	1.30×10^{10}
Chain	3.36	4.60×10^9
Connections	3.74	1.79×10^{10}
Tubular Riser	4.38	1.50×10^{12}
Articulations	3.74	1.79×10^{10}

Table 4.4 : Component Fatigue Characteristics

These values for K are biased, however. This bias is a result of the standard practice of offsetting S-N curves by two standard deviations for design guidelines. This bias is removed by Equation 4.10, which, in effect, adds the two standard deviations back to K.

$$B_K = \frac{\bar{K}_{True}}{\bar{K}_{Nominal}} = \frac{\exp[\ln(\bar{K}_{Nominal}) + 2\sigma_{lnK}]}{\bar{K}_{Nominal}} \quad (4.10)$$

The natural log of the standard deviation of the fatigue life is calculated by Equation 4.11.

$$\sigma_{lnT} = \left[\ln(1 + C_\Delta^2)(1 + C_K^2)(1 + C_B^2)^{m'} \right]^{1/2} \quad (4.11)$$

where :

$$C_\Delta = 0.3$$

$$C_K = 0.73$$

$$C_B = 0.5$$

The values given for the various coefficients of variation are considered typical for marine systems [Bea, 1990].

The unbiased values for K can be found in Table 4.5. The stress range parameter is calculated by Equation 4.12.

$$\Omega = \lambda(m) f_0 S_m^m [\ln N_T]^{-m/\epsilon} \Gamma \left[\frac{m}{\epsilon} + 1 \right] \quad (4.12)$$

The rainflow correction and epsilon are both taken as 1.0, considered a typical value for both variables for marine systems [Bea, 1990]. The design period was taken as 100 years, and the number of cycles was based on this length of time and an average wave period of 13 seconds. The average frequency of

the stresses was taken as the inverse of the period of the waves. The gamma function has been approximated by Sterling's asymptotic formula, Equation 4.13 [Froberg, 1985].

$$\Gamma(z) = \left(\frac{z}{e}\right)^z \sqrt{\frac{2\pi}{z}} \left[1 + \frac{1}{12z} + \frac{1}{288z^2} - \dots\right] \tag{4.13}$$

The fatigue design stress range is calculated in Equation 4.14.

$$S_{FD} = \left[\frac{KH_{FD}^{\alpha m}}{T_{SF} Y_0}\right]^{1/m} \tag{4.14}$$

where :

$$T_{SF} = F_{SF} T_{DP}$$

$$Y_0 = \frac{N_T}{T_{DP}} H_{FD} (\ln N_T)^{-m\alpha/\epsilon} \Gamma\left(1 + \frac{\alpha m}{\epsilon}\right)$$

The fatigue life safety factor is typically 3.0 for marine systems [Bea, 1990]. This value is related to the nominal design stress range by the stress concentration factor, as given in Equation 4.13. This nominal design stress range is then used as the largest stress value in Equation 4.10.

These relations result in the mean fatigue lives given in Table 4.5. The calculations of these values can be found in Appendix 7.

Component	Unbiased K value	Mean Fatigue Life (years)
Wire Rope	4.8x10 ¹⁰	66,820
Chain	1.7x10 ¹⁰	25,460
Connections	6.6x10 ¹⁰	42,072
Tubular Riser	5.5x10 ¹²	98,034
Articulations	6.6x10 ¹⁰	42,072

Table 4.5 : Summary of Mean Fatigue Life

These mean fatigue life values may seem surprisingly large. This question is addressed in Section 5.6.

4.6 CALM Analytical Model

The details of the analytical model of a CALM are not discussed in depth here. The reader is referred to Ma [1994] for a detailed explanation of the model used in this project. After several design iterations, it was decided to use a system pretension of 23.6 LT for the CALM. The line tensions and anchor loads are summarized in Table 4.6 by return period.

	Line Tension (LT)	Anchor Load (LT)
CALM Buoy		
2-year conditions	31.7	15.6
10-year conditions	36.6	19.6
100-year conditions	42.9	25.4
Buoy with Lightship, 2-year conditions	45.5	26.3

Table 4.6 : CALM Line Tensions and Anchor Loads

The holding power of drag-embedment anchors needed to be determined for the CALM system [API, 1991]. The chain section of the anchor leg lying on the sea floor also contributes to the holding power of the system, as described by Equation 4.15.

$$P_c = f_{co} L_c W_c \quad (4.15)$$

The coefficient of friction between chain and the ocean bottom is taken as 0.7 [API, 1991]. The unit weight of 3.75" chain is 132 pounds per foot. Approximately 2000' of chain will be in contact with the ocean bottom during extreme conditions. This results in a chain holding power of 82.5 LT.

The anchor was then selected based on anchor loads and the given chain holding power. Bruce FFTS anchors of 6.7 LT weight (15 kilopounds) and 192 LT holding power (430 kilopounds) were selected, giving a total anchor leg holding power of 274 LT.

4.7 SALM Analytical Model

A SALM derives its restoring force from the tension in its leg, as described in Section 3.5. This leg may be a chain, wire rope or solid riser. The choice depends on the tension required and the specifics of the application. In this project it was decided that a solid riser would be necessary to support the tension required. Articulations were necessary in the riser due to the water depth of 1000 feet.

There are two components involved in SALM restoring force : the horizontal component of the leg tension and the added buoyancy due to offset which acts to increase leg tension. The design inputs for a

SALM are : buoy size, riser size, number of riser sections, weight of buoy and risers, and the dimensions of the anchor pile. The anchor leg of a SALM is given a pretension so that it will develop an adequate restoring force. In this design, a pretension of 300 LT was decided upon.

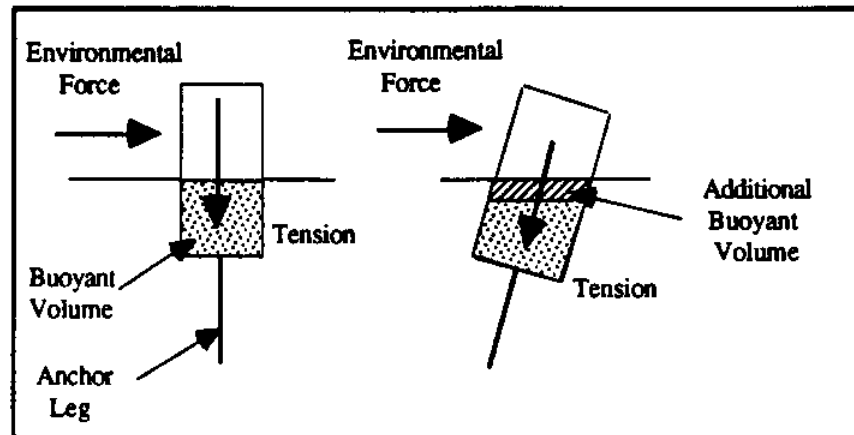


Figure 4.1 : SALM Restoring Force Schematic

With system dimensions, weights and pretension decided, the SALM could be modeled analytically. The horizontal environmental force acting on the system must be offset by the horizontal component of the system tension, which is a function of the system pretension and the added buoyancy due to offset, which are both functions of the angular displacement of the anchor leg. The spreadsheets used for the calculation of this iterative procedure are given in Appendix 9. The line tensions for storms and seismic events are given in Table 4.7.

	Leg Tension, Storm (LT)	Leg Tension, Seismic (LT)
SALM Buoy		
2-year conditions	275	272.8
10-year conditions	278.3	273.8
100-year conditions	284.5	277.3
Buoy with Full-load ship,		
2-year conditions	300.5	200.3
Buoy with Lightship,		
2-year conditions	311.7	200.3

Table 4.7 : SALM Storm and Seismic Leg Tensions

The determination of anchor pile characteristics were based on the leg tensions, as these act directly on the anchor. The anchor load is considered to be strictly vertical, due to the small angular

displacement values involved (usually under 10 degrees for extreme conditions). It was decided, based upon the reliability analysis described in Chapter 5, to have a target pull-out capacity of 2,000 LT. The ultimate pull-out capacity is given by Equation 4.16 [API, 1989].

$$Q_p = f_{sc} A_s + W_p \quad (4.16)$$

The unit skin friction capacity is determined by Equation 4.17 [API, 1989].

$$f_{sc} = \alpha_p c_{uss} \quad (4.17)$$

The undrained shear strength was determined to be 2.0 kilopounds per foot in section 2.3.7. The dimensionless pile factor is taken as 1.0 [API, 1989].

From these relations, the length of a pile can be estimated. Wall thickness was based upon API criteria for minimum wall thickness, as given in Equation 4.18 [API, 1989].

$$T_{WT} = 0.25 + \frac{D_p}{100} \quad (4.18)$$

With these criteria, a pile was selected of 140' length, 60" diameter and 1" wall thickness. This gives a pull-out load of approximately 2,000 LT.

Chapter 5

Reliability

5.1 Introduction

This chapter examines the determination of the probability of failure for each SPMS facility. Failure is defined as the breakage of one or more legs of the SPMS, insufficient holding power developed by the anchors or anchor pile, major damage to the risers / hoses, or damage to the buoy which would halt operations. The total probability of failure is the sum of the annual chances of these events occurring, as given in Equation 5.1 (neglecting small cross-product terms).

$$P_{f, Total} = P_{f, Mooring\ Legs} + P_{f, Anchors} + P_{f, Risers} \quad (5.1)$$

The probability of failure of the facilities under consideration is dependent upon four relatively independent failure hazards. Failure may be due to storm loadings, seismic loadings, cyclic fatigue loadings, or human and organizational error (HOE). The total annual probability of failure is expressed in Equation 5.2 (again neglecting small cross-product terms).

$$P_{f, total} = P_{f, storms} + P_{f, seismic} + P_{f, fatigue} + P_{f, HOE} \quad (5.2)$$

The method of computation of each one of these components is examined in depth in the following sections.

5.2 Component Probability of Failure Calculations

The procedure for calculating probabilities of failure for storm loadings, seismic loadings and cumulative fatigue damage is outlined in this section. The analyses are based on a log normal - log normal relationship

between component loadings and component capacities, whether loadings are due to storms, seismic events or fatigue degradation. This type of relationship is considered typical of large marine systems analyzed over a long time period [Bea, 1990]. The mean values of load (line tensions, anchor loadings and fatigue damage) and capacity (line strength, anchor holding power and mean fatigue life) are known, either by calculation (in Chapter 4) or from manufacturer's specifications. Variances of these properties are considered, as well as possible correlation between load and capacity.

The probability of failure for a log-normal system is determined by Equation (5.3).

$$P_f = 1 - \Phi(\beta) \quad (5.3)$$

In some cases, values for the safety index fall outside those tabulated in the standard normal distribution table. In these cases, an approximation due to Abramowitz [Melchers, 1987] is used. This approximation was found to be a superior fit for the values of beta encountered in this project (Appendix 10).

The safety index, β , is defined in Equation 5.4 for components exposed to lognormally distributed loadings due to storms or seismic events. This equation relates the median load to the mean capacity, with the effect of variations and correlations included.

$$\beta = \frac{\ln\left(\frac{R_{50}}{S_{50}}\right)}{\sqrt{\sigma_{\ln R}^2 + \sigma_{\ln S}^2 - 2 \rho_{RS} \sigma_{\ln R} \sigma_{\ln S}}} \quad (5.4)$$

The safety index is defined by Equation 5.5 for cyclic fatigue degradation. This fatigue analysis differs from the environmental analysis in that it is based on a time frame instead of a peak loadings frame, as explained in Section 4.5. This means that capacity will be replaced by mean fatigue life and load will be replaced by accumulated fatigue damage, as in Equation 5.5, where mean fatigue life is that calculated in section 4.5.

$$\beta = \frac{\ln\left(\frac{T_{\mu}}{T_{SL}}\right)}{\sigma_{\ln T}} \quad (5.5)$$

The components of both peak load-based and time-based relations are discussed in the following sections.

5.2.1 Load and Capacity

The mean loadings used in the peak load analysis are the 2-year return period loadings determined in Chapter 4 for storms and seismic events, and the mean fatigue life for the seismic analysis. The capacities are the ultimate limit state capacities given by the manufacturer, which are listed in Table 5.1 [Avallone, 1987; Bureau Veritas, 1980].

5.2.2 Variance and Deviation

There are two types of uncertainty in structures analyzed by reliability methods. Type I uncertainties refer to natural or inherent randomness, such as peak environmental conditions. This type of uncertainty cannot be controlled. Type II uncertainties refer to modeling uncertainties. This type of uncertainty includes uncertainties in computations of forces, uncertainties in measurement and uncertainties due to limited data sets. This type of uncertainty is systematic. It is also information sensitive and can be reduced by acquiring additional information, whether the information is research, inspection or quality control / assurance [Bea, 1992].

For distributions with Type I uncertainties, the variance of the distribution can be measured by two parameters. The coefficient of variation is a normalized measure of the variability of a parameter. The standard deviation is a measure of dispersion or variability of a distribution, as is the natural log standard deviation. The relations between these parameters are given in Equations 5.6 and 5.7.

$$V_x = \frac{\sigma_x}{X} \quad (5.6)$$

$$\sigma_{\ln x} = \sqrt{\ln(1 + V_x^2)} \quad (5.7)$$

However, these relations require more information than has been generated to this point. Therefore, Equation 5.8 is more suitable for the calculation of Type I uncertainties, as loads for the 2-year and 100-year return periods are known. Equation 5.8 calculates the coefficient of variation from the 2-year return period and the 100-year return period loads.

$$\sigma_{\ln x} = \frac{\ln(X_{99}/X_{50})}{2.33} \quad (5.8)$$

Systems were analyzed for the disconnected case to determine loading variance, as the systems should never be connected for the 100-year condition. This gave the variances listed in Table 5.2 for

environmental Type I uncertainty. The values for capacity Type I uncertainty [Yong, 1991; Bea, 1990] are considered typical for marine system components. The anchor / anchor pile variation is based on the soil type [Bea, 1990]. These uncertainties are listed in Table 5.1. It should be noted that the systems were designed so that the connections are the most likely element of each system to fail. This was done because the connections are the easiest component of each system to maintenance and replace. Their failure should also cause the least amount of damage. However, for the SALM system, nearly any failure will be a serious one, as its single load-path allows for no redundancy.

Component	Capacity (LT)	Variance
CALM		
Wire rope	447	0.10
Chain	321	0.10
Connections	100	0.10
Anchors	274	0.40
SALM		
Articulations	600	0.10
Riser	700	0.10
Anchor Pile	2000	0.40

Table 5.1 : Component Reliability Characteristics

Type II uncertainties are more difficult to determine, as they must be based on historical analysis of analytical methods. Therefore, representative values were taken from existing literature for this project. For storm loadings on marine structures, the Type II variation has been estimated as 0.07 to 0.11 [Bea, 1992; Nikolaidis, 1992]. A value of 0.10 was taken for this project. For seismic loadings, literature Type II variation ranged from 0.0 to 0.31 [Bea, 1992; Nikolaidis, 1992]. A value of 0.10 was selected for this project, based on the modeling tool used (PCNSPEC) and the small effect of seismic loadings in line tensions. Type II uncertainties in fatigue analysis are discussed in section 5.6.

System	Leg Loads (Storm)	Leg Loads (Seismic)	Anchor Loads (Storm)
CALM	0.115	—	0.186
SALM	0.015	0.007	0.015

Table 5.2 : Environmental Variance

The Type I and II variances are combined into a single variance by Equation 5.9. This relation is valid for systems which have independence between load and capacity. This independence is discussed in the next section.

$$V_X = \sqrt{V_S^2 + V_R^2 + V_H^2} \quad (5.9)$$

5.2.3 Correlation

The correlation coefficient expresses how strongly two variables are related. A value of 1.0 indicates perfect correlation, while a value of 0.0 indicates complete independence. The correlation coefficient can be negative as well, up to a value of -1.0, which indicates perfect negative correlation. There are three types of correlation which required examination in this project : correlation between load and capacity, correlation between capacities of different components, and correlation between failure modes of different components.

The determination of load to capacity correlation is best carried out by model testing, as it is very difficult to determine analytically. There may be some small positive correlation in these systems due to larger wave heights (higher loads) encountering more structure (higher capacity), but this may be offset by slight changes in environmental directionality. Therefore, it was assumed that there would be no correlation between load and capacity.

The correlation between capacities of different elements is likely to be very high, due to similarities in design and manufacture, and has been taken as 1.0.

5.3 System Probability of Failure Calculations

The system probability of failure is determined by calculating component probabilities of failure from the given relations and reducing the systems into series and parallel elements. The CALM system is composed of eight catenary anchor legs, which are modeled as series-loaded subsystems. These subsystems are combined to form an eight element parallel system. The SALM system is much simpler, being composed of one tensioned leg modeled as a series system.

One new attribute of a system must be considered in the calculation of system reliability. Correlation can exist between system components, due to their manufacture and due to their modes of failure. An estimate of system failure mode correlation based on relative uncertainties attributed to Cornell [Bea, 1990] is used here (Equation 5.10).

$$\rho_{FM} = \frac{V_S^2}{V_R^2 + V_S^2} \quad (5.10)$$

For series elements, the system reliability is calculated by Equation 5.11. This equation is valid for systems with perfect element-to-element correlation. Although this assumption concerning correlation may not be completely true here, it is a good approximation. As has been pointed out elsewhere [Bea,

1990], the probability of failure of a system is well approximated by the probability of failure of the most-likely-to-fail element in the system.

$$P_{f, system} = \max(P_{f, element}) \quad (5.11)$$

For parallel elements, the system reliability is calculated by Equation 5.12. This equation is valid for normally distributed identical parallel elements.

$$\beta_s = \beta_E \sqrt{\frac{N}{1 + \rho_E(N - 1)}} \quad (5.12)$$

It should be noted that failure of one leg is essentially system failure for the CALM system (it has already been defined as failure for the SALM). Failure of one leg of the CALM will halt operation. Therefore, "system failure" for the CALM refers to one leg broken or severely damaged.

5.4 Failure due to Storm Loadings

The probability of failure due to storms must cover two separate cases : the SPMS alone and the SPMS with an attached tanker. Loads in the system will be much higher with a tanker present. The facility will have guidelines governing when a tanker using the facility should disconnect from the SPMS to reduce loads. Therefore, the probability of failure due to storm loadings can be expressed as given in Equation 5.13.

$$P_{f, storms} = (P_{f, storms} | disc)(1 - P_{conn}) + (P_{f, storms} | conn)(P_{conn}) \quad (5.13)$$

The percent of time in which a tanker is using the facility is an economic decision, affected only slightly by environmental conditions and facility downtime. The downtime of the facility is expected to be very small, according to gathered data. Typical downtime values for systems operating for one year or more range from 0% to 3.2% [Key, 1993]. For this study, three San Diego class tankers will use the facility per week. Given the 17 hour discharge time of this tanker class and the estimated 2 hour connection and disconnection time, the facility will be in use approximately 31% of the time, allowing for a 3% system downtime.

The probability of failure while disconnected is a relatively straightforward calculation, comparing disconnected loadings with capacities, as in Equation 5.4. All data for this calculation have already been determined, and the resulting probability of failure can be found in Table 5.5. Failure while connected is a

more difficult calculation, due to the question of what sea states will be encountered before disconnection will occur. Disconnection can be ordered to occur at the appearance of a certain sea state, but the actual occurrence of disconnection depends upon perception of the sea state by the captain and pilot, and weather deterioration which may occur before disconnection can be completed. Therefore, it is necessary to model disconnection as a probability distribution in relation to sea state. The probability of failure while connected can therefore be expressed as in Equation 5.14.

$$P_{f, storms} | conn = \sum_{all\ sea\ states} [(P_f | conn, sea\ state) \times P_{sea\ state} \times P_{conn} | sea\ state] \quad (5.14)$$

The probability of a given sea state being the annual maximum encountered is a function of the annual maximum wave height distribution. The probability of connection in a given sea state is based upon assumed behavior and the uncertainty associated with identifying sea states and weather deterioration. The values for these probabilities are given in Table 5.2. The probability of connection in a given sea state is based on an instruction to disconnect to avoid operation (and connection) during conditions with wind exceeding 32.5 knots or waves exceeding 25.5 feet maximum, or a 13.7 foot significant wave height. The distribution is log normal over the spectrum of sea states. The distribution is based on the mean disconnection state being the 25.5 maximum wave height state, with a 5% chance of being connected at the 29.5 foot maximum wave height state. This distribution has been assumed and should be verified.

P sea state	Wave Height (feet)	Wind Velocity (knots)	Current Velocity (knots)	Probability of Connection
10	21	26.5	0.8	0.90
10	23.5	30	0.9	0.70
10	25.5	32.5	1.02	0.50
10	27	35.5	1.1	0.25
10	28.5	38	1.18	0.10
10	29.5	40.5	1.28	0.05
10	31	43.5	1.35	0.01
10	32.5	46.5	1.45	0.002
20	37	55	1.7	0.0005

Table 5.3 : Probability of Connection by Sea State

The probability of failure for a given sea state while connected is then calculated based on Equation 5.3, with the mean loading based on the mean wave height, wind speed and current speed for a given sea state, and variance as previously determined. The probabilities of failure are then summed over all sea states

to produce a total probability of failure for the SPMS while connected to a tanker. The results of this calculation are given in Table 5.4.

5.4.1 Storm Loadings by Sea State

The loadings due to various sea states are given in Table 5.3. These loadings are based on the same approach used to calculate loadings for the various environmental return periods. The variance on the loadings are those for Type I and Type II uncertainties given earlier in the chapter. The correlation of load and capacity remains equal to 0.0. The probability of failure is also given in Table 5.3. This is the probability of failure for the connected condition and the given sea state being the maximum sea state encountered in a year.

Plsea state	Maximum Wave Height (feet)	CALM Leg Tension (LT)	CALM Anchor Tension (LT)	SALM Leg Tension (LT)	CALM Probability of Failure	SALM Probability of Failure
10	21	32.1	15.4	283.4	2.4×10^{-13}	4.8×10^{-7}
10	23.5	35.0	17.7	289.3	3.0×10^{-12}	6.3×10^{-7}
10	25.5	37.7	19.9	297.2	2.3×10^{-11}	7.1×10^{-7}
10	27	41.1	22.6	301.9	1.9×10^{-10}	1.1×10^{-6}
10	28.5	44.1	25.1	309.1	3.5×10^{-9}	1.6×10^{-6}
10	29.5	46.4	27.5	316.5	4.3×10^{-8}	3.5×10^{-6}
10	31	50.7	30.6	326.2	7.9×10^{-7}	9.4×10^{-6}
10	32.5	55.8	34.9	337.3	1.9×10^{-5}	2.7×10^{-5}
20	37	70.1	47.1	373.3	6.7×10^{-3}	4.5×10^{-5}

Table 5.4 : Storm Loadings and Probability of Failure by Sea State

5.4.2 Reliability for Storm Loadings

System reliability is calculated for each facility based on the component probabilities of failure, as explained earlier. The probabilities of failure are combined with the probabilities of given sea states being encountered and connection during that sea state, as in Equation 5.13. The probabilities of failure are then summed over all sea states, as in Equation 5.14. The resulting values for probability of failure are given in Table 5.5 for the case of storm loadings.

System State	Percent Time	CALM P_f	SALM P_f
Connected	31%	6.7×10^{-7}	2.4×10^{-7}
Disconnected	69%	1.6×10^{-11}	3.4×10^{-7}
TOTAL	100%	2.1×10^{-7}	3.0×10^{-7}

Table 5.5 : Probability of Failure due to Storm Loadings

5.5 Failure due to Seismic Loadings

The probability of failure due to seismic loadings is calculated in the same manner as the storm loadings for the unconnected facility. However, seismic loadings are considered to be significant for the SALM system only, as the CALM system has relatively little vertical stiffness. Seismic loadings were calculated in Section 4.4, while capacities are given in Table 5.1. Variations in load are as given in section 5.2.2. Correlation between load and capacity is again assumed to be equal to 0.0.

It will be noted that the variance on seismic loadings is lower than that of storm loadings, which may seem counter-intuitive. However, seismic loadings on the SALM system are of relatively low magnitude, and tend to be overshadowed by the constant pretension force of the system.

System reliability for seismic loadings is similar to that for storm loadings. Since there is only one load path, the system is a series one. As in other system reliability sections, the system probability of failure of a series system is well approximated by the probability of failure of the most-likely-to-fail component.

The probability of failure of the SALM system due to seismic loadings is given in Table 5.7.

5.6 Failure due to Cyclic Fatigue Loadings

Fatigue reliability is characterized by mean fatigue life (analogous to capacity), service life (analogous to load) and a deviation on the fatigue life, as in Equation 5.5. Mean fatigue life is calculated in section 4.5, while service life is a design decision. The natural log of the standard deviation of the fatigue life is calculated by Equation 4.11.

The resulting natural log standard deviation is found to be 1.53. It should be pointed out that this variance is very high in comparison with other variances calculated in this chapter.

The fatigue probabilities of failure, as calculated by Equation 5.5, are given in Table 5.6. It can be seen that the large mean fatigue lives calculated in Section 4.5 are offset by the large variance calculated in equation 5.15, giving probabilities of failure similar to those for other environmental components.

Component	Probability of Failure
CALM	
Chain	2.4×10^{-7}
Wire Rope	5.8×10^{-8}
Connections	1.2×10^{-7}
SALM	
Riser Sections	3.3×10^{-8}
Articulations	1.2×10^{-7}

Table 5.6 : Component Probability of Failure due to Fatigue

These probabilities of failure are for a service life of 20 years. To see the effect of service life on probability of failure, Figure 5.1 is a graph of reliability versus service life for chain, the element most likely to suffer from fatigue degradation.

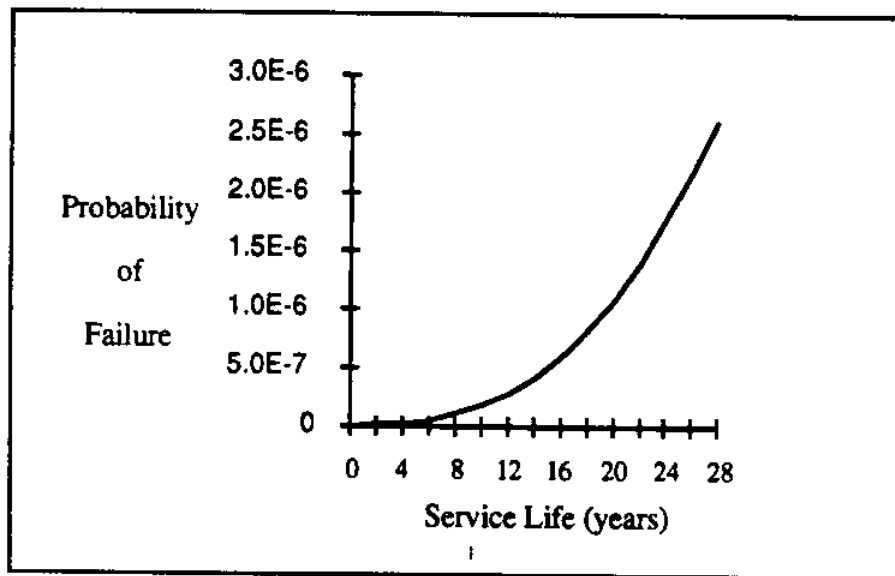


Figure 5.1 : Fatigue Reliability versus Service Life

This figure serves to illustrate the concept of preventative maintenance. By periodically replacing the elements of a system which are likely to suffer fatigue damage, the probability of failure is lowered.

The system effect of fatigue reliability is similar to that for other types of system reliability with high correlation. The most likely to fail element's probability of failure is taken as the system probability of failure.

5.7 Failure due to Human and Organizational Error

Human and Organizational Error (HOE) covers failures attributable to humans (individuals), organizations (groups of individuals) and systems (structures and equipment) [Moore, 1993]. Approximately 80% of high-consequence marine accidents can be attributed to HOE.

The modeling of HOE is very complex. It involves the culture and specific work methods of an organization, which have not been detailed in this project for these facilities and are considered to be outside the scope of this research. However, a description of the procedures and contingencies at the LOOP facility will be helpful in understanding the type of culture and work methods encountered at this type of facility in the US [LOOP, 1992]. LOOP experiences the same types of risk from HOE that these facilities can be expected to face : vessel traffic problems, exposed floating transfer hoses, communication between ship, tending vessels and shoreside facility, etc.

At the LOOP facility, a Port Superintendent is always physically present at the facility. It is the Port Superintendent's responsibility to direct all actions in the event of an emergency. The LOOP facility defines emergency conditions as those which involve or could involve : safety, environmental protection, personnel injury or property damage. These conditions could occur at the Marine Terminal, on a tanker, on any other vessel or aircraft, or at any other location lying within the port's safety zone. Examples of emergency conditions include :

- Oil spill
- Fire or explosion
- Tanker collision (actual or potential, with other vessel or platform)
- Tanker grounding
- Electrical power failure on platform
- Disruption of communications between shore and port
- Aircraft disaster
- Serious illness, injury or death
- Presence of poisonous gas
- Evacuation operation of the platform

The LOOP facility has a Safety Zone established around it. This zone consists of three sections : the approach section, the anchorage section and the terminal section. The approach section is a 2 nautical mile wide corridor leading to the terminal. The anchorage section is a 2 NM by 4 NM area adjacent to the approach section. The terminal section is approximately 2.5 nautical miles in radius from the pumping complex platform.

Inside the terminal section, there are four "Areas to be Avoided." One is a 600 meter radius around the platform, the other three are 500 meter radii about the SALM buoys.

5.8 Facility Probability of Failure

The probabilities of failure calculated in the previous sections are summarized in Table 5.7. These probabilities are the annual probabilities of failure due to each loading case for each facility. The total probability of failure for each facility is the sum of these individual probabilities of failure for each facility. This total probability of failure assumes independence between the individual probabilities of failure. Although values for probability of failure have not been calculated for HOE, it should be noted that these probabilities of failure may be very significant.

Failure Component	CALM Facility	SALM Facility
Storm Loadings	2.1×10^{-7}	3.0×10^{-7}
Seismic Loadings	—	4.6×10^{-6}
Fatigue Degradation	2.4×10^{-7}	1.2×10^{-7}
Human and Organizational Error	???	???
TOTAL	4.5×10^{-7}	5.0×10^{-6}

Table 5.7 : Facility Probabilities of Failure

5.9 Acceptable Reliability

An acceptable reliability can be determined from the factors of safety given in Table 3.1. This is done by equating the factor of safety (FS) to the 100-year load and the mean capacity, as in Equation 5.16 [Jones, 1992; Bea, 1990].

$$FS = \frac{R_{99}}{S_{50}} \quad (5.16)$$

Using the relations developed in this chapter, Equation 5.16 can be manipulated to give Equation 5.17, relating the safety index to the deviations and factor of safety.

$$\beta_{\text{acceptable}} = \frac{2.33\sigma_{\ln S} + \ln(FS)}{\sigma_{\ln}} \quad (5.17)$$

Table 5.8 presents the applicable factors of safety and the calculated target probabilities of failure (for the intact condition only).

System Component	Factor of Safety	Acceptable P_f
SALM anchor legs	2	7.1×10^{-8}
CALM anchor legs	1.67	9.8×10^{-6}
Fatigue (all components)	3	1.0×10^{-3}
CALM anchors	1.5	3.2×10^{-4}
SALM anchor leg	2	3.8×10^{-5}

Table 5.8 : Acceptable Reliabilities

It can be seen that all components meet the acceptable reliability except for the SALM anchor legs.

Chapter 6

Feasibility

6.1 Introduction

This chapter examines the cost associated with the two facilities investigated in this study. Cost information is relatively rare in published literature. Therefore, all information given in this chapter has been obtained from industry contacts, based on existing structures or extrapolated from existing structures. The information regarding SPMS costs is courtesy of M. Steven Mostarda of IMODCO, while the information regarding tankers and tending vessels is courtesy of Capt. A.F. Fantauzzi. Unfortunately, it was not possible to obtain an estimate of the cost of the pipeline during the research. The pipeline is expected to account for a large percentage of the total facility cost, due to the water depth, the pipeline length and the lack of deepwater equipment on the West Coast. The cost of shoreside facilities, vessel traffic control equipment and personnel is considered to be outside the scope of this study.

6.2 System Hardware

The cost of the hardware of the CALM system has been estimated as \$13.5 million. This figure includes the cost of the CALM buoy, eight catenary anchor legs, drag embedment anchors, product risers, hawsers and transfer hoses. It does not include the cost of the pipeline or any onshore facilities, including vessel traffic control. This cost estimate was based upon the existing Marlim CALM facility offshore Brazil [Hwang and Bensimon, 1990].

The cost of the hardware of the SALM system has been estimated as \$12.5 to \$14 million. This price is more uncertain than that of the CALM, as no SALM systems have been constructed for this water depth to date.

6.3 System Development and Engineering

For the both the CALM and the SALM system, the total cost of engineering, certifying, project management, construction and installation supervision, and transportation has been estimated as \$1.25 million. This figure is also based on the Marlim CALM facility.

6.4 Installation

The installation of the CALM system is expected to take 20 to 25 days. The installation is estimated to cost from \$3 to \$5 million. This high cost is due in part to the necessity of bringing equipment to the site from the Gulf of Mexico, as deepwater equipment is not readily available on the West Coast.

The installation of the SALM system cannot be estimated as accurately. It is expected to be more expensive, due to the necessity of driving the anchor pile at the sea floor. An estimate of \$5 million is used in this study.

6.5 Total Initial Costs

The total facility costs are given in Table 6.1. These total costs do not include the pipeline or any onshore facilities. The costs also do not include vessel traffic control. The entry "Other" refers to contingencies, insurance and overheads.

Cost Component	CALM Facility (millions of \$)	SALM Facility (millions of \$)
Hardware	13.5	12.5 to 14
Development & Engineering	1.25	1.25
Installation	3 to 5	5
Other	1.5	1.5
TOTAL	19.25 to 21.25	20.25 to 21.75

Table 6.1 : Total Initial Facility Costs

These figures indicate that the initial cost of the two systems are comparable. However, there is a greater degree of uncertainty regarding the SALM estimates, as no comparable system exists.

6.6 Costs of Operation and Maintenance

Table 6.2 outlines the cost of operation for the tending vessels which will be used at the facilities. The total yearly cost is based on three days of operation per week, following the earlier assumption that the

facility will be visited by three tankers per week, each taking approximately 19 hours to connect, discharge and disconnect. No spot chartering has been assumed in the cost estimate.

Activity	Annual Estimated Cost
Yearly contract (vessel and crew, per tending vessel)	\$2.4 million
Fuel (on a consumption basis)	1000 gallons per day @ \$0.50 per gallon
Spot Charter	\$6500 per day (plus fuel consumption)
TOTAL ANNUAL COST	\$4.96 million

Table 6.2 : Annual Operation Costs

The cost of tanker charter is approximately \$40,000 per day.

Preventative maintenance will be carried out on the hawsers used to connect tankers to the SPM buoys. These hawsers can have severe fatigue problems, and they can cause serious problems when they break by snapping back. It is common practice to replace them often -- LOOP uses only 20% of the calculated fatigue life before replacing hawsers. Hawsers will be replaced every six months at the facilities to avoid fatigue problems. The cost of hawsers was not available.

Chapter 7

Conclusions and Recommendations

7.1 Summary

This study has examined the determination of feasibility of single point mooring systems (SPMS) for use as deepwater ports for the import of hazardous liquid cargoes offshore southern California. Two configurations of SPMS were examined : CALM and SALM. The study examined the environmental conditions at two sites, developed analytical models with which to evaluate the suitability of SPMS, determined the reliability of the systems by use of state-of-the-art reliability methods, and evaluated the feasibility of the systems.

7.2 Conclusions

Several preliminary conclusions can be drawn from the work conducted in this study. These conclusions have been divided into two groups : deepwater ports and SPMS, and reliability analysis of SPMS. The conclusions are detailed in the following sections.

7.2.1 Deepwater Ports and SPMS

It has been proven by the US Coast Guard that deepwater ports are a viable way to reduce the environmental risks arising from the import of crude oil. This study has shown that two types of single point moorings can be used -- without stretching today's technology -- as deepwater ports offshore southern California.

The CALM system developed in this study is a relatively simple design. The engineering is not complex and the system does not require custom-built components other than the CALM buoy itself. The

CALM proved to be relatively robust in reliability analysis, due to its inherent safety by redundancy -- failure of one anchor leg will not entail catastrophic failure for the system.

The SALM system was also a relatively straightforward design. However, it requires more complex engineering and more custom construction of components. It also requires much more attention for reliability, as the system is not inherently robust -- failure in one leg is a catastrophic failure. The system also required a very high pretension to provide adequate restoring force to meet API guidelines concerning system offset. This high pretension dictated the need for the unusual components. It also necessitated the high vertical stiffness of the SALM also means that more attention must be paid to seismic effects.

The study also showed that disconnection criteria are very important in reliability based design of systems which disconnect to avoid extreme environmental conditions. The greater probabilities of failure associated while connected strongly affect total system reliability.

In summary, there are no technical barriers to the use of SPMS for deepwater ports offshore southern California in 1,000 feet of water. However, a CALM system appears to be much simpler than a SALM, with a greater degree of control of risk in the design.

7.2.2 Reliability Analysis of SPMS

The reliability analysis showed the importance of characteristics which might otherwise have been overlooked. It was shown that seismic loadings, although low in magnitude, can have a substantial effect on system reliability. Type II uncertainties can have a large effect on reliability of systems, especially systems which have relatively little Type I variation in loading and capacity. These Type II uncertainties are often overlooked, due to the difficulty in their determination.

The correlation between load and capacity has a great effect on reliability. This is also often overlooked, as it can rarely be determined without the use of model testing. Human and Organizational Error can play a substantial role in system reliability, and is perhaps the most difficult reliability component to evaluate. The benefits of preventative maintenance were clearly proven in the reliability analysis.

7.3 Recommendations for Future Work

In the course of this research, several topics were touched upon which were too broad for detailed examination, or required tools or testing facilities which were unavailable. These topics would make good

subjects for future work. These recommendations for future work are divided into three groups : environmental analysis, facility design, and reliability analysis.

7.3.1 Environmental Analysis

Several components of the environmental analysis carried out in this study could be further pursued. The exact directionality of the environmental components at the specified sites would result in more exact loadings modeling. This would require environmental data more specific to the sites than that currently available.

Further examination of seismic characteristics, i.e. the exact positions of local and distant faults, would enhance the precision of the seismic analysis. The low frequency modeling could be carried out by a more exact tool than the API guidelines.

The most substantial work in extension of this study would be to create an uncoupled model for tanker and SPM loads and motions. This would give more accurate dynamic offsets for given environmental conditions.

7.3.2 Facility Design

Other types of SPMS deepwater ports could be examined, such as permanently moored tankers, or deepwater port systems employing dynamic positioning. These types of facilities were not examined in this study because their cost was considered to be too high.

The design of the facilities could be extended to include shoreside operations, pipeline and vessel traffic control (VTC). The effect of other tanker sizes using the facilities could be investigated.

7.3.3 Reliability Analysis

The reliability analysis could be extended in several ways. Further examination of Type II uncertainties, in the form of more historic data on analytical modeling, would result in a more accurate model. Model testing to determine load/capacity correlation would also improve the accuracy of the reliability model. The reliability for the case of one leg damaged could be investigated for the CALM system. Lastly, the disconnection distribution could be verified and improved by examination of historical disconnection data.

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Appendices

Appendix 1 : Partial List of Existing SPMS

The following is a partial list of existing SPMS, their location, installation date and configuration. This list has been provided by M. Steven Mostarda of IMODCO. The configuration listing of "CALM" (Catenary Anchor Leg Mooring) includes all SPMS connected by rigid or soft means to permanent production/storage tankers utilizing catenary legs. The listing of "SALM" (Single Anchor Leg Mooring) includes all single anchor leg systems (single chain, rigid leg or articulated rigid leg). The listing of "Tower" refers to any unarticulated fixed mooring structure, including jacket structures when they are attached to soft moorings.

NORTH AMERICA

Canada

Location/Name	Installed	Config.	Location/Name	Installed	Config.
St. John's, NB	1970	CALM	St. John's, NB	1987	CALM

United States

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Louisiana	1967	CALM	Worldwide	1984	SALM
LOOP-SPM 102	1980	SALM	Worldwide	1986	SALM
LOOP-SPM 103	1980	SALM	Worldwide	1986	SALM
LOOP-SPM 104	1980	SALM	Worldwide	1986	SALM
Worldwide	1980	CALM	Hawaii	1986	CALM
Hondo	1981	SALM			

CENTRAL AMERICA

Mexico

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Tuxpan I	1973	CALM	Dos Bocas I	1982	CALM
Tuxpan II	1976	CALM	Cayo Arcas I	1981	CALM
Santa Cruz I	1976	CALM	Cayo Arcas II	1982	CALM
Santa Cruz II	1977	CALM	Cayo Arcas III	1982	SALM
Rabon Grande I	1978	CALM	Santa Cruz III	1982	CALM
Rabon Grande II	1980	CALM	Back up Buoy	1985	CALM
Rosarito Beach	1980	CALM	Vera Cruz	1988	CALM
Dos Bocas II	1980	CALM			

Panama

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Chiriqui Grande	1981	CALM	Chiriqui Grande	1983	CALM
Chiriqui Grande	1981	CALM			

Dominican Republic

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Santo Domingo	1972	CALM	Palenque	1984	CALM

Trinidad & Tobago

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Galeota Point	1972	CALM	Galeota Point	1976	CALM
Pointe a Pierre	1973	CALM			

SOUTH AMERICA

Colombia

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Covenas	1985	CALM	Covenas	1986	CALM

Venezuela

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Moron	1968	CALM	Quintero Bay	1971	CALM

Chile

Brazil

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Tramandai SBM I	1969	CALM	Pampo	1981	CALM
Tramandai SBM II	1971	CALM	Corvina	1982	CALM
Sao Francisco	1976	CALM	Garoupa	1982	CALM
Garoupa	1977	SALM	Linguado	1982	CALM
Garoupa	1977	SALM	Albacora SBM 05	1987	CALM
Garoupa	1978	CALM	Albacora	1987	CALM
Enchova	1978	CALM	Pirauna SBM 02	1988	CALM
Tramandai & SF	1979	CALM	Bicudo SBM 01	1988	CALM
Arenbepe	1980	CALM	Bonito EMH 01	1989	CALM
Garoupa	1981	CALM	Garoupa SBM 03	1989	CALM
Balejo	1981	CALM	Marlim	1990	CALM
RJS 28A	1981	CALM			

Uruguay

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Jose Ignacio	1976	CALM	Jose Ignacio	1988	CALM

Argentina

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Puerto Rosales	1970	CALM	Punta Ciquena	1980	CALM
Caleta Olivia	1974	CALM	Hidra	1989	CALM
Caleta Cordova	1979	CALM			

Ecuador

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Balao Terminal	1972	CALM	Balao Terminal	1978	CALM
Balao Terminal	1972	CALM			

NORTH EUROPE**Sweden**

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Oxno	1959	CALM	Dalarna	1959	CALM

Norway

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Ekofisk	1971	CALM	Gullfaks	1983	SALM
Ekofisk	1971	CALM	Gullfaks	1983	SALM
Statfjord A	1976	SALM	Statfjord A	1987	CALM
Statfjord B	1979	SALM	Statfjord B	1990	CALM
Statfjord C	1982	SALM			

Denmark

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Dan	1971	CALM	Frederikshavn	1982	SALM
Gorm	1979	CALM			

Britain

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Nore Estuary	1965	SALM	Buchan	1979	CALM
Tetney	1971	CALM	Maureen	1980	SALM
Auk	1972	CALM	Beryl	1981	CALM
Argyll	1974	CALM	Fulmar	1981	SALM
Beryl I	1974	SALM	Beryl 2	1981	SALM
Flotta	1975	Tower	Tetney	1981	CALM
Flotta	1975	Tower	Falkland Isles	1985	CALM
Montrose	1975	CALM	Beryl 3	1985	SALM
Montrose	1975	CALM	Birch	1988	CALM
Anglesey	1975	CALM	Crawford	1989	CALM
Brent	1975	CALM	SWOPS	1989	CALM
Thistle	1975	SALM	Emerald	1990	SALM
Brent Standby	1977	CALM	Kiitiwake	1990	CALM

West Germany

Location/Name	Installed	Config.
Cuxhaven	1962	CALM

MEDITERRANEAN**Spain**

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Huelva	1967	CALM	Algeciras	1975	SALM
Amposta	1972	CALM	Castellon	1976	SALM
Taaragona	1974	CALM	Badalona	1979	SALM

Italy

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Flumicino	1963	CALM	Nilde	1978	SALM
Flumicino	1963	Tower	Rospo Mare	1981	CALM
Ravenna	1963	CALM	Genoa	1983	Tower
Flumicino	1970	Tower	Vega	1985	SALM
Porto Torres	1971	CALM	Mila	1985	CALM
Ancona	1972	Tower	Rospo Mare	1985	SALM
Genoa	1973	Tower	Nilde	1985	CALM
Ravenna	1975	Tower			

Egypt

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Ras el Shaqiq	1968	CALM	Alexandria	1976	CALM
Alexandria	1974	CALM	Agami	1976	CALM
Alexandria	1974	CALM	Alamein	1978	CALM
Alexandria	1974	CALM	Ras Bhudran	1980	CALM
Suez	1974	CALM	Suez	1981	CALM
Suez	1976	CALM	Alexandria	1982	CALM
Suez	1976	CALM	El Zeit Bay	1983	CALM
Alexandria	1976	CALM	El Zeit	1984	CALM

Libya

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Marsa el Brega	1963	Tower	Assawiya	1976	SALM
Ras es Sider	1965	CALM	Assawiya	1976	SALM
Zuetina	1968	CALM	Zuetina	1979	CALM
Zuetina	1969	CALM	Ras es Sider	1980	CALM
Marsa el Brega	1970	SALM	Ras es Sider	1983	CALM
Ras es Sider	1970	CALM	Marsa el Brega	1984	CALM
Ras Lanouf	1970	CALM	Marsa el Brega	1985	CALM
Assawiya	1974	SALM	Bouri	1989	SALM
Assawiya	1974	SALM			

Tunisia

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Ashtart	1972	CALM	Ashtart	1979	CALM
Ashtart	1975	CALM	Tazerka	1980	SALM

Morocco

Location/Name	Installed	Config.	Location/Name	Installed	Config.
El Aaium	1961	CALM	Mohammedia	1971	CALM

France

Location/Name	Installed	Config.
Frontignan	1973	CALM

AFRICA**Ivory Coast**

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Bouet	1980	CALM	Espoir	1982	CALM

Ghana

Location/Name	Installed	Config.
Saltpond	1979	SALM

Equitorial Guinea

Location/Name	Installed	Config.
Bata	1963	CALM

Nigeria

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Apapa	1967	CALM	North Apoi Field	1975	CALM
Escravos	1968	CALM	Brass River	1976	CALM
Forcados	1969	CALM	North Apoi Field	1977	CALM
Forcados	1969	CALM	Escravos	1977	CALM
Escravos	1970	CALM	Que Iboe	1979	CALM
Que Iboe	1971	CALM	Antan	1986	Tower
Forcados	1972	CALM	Forcados	1986	CALM
Brass River	1972	CALM	Brass River	1987	CALM
Bonny	1973	CALM	Que Iboe	1989	CALM
Bonny	1973	CALM	Forcados	1990	CALM
North Apoi Field	1975	CALM			

Cameroon

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Kole	1977	CALM	Kole	1981	CALM
Limboh Point	1979	CALM	Victoria	1982	CALM
Rio del Ray	1980	CALM			

Gabon

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Gamba	1965	CALM	Inguessi	1983	CALM
Lucina	1967	CALM	Mayumba	1983	CALM
Gamba	1969	CALM	Gamba	1989	CALM
Mayumba	1980	CALM			

Congo

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Djeno	1973	CALM	Yombo	1990	CALM

Zaire

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Moanda	1975	CALM	Moanda	1989	CALM
Moanda	1976	CALM			

Angola

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Cabinda	1967	CALM	Palanca	1984	CALM
Essungo	1980	CALM	Takula	1985	CALM
Takula	1980	CALM	Takula	1988	CALM
Takula	1980	CALM	Takula	1989	CALM

South Africa

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Durban	1970	CALM	Durban	1974	CALM

Tanzania

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Dar es Salaam	1972	CALM	Dar es Salaam	1983	CALM

Sudan

Location/Name	Installed	Config.
Marsa Nimeini	1983	CALM

MIDDLE EAST**Yemen Arab Republic**

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Saleef	1987	CALM	Saleef	1988	CALM

South Yemen

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Bir Ali	1989	CALM	Saleh Field	1983	CALM

UAE**Oman**

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Mina al Fahal	1967	CALM	Mina al Fahal	1974	CALM
Mina al Fahal	1967	CALM	Mina al Fahal	1979	CALM
Mina al Fahal	1967	CALM	Mina al Fahal	1984	CALM

Sharjah

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Mubarek	1973	CALM	Mubarek	1987	CALM
Sharjah	1981	CALM			

Dubai

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Fateh	1969	CALM	Fateh	1979	CALM
Fateh	1972	CALM	Fateh	1984	CALM

Abu Dubai

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Das Island	1972	CALM	Arzanah	1977	CALM
Mubarras	1972	CALM	Abu Al Bu Koosh	1981	CALM
Abu Al Bu Koosh	1974	CALM	Zakum	1981	CALM
Das Island	1977	CALM	Zakum	1981	CALM

Qatar

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Halul	1964	CALM	Doba	1976	CALM
Halul	1965	CALM	Halul	1980	CALM
Halul	1972	CALM	Umm Said	1983	CALM
Umm Said	1972	CALM			

Saudi Arabia

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Zuluf	1972	CALM	Ju'Aymah	1974	CALM
Zuluf	1972	CALM	Ju'Aymah	1976	SALM
Ju'Aymah	1974	CALM	Ju'Aymah	1976	SALM
Ju'Aymah	1974	CALM	Asir	1985	CALM

Neutral Zone

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Ras al Khafji	1967	CALM	Ras al Khafji	1980	CALM
Ras al Khafji	1972	CALM	Ras al Khafji	1987	CALM

Iraq

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Khor al Amaya	1980	CALM	Khor al Amaya	1980	CALM
Khor al Amaya	1980	CALM	Khor al Amaya	1980	CALM

Iran

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Cyrus	1970	CALM	Ganaveh	1986	CALM
Iman Hasan	1971	CALM	Bandar Kangan	1987	CALM
Ganaveh	1986	CALM	Bandar Kangan	1987	CALM
Ganaveh	1986	CALM	Bandar Kangan	1987	CALM
Ganaveh	1986	CALM	Bandar Taberi	1989	CALM

Kuwait

Location/Name	Installed	Config.
Mina al Ahmadi	1980	Tower

INDIAN SUBCONTINENT**India**

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Bombay High	1975	CALM	Gulf of Kutch	1984	CALM
Bombay High	1975	CALM	Panna	1985	CALM
Gulf of Kutch	1977	CALM	Bombay High D18	1989	CALM
Ralangiri R12	1982	CALM	Hazira	1990	CALM
Bombay High S11	1983	CALM			

Sri Lanka

Location/Name	Installed	Config.
Colombo Harbor	1986	CALM

Bangladesh

Location/Name	Installed	Config.
Chittagong	1967	CALM

FAR EAST**Singapore**

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Singapore Harbour	1971	CALM	Singapore Harbour	1980	CALM
Pulau Bukom	1974	CALM			

Malaysia

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Miri	1960	CALM	Binulu	1978	CALM
Miri	1960	CALM	Trengganu	1981	SALM
Port Dickson	1963	CALM	Trengganu	1981	SALM
Miri	1964	CALM	Korteh	1982	CALM
Miri	1964	CALM	Port Dickson	1983	CALM
Tembungo	1974	SALM	Miri	1984	CALM
Labuan	1974	CALM	Miri	1984	CALM
Pulai	1974	SALM	Miri	1985	CALM
Pulai	1977	SALM	Miri	1985	CALM

Brunei

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Seria	1971	CALM	Seria	1981	CALM
Seria	1975	CALM			

Vietnam

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Tan My	1969	CALM	White Tiger	1989	CALM
Da Nang	1969	CALM	White Tiger	1990	CALM
White Tiger	1986	CALM			

China

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Hainan	1984	CALM	Bozhong 34-2/4E	1990	Tower
Liuhoa	1987	CALM	Huizhou 16/08	1990	CALM
Bozhong 28-1	1988	Tower	Lufeng 17/16	1990	CALM

Taiwan

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Tai Chung	1967	CALM	Sha-Lung	1974	CALM
Tai Chung	1968	CALM	Sha-Lung	1976	SALM
Kaohsiung	1968	CALM	Sha-Lung	1979	CALM
Kaohsiung	1972	CALM	Kaohsiung	1981	CALM
Kaohsiung	1972	CALM			

Korea

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Ulsan	1963	CALM	Pusan	1979	CALM
Ulsan	1969	CALM	Jiseapo	1984	CALM
Yosu	1969	CALM	Daesan	1987	CALM
Onsan	1978	CALM	Daesan	1989	CALM

Japan

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Niigata	1961	CALM	Toyama	1969	CALM
Oita	1963	CALM	Ube Onoda	1970	CALM
Yokkaichi	1964	CALM	Atsumi	1970	CALM
Yokkaichi	1964	CALM	Himeji	1970	CALM
Chiba	1965	CALM	Yokkaichi	1971	CALM
Koshiha	1967	CALM	Onoda	1972	CALM
Yokkaichi	1968	CALM	Kawasaki	1974	CALM
Kawasaki	1968	CALM	Ogishima	1974	CALM
Hakodate	1968	CALM	Yokkaichi	1976	CALM
Hakozaki	1968	CALM	Mutsu Ogawara	1983	SALM
Ogishima	1969	CALM	Fukui	1984	SALM
Yokohama	1969	CALM			

Okinawa

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Buckner Bay	1970	CALM	Nakagusuki Bay	1971	SALM
Tengan	1970	CALM			

Phillipines

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Subie Bay	1967	CALM	Bataan	1980	SALM
Nido	1977	CALM	Nido	1982	CALM
Cadlao	1980	CALM			

Indonesia

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Pangkalan Susu	1970	CALM	Cinta	1981	CALM
Ardjuna	1971	CALM	Ardjuna	1981	CALM
Java Sea	1971	CALM	Balikpapan	1981	CALM
Balikpapan	1971	CALM	Krisna	1983	CALM
Java Sea	1972	CALM	Balongan	1983	CALM
Ardjuna	1972	CALM	Lalang	1984	Tower
Djati Barang	1973	CALM	Chengkareng Air	1984	CALM
Bekapai	1974	CALM	Kakup	1984	CALM
Ardjuna	1974	CALM	Arun	1984	CALM
Ardjuna	1974	CALM	Bima	1985	CALM
Poleng	1975	CALM	Bima	1985	CALM
Ardjuna	1975	CALM	Bima	1985	CALM
Handii	1976	CALM	Mdura Island	1985	CALM
Balongan	1977	CALM	Intan	1989	CALM
Udang	1978	CALM	Anoa	1990	CALM
Semarang	1980	CALM			

Thailand

Location/Name	Installed	Config.
Erawan	1981	CALM

AUSTRILASIA**Australia**

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Botany Bay	1971	CALM	Jabiru	1989	CALM
Talisman	1989	CALM	Challis	1989	SALM

New Zealand

Location/Name	Installed	Config.	Location/Name	Installed	Config.
Waipipi Point	1971	CALM	Taharoa	1976	CALM
Taharoa	1972	CALM			

New Caledonia

Location/Name	Installed	Config.
Mounea	1977	CALM

Appendix 2 : Floor Slopes at Designated SPMS Sites

Floor slopes were evaluated at six locations for each site specified by the California State Lands Commission [NOAA, 1991]. These slopes are given in the tables below.

Location No.	Location (deg-min N/ deg-min W)	Slope (degrees)	Description
1	33-48 / 118-30	12.8	6.3 miles WNW of Flat Rock Pt
2	33-50 / 118-35	6.8	11.0 miles WNW of Flat Rock Pt
3	33-52 / 118-38	6.4	13.9 miles WNW of Flat Rock Pt
4	33-55 / 118-36	5.4	11.8 miles ESE of Pt Dume
5	33-56 / 118-39	2.2	10.3 miles ESE of Pt Dume
6	33-57 / 118-40	2.5	8.8 miles ESE of Pt Dume

Table A.2.1 : Ocean Floor Slopes at El Segundo

Location No.	Location (deg-min N/ deg-min W)	Slope (degrees)	Description
1	35-10 / 121-0	1.6	7.8 miles SW of Pt Buchon
2	35-13 / 121-1	1.7	7.5 miles WSW of Pt Buchon
3	35-19 / 121-5	1.1	12 miles WNW of Pt Buchon
4	35-23 / 121-7	0.9	9.2 miles SW of Pt Estero
5	35-26 / 121-11	1.1	9.9 miles W of Pt Estero
6	35-31 / 121-13	1.9	8.5 miles S of San Simeon Pt

Table A.2.2 : Ocean Floor Slopes at Morro Bay

Appendix 3 : Calculations of Steady Forces

Steady forces were calculated based on the relations described in Chapter 4, with the environmental conditions described in Chapter 2. The calculations of steady forces were carried out using Microsoft Excel spreadsheets, which are reproduced here. Calculations for the CALM and the SALM are given for three return periods : the 2-year, the 10-year and the 100-year.

Steady Force Evaluation for CALM, 2-year return period conditions

WIND FORCES

Velocity @ 10m	39 knots
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Wind Speed by Centroid Elevation

	Full - Load Tanker		Lightship Tanker		Buoy
	Super	Hull	Super	Hull	CALM
Centroid Elevation (feet)	39	9	50	20	5
Velocity @ Centroid (knots)	39.85	33.18	41.11	36.66	30.83

Wind Speed by Gust Duration

	Full - Load Tanker		Lightship Tanker		Buoy
	Super	Hull	Super	Hull	CALM
Gust Duration (seconds)	180	180	180	180	180
1-minute Velocity (knots)	39.85	33.18	41.11	36.66	30.83
Gust Factor	0.94	0.94	0.93	0.94	0.94
Speed, gust @ centroid	37.27	31.14	38.42	34.34	28.97

Element Dimensions

(All areas in feet squared)

Element	A	Bs	Bh	B	Cx	Vk, hull	Vk, sup
	long. area above WL	trans area superstruct	trans area hull	total trans area	shape coefficient	wind speed knots	speed knots
Full Load	0	5500	2988	6396.4	1.0	31.14	37.27
Lightship	0	5500	6640	7492	1.0	34.34	38.42
CALM	NA	NA	600	600	1.0	28.97	NA

Air density :	0.002 slugs/ft ³
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Steady Wind Forces

Bow-on Wind Only

Element	Force, kips
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Total Wind Forces

Element	Force, kips
CALM alone	1.70

Full Load Tanker Hull	9.79	Lightship, CALM	55.61
Lightship Tanker Hull	26.47	Full-load, CALM	37.31
Full Load Super	25.82		
Lightship Super	27.44		
CALM	1.70		

CURRENT FORCES

Surface Current (knots)	1.2
Surface Current (feet/second)	2.0268

Tanker

	Dimensions (ft ^2)					Bow-on	Beam-on
	Displacement long tons	Length feet	Cws	D vol., ft^3	S ft^2	Force kip	Force kip
Full Load	188500	915	2.7	7E+06	209780	4.8333	120.83
Lightship	30000	915	2.7	1E+06	83689	1.9282	48.205

Buoy

	Cd	Draft feet	Diameter feet	Vcurrent ft/sec	Frontal area ft^2	Force kip
CALM	1	15	60	2.0268	900	3.6971

Riser

Cd	Water Depth feet	Diameter feet	Vsurface ft/sec	Vcentroid ft/sec	Frontal area ft^2	Force kip
0.9	1000	3	2.0268	1.01	3000	2.7728

Total Current Forces

Element	Force, kips
CALM alone	6.47
Lightship, CALM	8.40
Full-load, CALM	11.30

AVERAGE DRIFT LOAD

Significant Wave Height (feet)	14.5	Period (seconds)	12
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Tanker

	Length	Beam	Draft	Char wave period	Cd	FORCE kips
Full load, bow wave	915	166	59.3	10.805	0.05	20.422
Full load, cross wave	915	166	59.3	10.805	0.05	157.09
Lightship, bow wave	915	166	20	9.1929	0.05	20.422
Lightship, cross wave	915	166	20	9.1929	0.05	157.09

Buoy, Riser

	Cd	Diameter	FORCE
CALM	1175	60	12.006
Riser	1175	3	0.03

Total Wave Drift Forces

Element	Force, kips
CALM alone	12.04
Lightship, CALM	32.46
Full-load, CALM	32.46

TOTAL STEADY ENVIRONMENTAL FORCES

Element	Force, kips	Force, LT
CALM alone	20.21	9.02
Lightship, CALM	96.46	43.06
Full-load, CALM	81.07	36.19

Steady Force Evaluation for CALM, 10-year return period conditions

WIND FORCES

Velocity @ 10m	55 knots
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Wind Speed by Centroid Elevation

	Full - Load Tanker		Lightship Tanker		Buoy
	Super	Hull	Super	Hull	CALM
Centroid Elevation (feet)	39	9	50	20	5
Velocity @ Centroid (knots)	56.20	46.79	57.98	51.70	43.48

Wind Speed by Gust Duration

	Full - Load Tanker		Lightship Tanker		Buoy
	Super	Hull	Super	Hull	CALM
Gust Duration (seconds)	180	180	180	180	180
1-minute Velocity (knots)	56.20	46.79	57.98	51.70	43.48
Gust Factor	0.93	0.93	0.93	0.93	0.93
Speed, gust @ centroid	52.12	43.59	53.72	48.05	40.58

Element Dimensions

(All areas in feet squared)

Element	A	Bs	Bh	B	Cx	Vk, hull	Vk, sup
	long. area above WL	trans area superstruct	trans area hull	total trans area	shape coefficient	wind speed knots	speed knots
Full Load	0	5500	2988	6396.4	1.0	43.59	52.12
Lightship	0	5500	6640	7492	1.0	48.05	53.72
CALM	NA	NA	600	600	1.0	40.58	NA

Air density	0.002 slugs/ft ³
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Steady Wind Forces

Bow-on Wind Only

Element	Force, kips
Full Load Tanker Hull	19.19
Lightship Tanker Hull	51.82
Full Load Super	50.50
Lightship Super	53.64
CALM	3.34

Total Wind Forces

Element	Force, kips
CALM alone	3.34
Lightship, CALM	108.80
Full-load, CALM	73.03

CURRENT FORCES

Surface Current (knots)	1.7
Surface Current (feet/second)	2.8713

Tanker

	Dimensions (ft ^2)					Bow-on	Beam-on
	Displacement long tons	Length feet	Cws	D vol., ft^3	S ft^2	Force kip	Force kip
Full Load	188500	915	2.7	6597500	209780	9.7002	242.51
Lightship	30000	915	2.7	1050000	83689	3.8698	96.745

Buoy

	Cd	Draft feet	Diameter feet	Vcurrent ft/sec	Frontal area ft^2	Force kip
CALM	1	15	60	2.8713	900	7.4199

Riser

Cd	Water Depth feet	Diameter feet	Vsurface ft/sec	Vcentroid ft/sec	Frontal area ft^2	Force kip
0.9	1000	4	2.8713	1.44	4000	7.4199

Total Current Forces

Element	Force, kips
CALM alone	14.84
Lightship, CALM	18.71
Full-load, CALM	24.54

AVERAGE DRIFT LOAD

Significant Wave Height (feet)	18.5	Period (seconds)	13
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Tanker

	Length	Beam	Draft	Char wave period	Cd	FORCE kips
Full load, bow wave	915	166	59.3	10.805	0.05	33.244
Full load, cross wave	915	166	59.3	10.805	0.05	255.72
Lightship, bow wave	915	166	20	9.1929	0.05	33.244
Lightship, cross wave	915	166	20	9.1929	0.05	255.72

Buoy, Riser

	Cd	Diameter	FORCE
CALM	1175	60	16.653
Riser	1175	4	0.074

Total Wave Drift Forces

Element	Force, kips
CALM alone	16.73
Lightship, CALM	49.97
Full-load, CALM	49.97

TOTAL STEADY ENVIRONMENTAL FORCES

Element	Force, kips	Force, LT
CALM alone	34.91	15.58
Lightship, CALM	177.48	79.23
Full-load, CALM	147.54	65.87

Steady Force Evaluation for CALM, 100-year return period conditions

WIND FORCES

Velocity @ 10m	72 knots
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Wind Speed by Centroid Elevation

	Full - Load Tanker		Lightship Tanker		Buoy
	Super	Hull	Super	Hull	CALM
Centroid Elevation (feet)	39	9	50	20	5
Velocity @ Centroid (knots)	73.58	61.25	75.90	67.68	56.91

Wind Speed by Gust Duration

	Full - Load Tanker		Lightship Tanker		Buoy
	Super	Hull	Super	Hull	CALM
Gust Duration (seconds)	180	180	180	180	180
1-minute Velocity (knots)	73.58	61.25	75.90	67.68	56.91
Gust Factor	0.92	0.93	0.92	0.92	0.93
Speed, gust @ centroid	67.68	56.67	69.75	62.43	52.76

Element Dimensions

(All areas in feet squared)

Element	A	Bs	Bh	B	Cx	Vk, hull	Vk, sup
	long. area above WL	trans area superstruct	trans area hull	total trans area	shape coefficient	wind speed knots	speed knots
Full Load	0	5500	2988	6396.4	1.0	56.67	67.68
Lightship	0	5500	6640	7492	1.0	62.43	69.75
CALM	NA	NA	600	600	1.0	52.76	NA

Air density	0.002 slugs/ft ³
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Steady Wind Forces

Bow-on Wind Only

Element	Force, kips
Full Load Tanker Hull	32.43
Lightship Tanker Hull	87.47
Full Load Super	85.16
Lightship Super	90.44
CALM	5.65

Total Wind Forces

Element	Force, kips
CALM alone	5.65
Lightship, CALM	183.55
Full-load, CALM	123.24

CURRENT FORCES

Surface Current (knots)	2.2
Surface Current (feet/second)	3.7158

Tanker

	Dimensions (ft ^2)					Bow-on	Beam-on
	Displacement long tons	Length feet	Cws	D vol., ft^3	S ft^2	Force kip	Force kip
Full Load	188500	915	2.7	7E+06	209780	16.245	406.13
Lightship	30000	915	2.7	1E+06	83689	6.4809	162.02

Buoy

	Cd	Draft feet	Diameter feet	Vcurrent ft/sec	Frontal area ft^2	Force kip
CALM	1	15	60	3.7158	900	12.426

Riser

Cd	Water Depth feet	Diameter feet	Vsurface ft/sec	Vcentroid ft/sec	Frontal area ft^2	Force kip
0.9	1000	4	3.7158	1.86	4000	12.426

Total Current Forces

Element	Force, kips
CALM alone	24.85
Lightship, CALM	31.33
Full-load, CALM	41.10

AVERAGE DRIFT LOAD

Significant Wave Height (feet)	23	Period (seconds)	14
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Tanker

	Length	Beam	Draft	Char wave period	Cd	FORCE kips
Full load, bow wave	915	166	59.3	10.805	0.05	51.383
Full load, cross wave	915	166	59.3	10.805	0.05	395.25
Lightship, bow wave	915	166	20	9.1929	0.05	51.383
Lightship, cross wave	915	166	20	9.1929	0.05	395.25

Buoy, Riser

	Cd	Diameter	FORCE
CALM	1175	60	22.194
Riser	1175	4	0.0986

Total Wave Drift Forces

Element	Force, kps
CALM alone	22.29
Lightship, CALM	73.68
Full-load, CALM	73.68

TOTAL STEADY ENVIRONMENTAL FORCES

Element	Force, kps	Force, LI
CALM alone	52.79	23.57
Lightship, CALM	288.56	128.82
Full-load, CALM	238.01	106.26

Steady Force Evaluation for SALM, 2-year return period conditions

WIND FORCES

Velocity @ 10m	39 knots
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Wind Speed by Centroid Elevation

	Full - Load Tanker		Lightship Tanker		Buoy
	Super	Hull	Super	Hull	SALM
Centroid Elevation (feet)	39	9	50	20	10
Velocity @ Centroid (knots)	39.85	33.18	41.11	36.66	33.62

Wind Speed by Gust Duration

	Full - Load Tanker		Lightship Tanker		Buoy
	Super	Hull	Super	Hull	SALM
Gust Duration (seconds)	180	180	180	180	180
1-minute Velocity (knots)	39.85	33.18	41.11	36.66	33.62
Gust Factor	0.94	0.94	0.93	0.94	0.94
Speed, gust, centroid, kts	37.27	31.14	38.42	34.34	31.54

Element Dimensions

(All areas in feet squared)

Element	A	Bs	Bh	B	Cx	Vk, hull	Vk, sup
	long. area above WL	trans area superstruct	trans area hull	total trans area	shape coefficient	wind speed knots	speed knots
Full Load	0	5500	2988	6396.4	1.0	31.14	37.27
Lightship	0	5500	6640	7492	1.0	34.34	38.42
SALM	NA	NA	400	400	1.0	31.54	NA

Air density :	0.002 slugs/ft ³
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Steady Wind Forces

Bow-on Wind Only

Element	Force, kips
Full Load Tanker Hull	9.79
Lightship Tanker Hull	26.47
Full Load Super	25.82
Lightship Super	27.44
SALM	1.35

Total Wind Forces

Element	Force, kips
SALM alone	1.35
Lightship, SALM	55.25
Full-load, SALM	36.96

CURRENT FORCES

Surface Current (knots)	1.2
Surface Current (feet/second)	2.0268

Tanker

	Displacement long tons	Dimensions (ft ^2)				Bow-on	Beam-on
		Length feet	Cws	D vol., ft^3	S ft^2	Force kip	Force kip
Full Load	188500	915	2.7	7E+06	209780	4.8333	120.83
Lightship	30000	915	2.7	1E+06	83689	1.9282	48.205

Buoy

	Cd	Draft feet	Diameter feet	Vcurrent ft/sec	Frontal area ft^2	Force kip
SALM	1	43	15	2.0268	645	2.6496

Riser

Cd	Water Depth feet	Diameter feet	Vsurface ft/sec	Vcentroid ft/sec	Frontal area ft^2	Force kip
0.9	1000	5	2.0268	1.01	5000	4.6214

Total Current Forces

Element	Force, kips
SALM alone	7.27
Lightship, SALM	9.20
Full-load, SALM	12.10

AVERAGE DRIFT LOAD

Significant Wave Height (feet)	14.5	Period (seconds)	12
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Tanker

	Length	Beam	Draft	Char wave period	Cd	FORCE kips
Full load, bow wave	915	166	59.3	10.805	0.05	20.422
Full load, cross wave	915	166	59.3	10.805	0.05	157.09
Lightship, bow wave	915	166	20	9.1929	0.05	20.422
Lightship, cross wave	915	166	20	9.1929	0.05	157.09

Buoy, Riser

	Cd	Diameter	FORCE
SALM	1175	15	0.7504
Riser	1175	5	0.0834

Total Wave Drift Forces

Element	Force, kp
SALM alone	0.83
Lightship, SALM	21.26
Full-load, SALM	21.26

TOTAL STEADY ENVIRONMENTAL FORCES

Element	Force, kp	Force, LI
SALM alone	9.45	4.22
Lightship, SALM	85.70	38.26
Full-load, SALM	70.32	31.39

Steady Force Evaluation for SALM, 10-year return period conditions

WIND FORCES

Velocity @ 10m	55 knots
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Wind Speed by Centroid Elevation

	Full - Load Tanker		Lightship Tanker		Buoy
	Super	Hull	Super	Hull	SALM
Centroid Elevation (feet)	39	9	50	20	10
Velocity @ Centroid (knots)	56.20	46.79	57.98	51.70	47.41

Wind Speed by Gust Duration

	Full - Load Tanker		Lightship Tanker		Buoy
	Super	Hull	Super	Hull	SALM
Gust Duration (seconds)	180	180	180	180	180
1-minute Velocity (knots)	56.20	46.79	57.98	51.70	47.41
Gust Factor	0.93	0.93	0.93	0.93	0.93
Speed, gust, centroid, kts	52.12	43.59	53.72	48.05	44.16

Element Dimensions

(All areas in feet squared)

Element	A	Bs	Bh	B	Cx	Vk, hull	Vk, sup
	long. area above WL	trans area superstruct	trans area hull	total trans area	shape coefficient	wind speed knots	speed knots
Full Load	0	5500	2988	6396.4	1.0	43.59	52.12
Lightship	0	5500	6640	7492	1.0	48.05	53.72
SALM	NA	NA	400	400	1.0	44.16	NA

Air density	0.002 slugs/ft ³
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Steady Wind Forces

Bow-on Wind Only

Element	Force, kips
Full Load Tanker Hull	19.19
Lightship Tanker Hull	51.82
Full Load Super	50.50
Lightship Super	53.64
SALM	2.64

Total Wind Forces

Element	Force, kips
SALM alone	2.64
Lightship, SALM	108.10
Full-load, SALM	72.33

CURRENT FORCES

Surface Current (knots)	1.7
Surface Current (feet/second)	2.8713

Tanker

	Dimensions (ft ^2)					Bow-on	Beam-on
	Displacement long tons	Length feet	Cws	D vol., ft^3	S ft^2	Force kip	Force kip
Full Load	188500	915	2.7	7E+06	209780	9.7002	242.51
Lightship	30000	915	2.7	1E+06	83689	3.8698	96.745

Buoy

	Cd	Draft feet	Diameter feet	Vcurrent ft/sec	Frontal area ft^2	Force kip
SALM	1	43	15	2.8713	645	5.3176

Riser

Cd	Water Depth feet	Diameter feet	Vsurface ft/sec	Vcentroid ft/sec	Frontal area ft^2	Force kip
0.9	1000	5	2.8713	1.44	5000	9.2749

Total Current Forces

Element	Force, kips
SALM alone	14.59
Lightship, SALM	18.46
Full-load, SALM	24.29

AVERAGE DRIFT LOAD

Significant Wave Height (feet)	18.5	Period (seconds)	13
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Tanker

	Length	Beam	Draft	Char wave period	Cd	FORCE kips
Full load, bow wave	915	166	59.3	10.805	0.05	33.244
Full load, cross wave	915	166	59.3	10.805	0.05	255.72
Lightship, bow wave	915	166	20	9.1929	0.05	33.244
Lightship, cross wave	915	166	20	9.1929	0.05	255.72

Buoy, Riser

	Cd	Diameter	FORCE
SALM	1175	15	1.0408
Riser	1175	5	0.1156

Total Wave Drift Forces

Element	Force, kp
SALM alone	1.16
Lightship, SALM	34.40
Full-load, SALM	34.40

TOTAL STEADY ENVIRONMENTAL FORCES

Element	Force, kp	Force, LT
SALM alone	18.39	8.21
Lightship, SALM	160.96	71.86
Full-load, SALM	131.02	58.49

WIND FORCES

Velocity @ 10m	72 knots
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Wind Speed by Centroid Elevation

	Full - Load Tanker		Lightship Tanker		Buoy
	Super	Hull	Super	Hull	SALM
Centroid Elevation (feet)	39	9	50	20	10
Velocity @ Centroid (knots)	73.58	61.25	75.90	67.68	62.07

Wind Speed by Gust Duration

	Full - Load Tanker		Lightship Tanker		Buoy
	Super	Hull	Super	Hull	SALM
Gust Duration (seconds)	180	180	180	180	180
1-minute Velocity (knots)	73.58	61.25	75.90	67.68	62.07
Gust Factor	0.92	0.93	0.92	0.92	0.92
Speed, gust, centroid, kts	67.68	56.67	69.75	62.43	57.40

Element Dimensions

(All areas in feet squared)

Element	A	Bs	Bh	B	Cx	Vk, hull	Vk, sup
	long. area above WL	trans area superstruct	trans area hull	total trans area	shape coefficient	wind speed knots	speed knots
Full Load	0	5500	2988	6396.4	1.0	56.67	67.68
Lightship	0	5500	6640	7492	1.0	62.43	69.75
SALM	NA	NA	400	400	1.0	57.40	NA

Air density	0.002 slugs/ft ³
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Steady Wind Forces

Bow-on Wind Only

Element	Force, kips
Full Load Tanker Hull	32.43
Lightship Tanker Hull	87.47
Full Load Super	85.16
Lightship Super	90.44
SALM	4.45

Total Wind Forces

Element	Force, kips
SALM alone	4.45
Lightship, SALM	182.36
Full-load, SALM	122.05

CURRENT FORCES

Surface Current (knots)	2.2
Surface Current (feet/second)	3.7158

Tanker

	Dimensions (ft ^2)					Bow-on	Beam-on
	Displacement long tons	Length feet	Cws	D vol., ft^3	S ft^2	Force kip	Force kip
Full Load	188500	915	2.7	7E+06	209780	16.245	406.13
Lightship	30000	915	2.7	1E+06	83689	6.4809	162.02

Buoy

	Cd	Draft feet	Diameter feet	Vcurrent ft/sec	Frontal area ft^2	Force kip
SALM	1	43	15	3.7158	645	8.9056

Riser

Cd	Water Depth feet	Diameter feet	Vsurface ft/sec	Vcentroid ft/sec	Frontal area ft^2	Force kip
0.9	1000	5	3.7158	1.86	5000	15.533

Total Current Forces

Element	Force, kips
SALM alone	24.44
Lightship, SALM	30.92
Full-load, SALM	40.68

AVERAGE DRIET LOAD

Significant Wave Height (feet)	2.3	Period (seconds)	14
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Tanker

	Length	Beam	Draft	Char wave period	Cd	FORCE kips
Full load, bow wave	915	166	59.3	10.805	0.05	51.383
Full load, cross wave	915	166	59.3	10.805	0.05	395.25
Lightship, bow wave	915	166	20	9.1929	0.05	51.383
Lightship, cross wave	915	166	20	9.1929	0.05	395.25

Buoy, Riser

	Cd	Diameter	FORCE
SALM	1175	15	1.3871
Riser	1175	5	0.1541

Total Wave Drift Forces

Element	Force, kp
SALM alone	1.54
Lightship, SALM	52.92
Full-load, SALM	52.92

TOTAL STEADY ENVIRONMENTAL FORCES

Element	Force, kp	Force, LT
SALM alone	30.43	13.59
Lightship, SALM	266.20	118.84
Full-load, SALM	215.66	96.28

Appendix 4 : Calculations of Oscillating Motions

Two types of oscillating motions were investigated in this study : low frequency and wave frequency oscillations. Low frequency oscillations were based on the methods recommended by the American Petroleum Institute [API, 1987], while wave frequency motions were calculated with the use of the ship motions program, SEAWAY [Journée, 1992], as described in section 4.3.

Calculation of low frequency motions was carried out using Microsoft Excel spreadsheets, which are given below. For further information on this analysis, the reader is referred to API RP 2P.

Low Frequency Motions

Summary rms amplitudes

	Low Frequency		
	2-year	10-year	100-year
Full load tanker	2.87	4.30	5.56
Lightship tanker	2.87	4.30	5.56
SALM	8.61	9.50	10.04
CALM	11.38	12.57	13.28

Summary max amplitudes

	Low Frequency		
	2-year	10-year	100-year
Full load tanker	6.13	9.20	11.88
Lightship tanker	7.26	10.89	14.07
SALM	28.60	31.57	33.36
CALM	37.83	41.77	44.13

Low Frequency Vessel Motions

	Full Load Tanker			Lightship Tanker		
	2-year	10-year	100-year	2-year	10-year	100-year
Stiffness (kips/ft)	1.4	1.4	1.4	1.4	1.4	1.4
Actual Length (ft)	915	915	915	915	915	915
Displacement (LT)	188500	188500	188500	30000	30000	30000
Reference Length(ft)	540	540	540	540	540	540
Sig. Wave Height (ft)	14.5	18.5	23	14.5	18.5	23
Ref. Sig. W.H. (ft)	8.56	10.92	13.57	8.56	10.92	13.57
<i>Reference surge/sway, rms</i>						
Xs, bow seas :	0.8	1.2	1.55	0.8	1.2	1.55
Xs, quarter seas :	1	1.3	1.7	1	1.3	1.7
Ys, quarter seas :	1.55	2	2.4	1.55	2	2.4
Ys, beam seas :	2.15	2.9	3.3	2.15	2.9	3.3
<i>Actual surge/sway, rms</i>						
Xs, bow seas :	2.87	4.30	5.56	2.87	4.30	5.56
Xs, quarter seas :	3.59	4.66	6.10	3.59	4.66	6.10
Ys, quarter seas :	5.56	7.17	8.61	5.56	7.17	8.61
Ys, beam seas :	7.71	10.40	11.83	7.71	10.40	11.83
<i>Actual surge/sway, sig. single amplitude</i>						
Xs, bow seas :	5.74	8.61	11.12	5.74	8.61	11.12
Xs, quarter seas :	7.17	9.32	12.19	7.17	9.32	12.19
Ys, quarter seas :	11.12	14.34	17.21	11.12	14.34	17.21
Ys, beam seas :	15.42	20.80	23.67	15.42	20.80	23.67
Natural Per., vessel	1098	1098	1098	438	438	438
Rayleigh Factor	1.07	1.07	1.07	1.27	1.27	1.27
<i>Actual surge/sway, max. single amplitude</i>						
Xs, bow seas :	6.13	9.20	11.88	7.26	10.89	14.07
Xs, quarter seas :	7.67	9.97	13.03	9.08	11.80	15.43
Ys, quarter seas :	11.88	15.33	18.40	14.07	18.16	21.79
Ys, beam seas :	16.48	22.23	25.30	19.52	26.33	29.96

	SALM			CALM		
	2-year	10-year	100-year	2-year	10-year	100-year
Stiffness (kips/ft) :	1.4	1.4	1.4	0.8	0.8	0.8
Displacement (LT)	292	292	292	400	400	400
Sig. Wave Height (ft)	14.5	18.5	23	14.5	18.5	23
<i>Reference surge/sway, rms</i>						
Xs, bow seas :	2.4	2.65	2.8	2.4	2.65	2.8
Xs, quarter seas :	1.25	1.35	1.4	1.25	1.35	1.4
Ys, beam seas :	2.2	2.7	3.1	2.2	2.7	3.1
<i>Actual surge/sway, rms</i>						
Xs, bow seas :	8.61	9.50	10.04	11.38	12.57	13.28
Xs, quarter seas :	4.48	4.84	5.02	5.93	6.40	6.64
Ys, beam seas :	7.89	9.68	11.12	10.44	12.81	14.70
<i>Actual surge/sway, sig. single amplitude</i>						
Xs, bow seas :	17.21	19.00	20.08	22.77	25.14	26.56
Xs, quarter seas :	8.96	9.68	10.04	11.86	12.81	13.28
Ys, beam seas :	15.78	19.36	22.23	20.87	25.61	29.41
Natural Per., vessel	43.2	43.2	43.2	66.9	66.9	66.9
Rayleigh Factor	1.66	1.66	1.66	1.59	1.59	1.59
<i>Actual surge/sway, max. single amplitude</i>						
Xs, bow seas :	28.60	31.57	33.36	37.83	41.77	44.13
Xs, quarter seas :	9.58	10.35	10.73	12.68	13.69	14.20
Ys, beam seas :	16.87	20.70	23.77	22.31	27.38	31.44

Wave frequency motions were calculated by the use of the ship motions program SEAWAY. The reader is referred to the SEAWAY manual for a detailed explanation of this program. The program requires two input files : a file describing the hull form of the ship in question, and a file describing the environmental conditions and the ship loading. The program produces one output file, describing wave frequency motions. Input files are given below for the CALM buoy, the SALM buoy, and the San Diego class tanker in light ship and full load conditions. Output files are given for the four input cases.

CALM Hull Input File

4.12

CALM buoy 60.0 x 15.0 dia*draft (18.3*4.57)

4.5700 0.0000 18.3000 0.3050

10

0.6098 0.6098 1.5244 1.5244 4.5732 4.5732 1.5244 1.5244

0.6098 0.6098

1

1.0	4	0.0					
0.00	0.0	1.17	0.0	2.34	0.0	2.34	2.500
2.34	5.00						
2.0	4	0.0					
0.00	0.0	1.99	0.0	3.99	0.0	3.99	2.500
3.99	5.00						
3.0	4	0.0					
0.00	0.0	2.53	0.0	5.06	0.0	5.06	2.500
5.06	5.00						
4.0	4	0.0					
0.00	0.0	3.41	0.0	6.82	0.0	6.82	2.500
6.82	5.00						
5.0	4	0.0					
0.00	0.0	3.96	0.0	7.92	0.0	7.92	2.500
7.92	5.00						
6.0	4	0.0					
0.00	0.0	4.57	0.0	9.15	0.0	9.15	2.500
9.15	5.00						
7.0	4	0.0					
0.00	0.0	3.96	0.0	7.92	0.0	7.92	2.500
7.92	5.00						
8.0	4	0.0					
0.00	0.0	3.41	0.0	6.82	0.0	6.82	2.500
6.82	5.00						
9.0	4	0.0					
0.00	0.0	2.53	0.0	5.06	0.0	5.06	2.500
5.06	5.00						
10.0	4	0.0					
0.00	0.0	1.99	0.0	3.99	0.0	3.99	2.500
3.99	5.00						
11.0	4	0.0					
0.00	0.0	1.17	0.0	2.34	0.0	2.34	2.500

2.34 5.00
 1.0000 1.0000 1.0000
 *** End of file ***

CALM Environment Input File

4.12
 CALM buoy with spring
 +1 +1 0 +1 0
 4.573 0.000 0.000 304.878 1.025E+00
 123456 1 6 +5 0
 1
 0.0
 1
 180.0
 2.500 1 0.200 1.700 0.033333
 1.474
 +6.00 +7.561 4.750 4.750
 0
 3
 5.000
 0.0 61.250 105.000
 0
 1
 9.00 0.0 0.0
 11.7 0.0 0.0
 -1
 9.000 0.000 4.000
 4
 +2
 4.76 12.00
 6.06 13.00
 7.53 14.00
 8.84 15.00
 0
 *** End of file ***

CALM Output File

```
#####
# Program: SEAWAY                               Journee #
#                                               #
# STRIPTHEORY CALCULATIONS OF MOTIONS AND LOADS IN A SEAWAY #
#                                               #
#               Release 4.12                       #
#               (31-07-1993)                       #
#####
```

User: University of California, Berkeley, U.S.A.
 INPUT DATA

 CALM buoy with spring

PRINT-CODE INPUT DATA KPR(1) : 1

PRINT-CODE GEOMETRIC DATA KPR(2) : 1
 PRINT-CODE HYDRODYNAMIC COEFFICIENTS KPR(3) : 0
 PRINT-CODE FREQUENCY CHARACTERISTICS KPR(4) : 1
 PRINT-CODE SPECTRAL DATA KPR(5) : 0

ACTUAL MIDSHIP DRAFT DRAFT : 4.573 m
 ACTUAL TRIM BY STERN TRIM : 0.000 m
 DUMMY VALUE, FOR THE TIME BEING DIST : 0.000

WATER DEPTH DEPTH : 304.9 m
 DENSITY OF WATER RHO : 1.025 ton/m³

DEGREES OF FREEDOM CODE MOT : 123456
 VERSION-CODE OF STRIP THEORY METHOD ... KTH : 1
 NUMBER OF TERMS IN POTENTIAL SERIES .. MSER : 6
 CODE OF USED 2-D APPROXIMATION KCOF : 5
 NUMBER OF "FREE-CHOICE" SECTIONS NFR : 0

NUMBER OF FORWARD SPEEDS NV : 1
 FORWARD SPEEDS (kn) VK(NV) : 0.00

NUMBER OF WAVE DIRECTIONS NWD : 1
 WAVE DIRECTIONS (deg off stern) WAVDIR(NWD) : 180.0

MAX. FREQ. OF ENCOUNTER IN SERIES . FREQMAX : 2.500 rad/sec (range = 0.000 - 3.125 rad/sec)
 CODE FOR WAVE FREQUENCY INPUT KOMEG : 1
 MINIMUM CIRCULAR WAVE FREQUENCY OMMIN : 0.200 rad/sec
 MAXIMUM CIRCULAR WAVE FREQUENCY OMMAX : 1.700 rad/sec
 INCREMENT IN WAVE FREQUENCIES OMINC : 0.033 rad/sec

WAVE AMPLITUDE FOR LINEARISATION ... WAVAMP : 1.474 m

INPUT DATA (continued)

 BASE LINE TO CENTRE OF GRAVITY ... +GKGM=KG : 6.000 m

MASS-GYRADIUS k-xx GYR(1) : 7.561 m
 MASS-GYRADIUS k-yy GYR(2) : 4.750 m
 MASS-GYRADIUS k-zz GYR(3) : 4.750 m

NUMBER OF LOAD-CALCULATION SECTIONS .. NBTM : 0

CODE OF ROLL DAMPING INPUT KRD : 3
 AVERAGE ROLL AMPLITUDE ROLAMP : 5.000 deg
 HEIGHT OF BILGE KEEL HBK : 0.000 m
 DISTANCE OF A.P.P. TO AFT END B.K. ... XBKA : 61.25 m
 DISTANCE OF A.P.P. TO FORWARD END B.K. XBKF : 105.00 m

CODE OF ANTI-ROLLING DEVICES KARD : 0

NUMBER OF LINEAR SPRINGS NCAB : 1

COORDINATES AND LINEAR SPRING COEFFICIENTS : 9.000 0.000 0.000 1.170E+01
0.000E-01 0.000E-01

NUMBER OF DISCRETE POINTS NPTS : -1
COORDINATES OF POINTS (m) .. PTSXYZ(NPTS,3) : 9.00 0.00 4.00

NUMBER OF SEA STATES NSEA : 4
CODE OF IRREGULAR SEA DESCRIPTION KSEA : 2
WAVE HEIGHTS (m) HW(K) / PERIODS (s) TW(K) : 4.76 12.00
6.06 13.00
7.53 14.00
8.84 15.00

INPUT-CODE OF CRITERIA FOR SHIPMOTIONS KRIT : 0

CALM buoy with spring
SEAWAY-4.12

Execution: 18-04-1994 , 18:28

GEOMETRICAL HULLFORM DATA

ACTUAL MIDSHIP DRAFT (T) : 4.573 m
ACTUAL TRIM BY STERN : 0.000 m

LENGTH BETWEEN PERPENDICULARS (Lpp) : 18.300 m
REAR SECTION TO A.P.P. : 0.305 m

WATERLINE : LENGTH (Lwl) : 17.683 m
BEAM (B) : 18.300 m
AREA : 261 m²
AREA COEFFICIENT (Lpp) : 0.7785
AREA COEFFICIENT (Lwl) : 0.8056
CENTROID TO A.P.P. : 8.537 m (-0.613 m or -3.35 % Lpp/2)
CENTROID TO REAR SECTION : 8.842 m (+0.000 m or +0.00 % Lwl/2)

DISPLACEMENT : VOLUME : 1304 m³
BLOCKCOEFFICIENT (Lpp) : 0.8516
BLOCKCOEFFICIENT (Lwl) : 0.8813
CENTROID TO A.P.P. : 8.537 m (-0.613 m or -3.35 % Lpp/2)
CENTROID TO REAR SECTION .. : 8.842 m (+0.000 m or +0.00 % Lwl/2)
CENTROID TO WATERLINE : 2.501 m
CENTROID TO KEELLINE : 2.072 m
MIDSHIP SECTION COEFFICIENT : 1.0913
LONG. PRISMATIC COEFFICIENT : 0.7804
VERT. PRISMATIC COEFFICIENT : 1.0940
RATIO Lpp/B : 1.000
RATIO Lwl/B : 0.966
RATIO B/T : 4.002
WETTED SURFACE HULL : 484 m²

STABILITY PARAMETERS

KB : 2.072 m
 KG : 6.000 m
 BM-TRANSVERSE . : 4.229 m
 GM-TRANSVERSE . : 0.300 m
 BM-LONGITUDINAL : 4.092 m
 GM-LONGITUDINAL : 0.164 m

CALM buoy with spring
 SEAWAY-4.12

Execution: 18-04-1994 , 18:28

SECTIONAL HULLFORM DATA

STATION NUMBER	X-APP (-)	HALF CL-CL (m)	HALF WIDTH (m)	HALF DRAFT (m)	DRAFT (m2)	AREA COEFF (-)	AREA (m)	AREA (m)	KB (m)	BO WETTED LENGTH
1.00	-0.305	0.000	2.340	5.003	23.4130	1.0000	2.072	2.501	14.685	
2.00	0.305	0.000	3.990	5.003	39.9221	1.0000	2.072	2.501	17.985	
3.00	0.915	0.000	5.060	5.003	50.6280	1.0000	2.072	2.501	20.125	
4.00	2.439	0.000	6.820	5.003	68.2378	1.0000	2.072	2.501	23.645	
5.00	3.963	0.000	7.920	5.003	79.2439	1.0000	2.072	2.501	25.845	
6.00	8.537	0.000	9.150	5.003	91.5507	1.0000	2.072	2.501	28.305	
7.00	13.110	0.000	7.920	5.003	79.2439	1.0000	2.072	2.501	25.845	
8.00	14.634	0.000	6.820	5.003	68.2378	1.0000	2.072	2.501	23.645	
9.00	16.159	0.000	5.060	5.003	50.6280	1.0000	2.072	2.501	20.125	
10.00	16.768	0.000	3.990	5.003	39.9221	1.0000	2.072	2.501	17.985	
11.00	17.378	0.000	2.340	5.003	23.4130	1.0000	2.072	2.501	14.685	

CALM buoy with spring
 SEAWAY-4.12

Execution: 18-04-1994 / 18:28

TWO-PARAMETER LEWIS CONFORMAL MAPPING COEFFICIENTS

STATION NUMBER	X-APP (m)	HALF WIDTH (m)	HALF DRAFT (m)	AREA COEFF (-)	M(S) (-)	A(-1) (-)	A(1) (-)	A(3) (-)	RMS	REMARKS
1.00	-0.305	2.340	5.003	1.0000	4.1765	+1.0000	-0.3188	-0.1209	0.138	BULBOUS
2.00	0.305	3.990	5.003	1.0000	5.2190	+1.0000	-0.0970	-0.1384	0.109	TUNNELED-
3.00	0.915	5.060	5.003	1.0000	5.8528	+1.0000	+0.0049	-0.1403	0.113	TUNNELED-
4.00	2.439	6.820	5.003	1.0000	6.8484	+1.0000	+0.1327	-0.1368	0.150	TUNNELED-
5.00	3.963	7.920	5.003	1.0000	7.4505	+1.0000	+0.1958	-0.1327	0.189	TUNNELED
6.00	8.537	9.150	5.003	1.0000	8.1115	+1.0000	+0.2556	-0.1276	0.235	TUNNELED
7.00	13.110	7.920	5.003	1.0000	7.4505	+1.0000	+0.1958	-0.1327	0.189	TUNNELED
8.00	14.634	6.820	5.003	1.0000	6.8484	+1.0000	+0.1327	-0.1368	0.150	TUNNELED-

9.00	16.159	5.060	5.003	1.0000	5.8528	+1.0000	+0.0049	-0.1403	0.113	TUNNELED-
BULBOUS										
10.00	16.768	3.990	5.003	1.0000	5.2190	+1.0000	-0.0970	-0.1384	0.109	TUNNELED-
BULBOUS										
11.00	17.378	2.340	5.003	1.0000	4.1765	+1.0000	-0.3188	-0.1209	0.138	BULBOUS

CALM buoy with spring
SEAWAY-4.12

Execution: 18-04-1994 / 18:28

N-PARAMETER CLOSE-FIT CONFORMAL MAPPING COEFFICIENTS

STATION	M(S)	A(-1)	A(1)	A(3)	A(5)	A(7)	A(9)	A(11)	A(13)	A(15)	A(17)
A(19)	RMS	(-)	(-)	(-)	(-)	(-)	(-)	(-)	(-)	(-)	(m)
1.00	+4.2559	+1.0000	-0.3345	-0.1430	+0.0274	+0.0057	-0.0058	+0.0000	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.014									
2.00	+5.2786	+1.0000	-0.1031	-0.1589	+0.0096	+0.0108	-0.0024	+0.0000	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.025									
3.00	+5.9161	+1.0000	+0.0052	-0.1610	-0.0005	+0.0115	+0.0001	+0.0000	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.032									
4.00	+6.9291	+1.0000	+0.1408	-0.1570	-0.0127	+0.0101	+0.0031	+0.0000	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.028									
5.00	+7.5555	+1.0000	+0.2071	-0.1540	-0.0184	+0.0092	+0.0043	+0.0000	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.025									
6.00	+8.2437	+1.0000	+0.2695	-0.1492	-0.0231	+0.0076	+0.0052	+0.0000	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.026									
7.00	+7.5555	+1.0000	+0.2071	-0.1540	-0.0184	+0.0092	+0.0043	+0.0000	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.025									
8.00	+6.9291	+1.0000	+0.1408	-0.1570	-0.0127	+0.0101	+0.0031	+0.0000	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.028									
9.00	+5.9161	+1.0000	+0.0052	-0.1610	-0.0005	+0.0115	+0.0001	+0.0000	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.032									
10.00	+5.2786	+1.0000	-0.1031	-0.1589	+0.0096	+0.0108	-0.0024	+0.0000	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.025									
11.00	+4.2559	+1.0000	-0.3345	-0.1430	+0.0274	+0.0057	-0.0058	+0.0000	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.014									

CALM buoy with spring
SEAWAY-4.12

Execution: 18-04-1994 / 18:28

NATURAL ROLL AND COEFFICIENTS AT FIXED AMPLITUDE

FORWARD SHIP SPEED (kn) : 0.00
MEAN ROLL AMPLITUDE (deg) : 5.000

NATURAL ROLL PERIOD (s) : 30.749
NATURAL FREQUENCY (r/s) : 0.204

LINEAR EQUIVALENT GM (m) : 0.308

1.200	0.654	1.200	0.446	101.8	0.000	92.5	0.280	340.6	0.000	94.6	2.281	276.1	0.000
175.8	6.34E+00	1.48E+01											
1.233	0.672	1.233	0.420	103.3	0.000	92.7	0.281	346.3	0.000	94.8	2.335	277.2	0.000
175.6	5.61E+00	1.17E+01											
1.267	0.690	1.267	0.394	105.1	0.000	93.1	0.283	350.1	0.000	95.0	2.384	278.2	0.000
175.3	5.08E+00	9.23E+00											
1.300	0.708	1.300	0.369	107.0	0.000	93.4	0.282	352.7	0.000	95.2	2.428	279.0	0.000
175.1	4.70E+00	7.24E+00											
1.333	0.727	1.333	0.344	109.0	0.000	93.7	0.280	354.6	0.000	95.3	2.467	279.7	0.000
174.9	4.40E+00	5.54E+00											
1.367	0.745	1.367	0.320	111.3	0.000	94.1	0.275	355.9	0.000	95.4	2.497	280.4	0.000
174.7	4.17E+00	4.06E+00											
1.400	0.763	1.400	0.297	113.9	0.000	94.5	0.268	356.9	0.000	95.6	2.519	281.0	0.000
174.5	3.99E+00	2.70E+00											
1.433	0.781	1.433	0.276	116.6	0.000	95.0	0.259	357.6	0.000	95.7	2.532	281.4	0.000
174.4	3.84E+00	1.44E+00											
1.467	0.799	1.467	0.255	119.7	0.000	95.5	0.249	358.2	0.000	95.8	2.535	281.8	0.000
174.3	3.72E+00	2.46E-01											
1.500	0.817	1.500	0.235	123.0	0.000	96.1	0.237	358.6	0.000	96.0	2.528	282.1	0.000
174.2	3.59E+00	-8.89E-01											
1.533	0.836	1.533	0.216	126.6	0.000	96.7	0.225	358.9	0.000	96.1	2.510	282.4	0.000
174.1	3.46E+00	-1.96E+00											
1.567	0.854	1.567	0.199	130.6	0.000	97.5	0.212	359.2	0.000	96.3	2.481	282.6	0.000
174.2	3.35E+00	-2.95E+00											
1.600	0.872	1.600	0.183	134.9	0.000	98.3	0.198	359.4	0.000	96.5	2.441	282.7	0.000
174.2	3.22E+00	-3.86E+00											
1.633	0.890	1.633	0.168	139.6	0.000	99.4	0.184	359.5	0.000	96.6	2.391	282.8	0.000
174.3	3.08E+00	-4.67E+00											
1.667	0.908	1.667	0.154	144.7	0.000	100.6	0.170	359.6	0.000	96.8	2.330	282.9	0.000
174.4	2.94E+00	-5.37E+00											
1.700	0.926	1.700	0.141	150.2	0.000	102.1	0.156	359.7	0.000	97.0	2.258	282.9	0.000
174.5	2.81E+00	-5.92E+00											

CALM buoy with spring
SEAWAY-4.12

Execution: 18-04-1994 / 18:28

FREQUENCY CHARACTERISTICS OF MOTIONS POINTS FORWARD SPEED = 0.00 km
WAVE DIRECTION = +180 deg off stern

POINT NR = 01
X-APP = 9.000 m
Y-CL = 0.000 m
Z-BL = 4.000 m

.....ABSOLUTE MOTIONS.....										..REL MOT..	
WAVE	SQRT	ENCX.....Y.....Z.....Z.....					
FREQ	SL/WL	FREQ	AMPL	PHASE	AMPL	PHASE	AMPL	PHASE	AMPL	PHASE	
(r/s)	(-)	(r/s)	(m/m)	(deg)	(m/m)	(deg)	(m/m)	(deg)	(m/m)	(deg)	
0.200	0.116	0.200	1.311	93.2	0.000	89.8	0.994	0.2	0.062	182.0	
0.233	0.131	0.233	1.162	90.4	0.000	90.0	0.997	0.1	0.087	179.7	
0.267	0.147	0.267	1.084	89.8	0.000	90.2	1.006	359.9	0.121	177.5	
0.300	0.164	0.300	1.052	90.4	0.000	90.2	1.001	0.2	0.143	180.0	
0.333	0.182	0.333	1.028	90.2	0.000	90.3	1.003	0.1	0.173	178.9	
0.367	0.200	0.367	1.009	90.0	0.000	90.4	1.005	360.0	0.206	178.0	

0.400	0.218	0.400	0.994	90.1	0.000	90.4	1.007	359.8	0.241	177.2
0.433	0.236	0.433	0.980	90.2	0.000	90.5	1.010	359.6	0.277	176.4
0.467	0.254	0.467	0.967	90.1	0.000	90.6	1.013	359.4	0.314	175.4
0.500	0.272	0.500	0.954	90.2	0.000	90.7	1.017	359.1	0.353	174.4
0.533	0.291	0.533	0.940	90.3	0.000	90.9	1.020	358.7	0.393	173.3
0.567	0.309	0.567	0.926	90.4	0.000	91.0	1.025	358.2	0.435	171.9
0.600	0.327	0.600	0.911	90.5	0.000	91.1	1.031	357.5	0.479	170.4
0.633	0.345	0.633	0.896	90.6	0.000	91.3	1.037	356.8	0.525	168.5
0.667	0.363	0.667	0.880	90.8	0.000	91.5	1.045	355.8	0.573	166.4
0.700	0.381	0.700	0.863	91.0	0.000	91.7	1.054	354.6	0.625	163.7
0.733	0.400	0.733	0.845	91.2	0.000	91.9	1.065	353.0	0.682	160.5
0.767	0.418	0.767	0.827	91.4	0.000	92.1	1.078	351.0	0.745	156.6
0.800	0.436	0.800	0.807	91.7	0.000	92.3	1.091	348.3	0.817	151.7
0.833	0.454	0.833	0.787	92.1	0.000	92.6	1.105	344.6	0.899	145.3
0.867	0.472	0.867	0.766	92.5	0.000	92.8	1.112	339.7	0.991	137.2
0.900	0.490	0.900	0.744	93.0	0.000	93.1	1.101	333.1	1.085	126.5
0.933	0.509	0.933	0.721	93.5	0.000	93.4	1.052	324.7	1.162	113.0
0.967	0.527	0.967	0.698	94.2	0.000	93.7	0.941	314.9	1.190	96.5
1.000	0.545	1.000	0.674	94.8	0.000	94.1	0.772	306.4	1.136	79.3
1.033	0.563	1.033	0.650	95.6	0.000	94.5	0.589	302.1	1.022	63.4
1.067	0.581	1.067	0.625	96.5	0.000	94.8	0.435	304.2	0.889	49.8
1.100	0.599	1.100	0.600	97.4	0.000	95.3	0.339	312.7	0.773	39.4
1.133	0.618	1.133	0.575	98.5	0.000	95.7	0.291	324.5	0.684	31.3
1.167	0.636	1.167	0.550	99.7	0.000	96.2	0.274	335.6	0.620	25.0
1.200	0.654	1.200	0.525	101.0	0.000	96.7	0.273	344.1	0.575	20.1
1.233	0.672	1.233	0.500	102.3	0.000	97.2	0.275	349.9	0.549	16.5
1.267	0.690	1.267	0.477	103.9	0.000	97.7	0.277	353.9	0.534	13.7
1.300	0.708	1.300	0.453	105.5	0.000	98.3	0.277	356.6	0.530	11.7
1.333	0.727	1.333	0.429	107.2	0.000	98.9	0.275	358.6	0.533	10.3
1.367	0.745	1.367	0.405	109.0	0.000	99.6	0.270	0.1	0.542	9.3
1.400	0.763	1.400	0.380	110.9	0.000	100.3	0.263	1.2	0.557	8.6
1.433	0.781	1.433	0.362	113.0	0.000	101.0	0.255	2.1	0.574	8.1
1.467	0.799	1.467	0.340	115.1	0.000	101.8	0.245	2.9	0.594	7.9
1.500	0.817	1.500	0.319	117.3	0.000	102.7	0.233	3.5	0.617	7.7
1.533	0.836	1.533	0.298	119.7	0.000	103.7	0.221	4.0	0.641	7.7
1.567	0.854	1.567	0.278	122.2	0.000	104.8	0.208	4.5	0.665	7.7
1.600	0.872	1.600	0.259	124.8	0.000	106.1	0.194	5.0	0.691	7.8
1.633	0.890	1.633	0.240	127.5	0.000	107.4	0.181	5.5	0.716	7.9
1.667	0.908	1.667	0.221	130.5	0.000	109.0	0.167	6.0	0.741	8.1
1.700	0.926	1.700	0.203	133.6	0.000	110.9	0.153	6.4	0.766	8.3

CALM buoy with spring
SEAWAY-4.12

Execution: 18-04-1994 / 18:28

STATISTICS OF BASIC MOTIONS

FORWARD SPEED = 0.00 kn
WAVE DIRECTION = +180 deg off stern

.....SEA..... SIGNIFICANT VALUES OF BASIC
MOTIONS..... MEAN ADDED
...INPUT... CALCULATED... SURGE... SWAY... HEAVE... ROLL... PITCH...
...YAW... RESISTANCE
HEIGHT PER HEIGHT PER AMPL PER AMPL PER AMPL PER AMPL PER
AMPL PER AMPL PER GER/BEU BOESE

(m)	(s)	(m)	(s)	(m)	(s)	(m)	(s)	(m)	(s)	(deg)	(s)	(deg)	(s)	(deg)	(s)	(kN)
4.76	12.00	4.75	12.16	2.16	13.13	0.00	12.87	2.39	12.47	0.00	7.27	2.64	9.67	0.00		
7.89	2.3	7.9														
6.06	13.00	6.05	13.14	2.82	14.12	0.00	13.84	3.05	13.39	0.00	7.56	3.17	10.39	0.00		
8.17	2.8	9.5														
7.53	14.00	7.52	14.12	3.58	15.13	0.00	14.82	3.79	14.33	0.00	8.34	3.73	11.11	0.00		
8.43	3.3	11.1														
8.84	15.00	8.83	15.10	4.28	16.17	0.00	15.81	4.44	15.27	0.00	10.91	4.16	11.83	0.00		
8.67	3.4	11.8														

CALM buoy with spring
SEAWAY-4.12

Execution: 18-04-1994 / 18:28

STATISTICS OF MOTIONS IN POINTS

FORWARD SPEED = 0.00 kn
WAVE DIRECTION = +180 deg off stern

POINT NR = 01
X-APP = 9.000 m
Y-CL = 0.000 m
Z-BL = 4.000 m

SIGNIFICANT VALUES

OF.....

.....DISPLACEMENTS..... VELOCITIES.....

.....ACCELERATIONS.....

SEA	X	Y	Z	X	Y	Z	X	Y	Z						
HEIGHT PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER						
PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER						
(m)	(s)	(m)	(s)	(m)	(s)	(m)	(s)	(m)	(s)						
4.76	12.00	2.25	13.0	0.00	12.8	2.39	12.5	1.13	10.8	0.00	10.5	1.26	10.5	0.30	7.54
0.00	6.28	0.79	8.74												
6.06	13.00	2.92	14.0	0.00	13.7	3.05	13.4	1.37	11.4	0.00	11.1	1.50	11.0	0.33	7.80
0.00	6.28	0.90	9.00												
7.53	14.00	3.69	15.0	0.00	14.7	3.79	14.3	1.62	12.1	0.00	11.7	1.75	11.6	0.38	8.24
0.00	7.50	1.00	9.25												
8.84	15.00	4.41	16.0	0.00	15.7	4.44	15.3	1.82	12.8	0.00	12.4	1.93	12.2	0.41	8.94
0.00	9.02	1.06	9.48												

.....VERTICAL RELATIVE MOTIONS.....

..SIGNIFICANT VALUES OF... EXCEEDING.

SEA	DISPLACEMENT	VELOCITY	Z-BL				
HEIGHT PER	AMPL PER	AMPL PER	PROB NR/H				
(m)	(s)	(m/s)	(s)				
4.76	12.00	1.03	8.87	0.69	7.44	54.0	230.0
6.06	13.00	1.18	9.20	0.76	7.50	62.5	258.4
7.53	14.00	1.33	9.52	0.82	7.55	69.0	277.0
8.84	15.00	1.42	9.83	0.85	7.60	72.2	282.2

SALM Hull Input File

4.12
SALM buoy 15.0 x 70.0 dia*draft (4.573x21.34)
21.340 0.0000 4.57300 0.0763

```

10
0.1525 0.1525 0.3811 0.3811 1.1433 1.1433 0.3811 0.3811
0.1525 0.1525
1
1.0 4 0.0
0.00 0.0 0.295 0.0 0.585 0.0 0.585 10.00
0.585 21.34
2.0 4 0.0
0.00 0.0 0.50 0.0 1.00 0.0 1.00 10.00
1.00 21.34
3.0 4 0.0
0.00 0.0 0.63 0.0 1.265 0.0 1.265 10.00
1.265 21.34
4.0 4 0.0
0.00 0.0 0.85 0.0 1.705 0.0 1.705 10.00
1.705 21.34
5.0 4 0.0
0.00 0.0 0.90 0.0 1.98 0.0 1.98 10.00
1.98 21.34
6.0 4 0.0
0.00 0.0 1.145 0.0 2.285 0.0 2.285 10.00
2.285 21.34
7.0 4 0.0
0.00 0.0 0.99 0.0 1.98 0.0 1.98 10.00
1.98 21.34
8.0 4 0.0
0.00 0.0 0.85 0.0 1.705 0.0 1.705 10.00
1.705 21.34
9.0 4 0.0
0.00 0.0 0.63 0.0 1.265 0.0 1.265 10.00
1.265 21.34
10.0 4 0.0
0.00 0.0 0.50 0.0 0.98 0.0 0.98 10.00
0.98 21.34
11.0 4 0.0
0.00 0.0 0.295 0.0 0.585 0.0 0.585 10.00
0.585 21.34
1.0000 1.0000 1.0000
*** End of file ***

```

SALM Environment Input File

```

4.12
SALM buoy with spring
+1 +1 0 +1 0
16.77 0.000 0.000 304.878 1.025E+00
123456 1 6 +5 0
1
0.0
1
180.0
2.500 1 0.200 1.700 0.033333
1.474
+3.00 +5.561 2.375 2.735
0

```



```

3
5.000
0.0 61.250 105.000
0
1
4.00 0.0 0.0
20.4 0.0 0.0
-1
9.000 0.000 4.000
4
+2
4.76 12.00
6.06 13.00
7.53 14.00
8.84 15.00
0

```

*** End of file ***

SALM Output File

```

#####
# Program: SEAWAY                               Journee #
#                                               #
# STRIP THEORY CALCULATIONS OF MOTIONS AND LOADS IN A SEAWAY #
#                                               #
#               Release 4.12                       #
#               (31-07-1993)                       #
#####

```

User: University of California, Berkeley, U.S.A.

INPUT DATA

SALM buoy with spring

```

PRINT-CODE INPUT DATA ..... KPR(1) : 1
PRINT-CODE GEOMETRIC DATA ..... KPR(2) : 1
PRINT-CODE HYDRODYNAMIC COEFFICIENTS KPR(3) : 0
PRINT-CODE FREQUENCY CHARACTERISTICS KPR(4) : 1
PRINT-CODE SPECTRAL DATA ..... KPR(5) : 0

```

```

ACTUAL MIDSHIP DRAFT ..... DRAFT : 16.770 m
ACTUAL TRIM BY STERN ..... TRIM : 0.000 m
DUMMY VALUE, FOR THE TIME BEING ..... DIST : 0.000

```

```

WATER DEPTH ..... DEPTH : 304.9 m
DENSITY OF WATER ..... RHO : 1.025 ton/m3

```

```

DEGREES OF FREEDOM CODE ..... MOT : 123456
VERSION-CODE OF STRIP THEORY METHOD ... KTH : 1
NUMBER OF TERMS IN POTENTIAL SERIES .. MSER : 6
CODE OF USED 2-D APPROXIMATION ..... KCOF : 5
NUMBER OF "FREE-CHOICE" SECTIONS ..... NFR : 0

```

NUMBER OF FORWARD SPEEDS NV : 1
 FORWARD SPEEDS (kn) VK(NV) : 0.00

NUMBER OF WAVE DIRECTIONS NWD : 1
 WAVE DIRECTIONS (deg off stern) WAVDIR(NWD) : 180.0

MAX. FREQ. OF ENCOUNTER IN SERIES , FREQMAX : 2.500 rad/sec (range = 0.000 - 3.125 rad/sec)

CODE FOR WAVE FREQUENCY INPUT KOMEQ : 1
 MINIMUM CIRCULAR WAVE FREQUENCY OMMIN : 0.200 rad/sec
 MAXIMUM CIRCULAR WAVE FREQUENCY OMMAX : 1.700 rad/sec
 INCREMENT IN WAVE FREQUENCIES OMINC : 0.033 rad/sec

WAVE AMPLITUDE FOR LINEARISATION ... WAVAMP : 1.474 m

INPUT DATA (continued)

BASE LINE TO CENTRE OF GRAVITY ... +GKGM=KG : 3.000 m

MASS-GYRADIUS k-xx GYR(1) : 5.561 m
 MASS-GYRADIUS k-yy GYR(2) : 2.375 m
 MASS-GYRADIUS k-zz GYR(3) : 2.735 m

NUMBER OF LOAD-CALCULATION SECTIONS .. NBTM : 0

CODE OF ROLL DAMPING INPUT KRD : 3
 AVERAGE ROLL AMPLITUDE ROLAMP : 5.000 deg
 HEIGHT OF BILGE KEEL HBK : 0.000 m
 DISTANCE OF A.P.P. TO AFT END B.K. ... XBKA : 61.25 m
 DISTANCE OF A.P.P. TO FORWARD END B.K. XBKF : 105.00 m

CODE OF ANTI-ROLLING DEVICES KARD : 0

NUMBER OF LINEAR SPRINGS NCAB : 1
 COORDINATES AND LINEAR SPRING COEFFICIENTS : 4.000 0.000 0.000 2.040E+01
 0.000E-01 0.000E-01

NUMBER OF DISCRETE POINTS NPTS : -1
 COORDINATES OF POINTS (m) .. PTSXYZ(NPTS,3) : 9.00 0.00 4.00

NUMBER OF SEA STATES NSEA : 4
 CODE OF IRREGULAR SEA DESCRIPTION KSEA : 2
 WAVE HEIGHTS (m) HW(K) / PERIODS (s) TW(K) : 4.76 12.00
 6.06 13.00
 7.53 14.00
 8.84 15.00

INPUT-CODE OF CRITERA FOR SHIPMOTIONS KRIT : 0

GEOMETRICAL HULLFORM DATA

ACTUAL MIDSHIP DRAFT (T) : 16.770 m
 ACTUAL TRIM BY STERN : 0.000 m

LENGTH BETWEEN PERPENDICULARS (L_{pp}) : 4.573 m
 REAR SECTION TO A.P.P. : 0.076 m

WATERLINE : LENGTH (L_{wl}) : 4.421 m
 BEAM (B) : 4.570 m
 AREA : 16.2806 m²
 AREA COEFFICIENT (L_{pp}) : 0.7790
 AREA COEFFICIENT (L_{wl}) : 0.8058
 CENTROID TO A.P.P. : 2.133 m (-0.153 m or -3.35 % L_{pp}/2)
 CENTROID TO REAR SECTION : 2.209 m (-0.001 m or -0.02 % L_{wl}/2)

DISPLACEMENT : VOLUME : 273 m³
 BLOCKCOEFFICIENT (L_{pp}) : 0.7790
 BLOCKCOEFFICIENT (L_{wl}) : 0.8058
 CENTROID TO A.P.P. : 2.133 m (-0.153 m or -3.35 % L_{pp}/2)
 CENTROID TO REAR SECTION .. : 2.209 m (-0.001 m or -0.02 % L_{wl}/2)
 CENTROID TO WATERLINE : 8.385 m
 CENTROID TO KEELLINE : 8.385 m
 MIDSHP SECTION COEFFICIENT : 0.9976
 LONG. PRISMATIC COEFFICIENT : 0.7809
 VERT. PRISMATIC COEFFICIENT : 1.0000
 RATIO L_{pp}/B : 1.001
 RATIO L_{wl}/B : 0.967
 RATIO B/F : 0.273
 WETTED SURFACE HULL : 204 m²

STABILITY PARAMETERS

KB : 8.385 m
 KG : 3.000 m
 BM-TRANSVERSE .. : 0.079 m
 GM-TRANSVERSE .. : 5.464 m
 BM-LONGITUDINAL : 0.076 m
 GM-LONGITUDINAL : 5.462 m

SALM buoy with spring
 SEAWAY-4.12

Execution: 18-04-1994 , 18:19

SECTIONAL HULLFORM DATA

STATION NUMBER	X-APP (-)	HALF CL-CL (m)	HALF WIDTH (m)	HALF DRAFT (m ²)	AREA COEFF (m)	AREA (m ²)	KB	BO	WETTED LENGTH (m)
----------------	-----------	----------------	----------------	------------------------------	----------------	------------------------	----	----	-------------------

5.00	+10.1497	+1.0000	-0.7571	-0.0665	+0.0276	-0.0099	+0.0009	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.036								
6.00	+10.3885	+1.0000	-0.7265	-0.0738	+0.0296	-0.0091	-0.0003	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.049								
7.00	+10.1497	+1.0000	-0.7571	-0.0665	+0.0276	-0.0099	+0.0009	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.036								
8.00	+9.9255	+1.0000	-0.7856	-0.0592	+0.0258	-0.0101	+0.0009	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.052								
9.00	+9.5451	+1.0000	-0.8344	-0.0459	+0.0208	-0.0093	+0.0014	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.044								
10.00	+9.2796	+1.0000	-0.8680	-0.0364	+0.0166	-0.0072	+0.0006	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.051								
11.00	+8.8661	+1.0000	-0.9177	-0.0211	+0.0079	-0.0001	-0.0030	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.082								

SALM buoy with spring
SEAWAY-4.12

Execution: 18-04-1994 / 18:19

NATURAL ROLL AND COEFFICIENTS AT FIXED AMPLITUDE

FORWARD SHIP SPEED . (kn) : 0.00
MEAN ROLL AMPLITUDE (deg) : 5.000

NATURAL ROLL PERIOD . (s) : 4.775
NATURAL FREQUENCY . (r/s) : 1.316

LINEAR EQUIVALENT GM (m) : 5.461

MASS, k-phi-phi (m) : 5.561
COMPONENTS k-phi-phi:
SOLID MASS PART .. (m) : 5.561
2-D POTENTIAL PART (m) : 0.000

DAMPING, kappa (-) : 2.6400
COMPONENTS kappa:
2-D POTENTIAL PART (-) : 2.4314
SPEED EFFECT PART (-) : 0.0000
SKIN FRICTION PART (-) : 0.0005
EDDY MAKING PART . (-) : 0.2081
LIFT MOMENT PART . (-) : 0.0000
BILGE KEEL PART .. (-) : 0.0000

(NON)LINEAR DAMPING COEFFICIENTS:

Kappa-1 (-) : 0.0003
Kappa-2 (-) : 2.3873

NATURAL HEAVE AT ZERO FORWARD SPEED

NATURAL HEAVE PERIOD (s) : 8.755

1.600	0.436	1.600	0.605	303.6	0.000	88.9	0.038	360.0	0.000	10.2	11.537	152.7	0.000
174.6	6.38E-05	-1.46E+01											
1.633	0.445	1.633	0.554	304.0	0.000	88.6	0.036	360.0	0.000	8.9	11.074	154.3	0.000
175.6	2.13E-05	-1.41E+01											
1.667	0.454	1.667	0.511	304.6	0.000	88.3	0.035	360.0	0.000	7.3	10.701	155.9	0.000
176.4	-6.48E-06	-1.37E+01											
1.700	0.463	1.700	0.474	305.2	0.000	88.0	0.033	360.0	0.000	5.2	10.408	157.5	0.000
177.0	-7.06E-06	-1.33E+01											

SALM buoy with spring
SEAWAY-4.12

Execution: 18-04-1994 / 18:19

FREQUENCY CHARACTERISTICS OF MOTIONS POINTS FORWARD SPEED = 0.00 kn
----- WAVE DIRECTION = +180 deg off stern

POINT NR = 01
X-APP = 9.000 m
Y-CL = 0.000 m
Z-BL = 4.000 m

.....ABSOLUTE MOTIONS.....										..REL MOT..	
WAVE	SQRT	ENCX.....Y.....Z.....Z.....					
FREQ	SL/WL	FREQ	AMPL	PHASE	AMPL	PHASE	AMPL	PHASE	AMPL	PHASE	
(r/s)	(-)	(r/s)	(m/m)	(deg)	(m/m)	(deg)	(m/m)	(deg)	(m/m)	(deg)	
0.200	0.058	0.200	43.934	8.2	0.000	9.5	1.182	201.5	2.150	12.5	
0.233	0.065	0.233	4.141	151.9	0.000	94.3	1.211	352.6	0.282	135.9	
0.267	0.073	0.267	1.632	91.1	0.000	140.7	0.953	358.8	0.085	57.0	
0.300	0.082	0.300	1.134	111.6	0.000	49.1	1.038	1.3	0.057	135.0	
0.333	0.091	0.333	1.361	96.9	0.000	96.1	1.076	357.5	0.147	122.1	
0.367	0.100	0.367	1.113	100.9	0.000	96.4	1.039	1.9	0.074	125.2	
0.400	0.109	0.400	1.033	100.9	0.000	96.2	1.082	4.6	0.089	163.7	
0.433	0.118	0.433	0.927	98.4	0.000	92.8	1.126	0.3	0.184	137.2	
0.467	0.127	0.467	0.907	92.2	0.000	102.6	1.159	1.9	0.205	146.3	
0.500	0.136	0.500	0.888	89.1	0.000	95.5	1.266	3.8	0.292	162.2	
0.533	0.145	0.533	0.874	87.6	0.000	92.3	1.387	3.2	0.422	163.3	
0.567	0.154	0.567	0.893	80.8	0.000	94.6	1.620	4.7	0.646	171.9	
0.600	0.163	0.600	0.887	75.5	0.000	84.1	1.926	3.2	0.965	171.6	
0.633	0.173	0.633	0.931	71.3	0.000	47.6	2.394	2.5	1.442	173.0	
0.667	0.182	0.667	0.918	66.6	0.000	303.0	3.315	356.5	2.411	167.8	
0.700	0.191	0.700	0.919	62.2	0.000	194.5	6.670	336.9	5.975	150.3	
0.733	0.200	0.733	0.967	58.5	0.000	125.3	5.182	216.5	6.153	34.1	
0.767	0.209	0.767	0.988	54.7	0.000	118.5	0.979	183.6	1.949	13.7	
0.800	0.218	0.800	1.016	50.7	0.000	116.0	0.259	84.2	0.893	11.3	
0.833	0.227	0.833	1.036	49.1	0.000	114.1	0.608	40.1	0.426	10.2	
0.867	0.236	0.867	1.067	46.0	0.000	113.0	0.892	31.2	0.109	21.0	
0.900	0.245	0.900	1.104	42.9	0.000	112.1	1.118	27.9	0.146	174.2	
0.933	0.254	0.933	1.147	39.6	0.000	111.2	1.312	26.1	0.359	180.7	
0.967	0.263	0.967	1.189	36.3	0.000	110.6	1.483	24.8	0.553	181.5	
1.000	0.272	1.000	1.235	32.3	0.000	110.0	1.654	23.2	0.755	180.6	
1.033	0.282	1.033	1.292	27.9	0.000	109.6	1.833	21.6	0.971	179.7	
1.067	0.291	1.067	1.351	22.7	0.000	109.5	2.019	19.2	1.209	177.6	
1.100	0.300	1.100	1.415	16.8	0.000	109.5	2.214	16.3	1.469	175.0	
1.133	0.309	1.133	1.468	9.8	0.000	109.8	2.402	12.4	1.745	171.1	
1.167	0.318	1.167	1.505	1.8	0.000	110.4	2.571	7.4	2.029	166.2	

1.200	0.327	1.200	1.516	352.7	0.000	111.2	2.701	1.4	2.303	160.2
1.233	0.336	1.233	1.479	343.1	0.000	112.2	2.750	354.9	2.518	153.6
1.267	0.345	1.267	1.398	333.6	0.000	113.5	2.714	348.3	2.656	146.9
1.300	0.354	1.300	1.282	324.7	0.000	114.9	2.602	342.5	2.710	140.9
1.333	0.363	1.333	1.152	316.8	0.000	116.4	2.447	337.6	2.703	135.9
1.367	0.372	1.367	1.019	310.3	0.000	118.1	2.268	334.1	2.646	132.3
1.400	0.381	1.400	0.895	305.1	0.000	119.8	2.092	331.7	2.568	129.9
1.433	0.390	1.433	0.785	301.0	0.000	121.6	1.931	330.5	2.484	128.5
1.467	0.400	1.467	0.693	297.7	0.000	123.5	1.791	330.0	2.408	128.1
1.500	0.409	1.500	0.613	295.3	0.000	125.3	1.671	330.2	2.337	128.5
1.533	0.418	1.533	0.545	293.5	0.000	127.1	1.569	331.0	2.278	129.5
1.567	0.427	1.567	0.489	291.9	0.000	128.9	1.485	332.0	2.230	130.9
1.600	0.436	1.600	0.440	290.8	0.000	130.6	1.416	333.4	2.191	132.7
1.633	0.445	1.633	0.400	289.9	0.000	132.3	1.360	334.9	2.162	134.9
1.667	0.454	1.667	0.365	289.2	0.000	133.9	1.314	336.5	2.141	137.2
1.700	0.463	1.700	0.335	288.3	0.000	135.4	1.278	338.1	2.129	139.7

SALM buoy with spring
SEAWAY-4.12

Execution: 18-04-1994 / 18:19

STATISTICS OF BASIC MOTIONS

FORWARD SPEED = 0.00 kn
WAVE DIRECTION = +180 deg off stern

SEA		SIGNIFICANT VALUES OF BASIC MOTIONS														
		MEAN ADDED														
....INPUT....	..CALCULATED..SURGE....SWAY....HEAVE....ROLL....PITCH....										
....YAW....	RESISTANCE	HEIGHT PER AMPL	HEIGHT PER AMPL	AMPL PER GER/BEU	AMPL PER BOESE	AMPL PER (deg)	AMPL PER (s)	AMPL PER (deg)	AMPL PER (s)	AMPL PER (deg)	AMPL PER (s)	AMPL PER (deg)	AMPL PER (s)	AMPL PER (kN)		
(m)	(s)	(m)	(s)	(m)	(s)	(m)	(s)	(m)	(s)	(deg)	(s)	(deg)	(s)	(deg)	(s)	(kN)
4.76	12.00	4.75	12.16	2.51	11.75	0.00	11.48	4.15	10.11	0.00	9.91	10.90	6.46	0.00		
13.14	0.7	-4.3														
6.06	13.00	6.05	13.14	3.32	13.24	0.00	12.04	4.85	10.57	0.00	9.93	11.96	6.51	0.00		
17.12	0.8	-5.2														
7.53	14.00	7.52	14.12	4.58	15.59	0.00	12.68	5.60	11.12	0.00	9.97	12.94	6.56	0.00		
19.69	1.0	-6.1														
8.84	15.00	8.83	15.10	7.43	20.69	0.00	13.41	6.18	11.74	0.00	10.02	13.49	6.71	0.00		
21.17	1.0	-6.5														

SALM buoy with spring
SEAWAY-4.12

Execution: 18-04-1994 / 18:19

STATISTICS OF MOTIONS IN POINTS

FORWARD SPEED = 0.00 kn
WAVE DIRECTION = +180 deg off stern

POINT NR = 01
X-APP = 9.000 m
Y-CL = 0.000 m
Z-BL = 4.000 m

.....SIGNIFICANT VALUES OF.....

.....DISPLACEMENTS.....											VELOCITIES.....			
.....ACCELERATIONS.....															
.....SEA.....X.....Y.....Z.....X.....Y.....Z.....X.....Y.....Z.....X.....Y.....Z.....			
HEIGHT PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER			
PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER			
(m)	(s)	(m)	(s)	(m)	(s)	(m)	(s)	(m/s)	(s)	(m/s)	(s)	(m/s ²)	(s)	(m/s ²)	(s)
4.76	12.00	2.42	12.3	0.00	11.7	4.43	9.80	1.33	8.91	0.00	10.4	2.94	8.48	1.10	7.13
0.00	9.26	2.28	7.03												
6.06	13.00	3.24	13.8	0.00	12.5	5.16	10.2	1.59	9.69	0.00	10.7	3.30	8.65	1.23	7.32
0.00	9.29	2.51	7.11												
7.53	14.00	4.51	16.2	0.00	13.5	5.94	10.8	1.92	10.8	0.00	11.2	3.65	8.82	1.37	7.65
0.00	9.33	2.73	7.17												
8.84	15.00	7.41	21.2	0.00	14.7	6.52	11.4	2.39	13.1	0.00	11.7	3.83	9.01	1.55	8.75
0.00	9.38	2.82	7.22												

.....VERTICAL RELATIVE MOTIONS.....												
..SIGNIFICANT VALUES OF... EXCEEDING.												
.....SEA.....	DISPLACEMENT				VELOCITY				Z-BL...			
HEIGHT PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER
PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER
(m)	(s)	(m)	(s)	(m/s)	(s)	(%)	(1/h)	(%)	(1/h)	(%)	(1/h)	(%)
4.76	12.00	3.40	8.56	15.86	8.61	0.0	0.0					
6.06	13.00	3.77	8.58	17.51	8.61	0.0	0.0					
7.53	14.00	4.09	8.61	18.98	8.62	0.0	0.0					
8.84	15.00	4.24	8.66	19.58	8.62	0.0	0.0					

San Diego Hull Input File

4.12

San Diego class Tanker. 278.89 x 50.6 x 23.77 meter. Hull-draft = ? meter.

18.0790	0.0000	278.8920	6.7060									
26												
3.048	3.0480	3.048	3.048	3.048	3.048	3.658	5.944	6.096				
10.668	10.6680	13.716	13.716	13.716	13.716	13.716	27.432	27.432				
27.432	27.4320	13.716	13.716	11.43	11.43	6.4	6.096					
3.023	2.9210											
2												
-6.706	4	0.0000										
13.970	0.0000	15.2400	2.1840	16.4590	3.9310	17.678	5.359					
18.079	5.6980											
-3.658	4	0.0000										
13.2060	0.0000	14.0210	1.6370	16.4590	5.4120	17.0000	5.9980					
18.0790	7.1670											
-0.610	4	0.0000										
12.4070	0.0000	14.6300	4.2540	17.0690	7.6070	17.5000	8.0490					
18.0790	8.6420											
0.50	6	0.0000										
11.6260	0.0000	12.1920	1.0840	14.6300	5.7530	15.8500	7.5220					
17.0690	9.0170	17.5000	9.3180	18.0790	9.7220							
1.00	6	0.0000										
10.9480	0.0000	13.4110	5.1820	14.6300	7.2260	15.8500	8.9280					
17.0690	10.3790	17.5000	10.6750	18.0790	11.0710							
2.00	8	0.0000										
9.5930	0.0000	9.7540	0.1840	10.3630	0.8830	10.9730	1.9810					
11.5820	3.2290	13.4110	6.7120	15.2400	9.5060	16.4590	11.0140					

APPENDIX 4: CALCULATIONS OF OSCILLATING MOTIONS

18.0790	12.7000							
3.00	12	0.0000						
0.0000	0.0000	0.3050	0.1830	0.6100	0.3140	2.4380	0.9910	
3.6580	1.3050	7.9250	1.8670	9.1440	2.3050	9.7540	2.7300	
10.3630	3.3750	15.2400	11.1060	16.4590	12.5320	17.6780	13.7890	
18.0790	14.1270							
4.00	14	0.0000						
0.0000	0.0000	0.0000	1.2190	0.3050	2.0420	0.6100	2.7080	
1.2190	3.2070	1.8290	3.5590	3.0480	4.0610	6.7060	5.0260	
7.9250	5.4390	9.1440	6.1020	10.3630	7.1750	15.2400	13.5950	
17.0690	15.4560	17.5000	15.8210	18.0790	16.3110			
5.00	14	0.0000						
0.0000	0.0000	0.0000	3.3530	0.3050	4.4200	0.6100	5.1910	
1.2190	5.8390	1.8290	6.2860	3.0480	6.9090	6.7060	8.2930	
7.9250	8.9060	9.1440	9.7150	10.3630	10.7890	14.0210	14.8080	
16.4590	17.1230	17.0000	17.5320	18.0790	18.3480			
6.00	10	0.0000						
0.0000	0.0000	0.0000	7.3150	0.3050	8.7780	0.6100	9.5030	
1.2190	10.2460	1.8290	10.7630	3.6580	11.9000	6.0960	13.3190	
14.0210	19.0310	17.6780	21.2150	18.0790	21.3930			
7.00	10	0.0000						
0.0000	0.0000	0.0000	11.2780	0.3050	12.8020	0.6100	13.5860	
1.2190	14.3830	1.8290	14.9800	6.7060	18.4210	9.1440	19.8180	
11.5820	21.0690	16.4590	23.0190	18.0790	23.5240			
8.00	10	0.0000						
0.0000	0.0000	0.0000	16.4590	0.3050	17.6780	0.6100	18.3450	
1.8290	19.8020	4.2670	21.6090	5.4860	22.2470	7.9250	23.1580	
10.3630	23.7870	13.4110	24.3650	18.0790	24.7740			
9.00	10	0.0000						
0.0000	0.0000	0.0000	20.2390	0.3050	21.4580	0.6100	22.1330	
1.2190	22.8730	2.4380	23.7680	3.6580	24.3050	4.8770	24.6440	
7.3150	25.0030	10.3630	25.1940	18.0790	25.2880			
10.00	10	0.0000						
0.0000	0.0000	0.0000	22.2500	0.3050	23.4090	0.6100	24.1710	
1.2190	24.6890	1.8290	24.9780	2.4380	25.1460	3.0480	25.2380	
4.2670	25.2980	12.0000	25.2980	18.0790	25.2980			
11.00	8	0.0000						
0.0000	0.0000	0.0000	22.8600	0.3050	23.8350	0.6100	24.4730	
1.2190	24.9710	1.8290	25.2220	2.4380	25.2980	10.0000	25.2980	
18.0790	25.2980							
12.00	8	0.0000						
0.0000	0.0000	0.0000	22.8600	0.3050	23.8350	0.6100	24.4730	
1.2190	24.9710	1.8290	25.2220	2.4380	25.2980	10.0000	25.2980	
18.0790	25.2980							
13.00	8	0.0000						
0.0000	0.0000	0.0000	22.8600	0.3050	23.8350	0.6100	24.4730	
1.2190	24.9710	1.8290	25.2220	2.4380	25.2980	10.0000	25.2980	
18.0790	25.2980							
14.00	8	0.0000						
0.0000	0.0000	0.0000	22.8600	0.3050	23.8350	0.6100	24.4730	
1.2190	24.9710	1.8290	25.2220	2.4380	25.2980	10.0000	25.2980	
18.0790	25.2980							
15.00	8	0.0000						
0.0000	0.0000	0.0000	22.2500	0.3050	23.5920	0.6100	24.4760	

```

1.2190 24.9710 1.8290 25.2220 2.4380 25.2980 10.0000 25.2980
18.0790 25.2980
16.00 10 0.0000
0.0000 0.0000 0.0000 21.3360 0.3050 23.4700 0.6100 24.0790
1.2190 24.6480 1.8290 24.9940 2.4380 25.2030 3.0480 25.2950
8.0000 25.2950 14.0000 25.2950 18.0790 25.2950
17.00 10 0.0000
0.0000 0.0000 0.0000 20.4220 0.3050 21.7930 0.6100 22.4310
1.2190 23.2120 1.8290 23.7390 3.0480 24.4090 4.2670 24.7680
5.4860 24.9020 12.0000 25.0110 18.0790 25.1130
18.00 10 0.0000
0.0000 0.0000 0.0000 16.1540 0.3050 18.2880 0.6100 19.3070
1.2190 20.3490 1.8290 21.0720 3.0480 22.0220 4.2670 22.5650
5.4860 22.8890 7.925 23.1710 18.0790 23.6970
19.00 10 0.0000
0.0000 0.0000 0.0000 9.4490 0.3050 11.8870 0.6100 13.6720
1.2190 14.8720 2.4380 16.3930 3.6580 17.4150 4.8770 18.1010
7.3150 18.8850 10.3630 19.3830 18.0790 19.6440
19.50 12 0.0000
0.0000 0.0000 0.0000 5.1820 0.3050 7.4680 0.6100 8.9410
1.2190 10.3440 1.8290 11.2840 3.0480 12.7190 4.2670 13.5190
5.4860 14.2050 7.9250 15.0530 10.3630 15.4810 14.0000 15.6830
18.0790 15.9090
19.60 10 0.0000
0.3050 0.0000 0.6100 2.9430 1.2190 4.5690 1.8290 5.7020
2.4380 6.5630 3.6580 7.7600 4.8770 8.5880 6.0960 9.1950
7.3150 9.6040 8.5340 9.8520 18.0790 10.3970
19.70 12 0.0000
1.6760 0.0000 1.8290 2.0290 2.4380 3.2320 3.0480 4.0290
3.658 4.6420 4.2670 5.1310 4.8770 5.5090 5.4860 5.8010
6.0960 6.0230 7.3150 6.3090 8.5340 6.3850 12.0000 6.4640
18.0790 6.6020
19.80 2 0.0000
17.0120 0.0000 17.5000 1.1490 18.0790 3.1020
1.0000 1.0000 1.0000
*** End of file ***

```

San Diego Full Load Environment File

```

4.12
San Diego class tanker (full load) motions with spring
+1 +1 0 +1 0
18.290 0.000 0.000 304.878 1.025E+00
123456 1 6 +5 0
1
0.0
1
180.0
2.500 1 0.200 1.700 0.033333
1.474
+16.800 +20.981 69.750 69.750
0
3
5.000

```

0.0	61.250	105.000
0		
1		
278.00	0.0	0.0
11.66	0.0	0.0
-1		
278.000	0.000	10.000
4		
+2		
4.76	12.00	
6.06	13.00	
7.53	14.00	
8.84	15.00	
0		

*** End of file ***

San Diego Full Load Output File

```
#####
# Program: SEAWAY                               Journey #
#                                               #
# STRIP THEORY CALCULATIONS OF MOTIONS AND LOADS IN A SEAWAY #
#                                               #
#           Release 4.12                       #
#           (31-07-1993)                       #
#####
```

User: University of California, Berkeley, U.S.A.
 INPUT DATA

San Diego class tanker (full load) motions with spring

```
PRINT-CODE INPUT DATA ..... KPR(1): 1
PRINT-CODE GEOMETRIC DATA ..... KPR(2): 1
PRINT-CODE HYDRODYNAMIC COEFFICIENTS KPR(3): 0
PRINT-CODE FREQUENCY CHARACTERISTICS KPR(4): 1
PRINT-CODE SPECTRAL DATA ..... KPR(5): 0
```

```
ACTUAL MIDSHP DRAFT ..... DRAFT : 18.290 m
ACTUAL TRIM BY STERN ..... TRIM : 0.000 m
DUMMY VALUE, FOR THE TIME BEING ..... DIST : 0.000
```

```
WATER DEPTH ..... DEPTH : 304.9 m
DENSITY OF WATER ..... RHO : 1.025 ton/m3
```

```
DEGREES OF FREEDOM CODE ..... MOT : 123456
VERSION-CODE OF STRIP THEORY METHOD ... KTH : 1
NUMBER OF TERMS IN POTENTIAL SERIES .. MSER : 6
CODE OF USED 2-D APPROXIMATION ..... KCOF : 5
NUMBER OF "FREE-CHOICE" SECTIONS ..... NFR : 0
```

```
NUMBER OF FORWARD SPEEDS ..... NV : 1
```

FORWARD SPEEDS (kn) VK(NV) : 0.00
 NUMBER OF WAVE DIRECTIONS NWD : 1
 WAVE DIRECTIONS (deg off stern) WAVDIR(NWD) : 180.0
 MAX. FREQ. OF ENCOUNTER IN SERIES . FREQMAX : 2.500 rad/sec (range = 0.000 - 3.125 rad/sec)
 CODE FOR WAVE FREQUENCY INPUT KOMEQ : 1
 MINIMUM CIRCULAR WAVE FREQUENCY OMMIN : 0.200 rad/sec
 MAXIMUM CIRCULAR WAVE FREQUENCY OMMAX : 1.700 rad/sec
 INCREMENT IN WAVE FREQUENCIES OMINC : 0.033 rad/sec
 WAVE AMPLITUDE FOR LINEARISATION ... WAVAMP : 1.474 m

INPUT DATA (continued)

 BASE LINE TO CENTRE OF GRAVITY ... +GKGM=KG : 16.800 m
 MASS-GYRADIUS k-xx GYR(1) : 20.981 m
 MASS-GYRADIUS k-yy GYR(2) : 69.750 m
 MASS-GYRADIUS k-zz GYR(3) : 69.750 m
 NUMBER OF LOAD-CALCULATION SECTIONS .. NBTM : 0
 CODE OF ROLL DAMPING INPUT KRD : 3
 AVERAGE ROLL AMPLITUDE ROLAMP : 5.000 deg
 HEIGHT OF BILGE KEEL HBK : 0.000 m
 DISTANCE OF A.P.P. TO AFT END B.K. ... XBKA : 61.25 m
 DISTANCE OF A.P.P. TO FORWARD END B.K. XBKF : 105.00 m
 CODE OF ANTI-ROLLING DEVICES KARD : 0
 NUMBER OF LINEAR SPRINGS NCAB : 1
 COORDINATES AND LINEAR SPRING COEFFICIENTS :278.000 0.000 0.000 1.166E+01
 0.000E-01 0.000E-01
 NUMBER OF DISCRETE POINTS NPTS : -1
 COORDINATES OF POINTS (m) .. PTSXYZ(NPTS,3) : 278.00 0.00 10.00
 NUMBER OF SEA STATES NSEA : 4
 CODE OF IRREGULAR SEA DESCRIPTION KSEA : 2
 WAVE HEIGHTS (m) HW(K) / PERIODS (s) TW(K) : 4.76 12.00
 6.06 13.00
 7.53 14.00
 8.84 15.00
 INPUT-CODE OF CRITERA FOR SHIPMOTIONS KRIT : 0

San Diego class tanker motions with spring
 SEAWAY-4.12

Execution: 18-04-1994 , 18:44

GEOMETRICAL HULLFORM DATA

ACTUAL MIDSHIP DRAFT (T) : 18.290 m
 ACTUAL TRIM BY STERN : 0.000 m

LENGTH BETWEEN PERPENDICULARS (Lpp) : 278.892 m
 REAR SECTION TO A.P.P. : 6.706 m

WATERLINE : LENGTH (Lwl) : 285.598 m
 BEAM (B) : 50.596 m
 AREA : 12909 m²
 AREA COEFFICIENT (Lpp) : 0.9148
 AREA COEFFICIENT (Lwl) : 0.8934
 CENTROID TO A.P.P. : 139.862 m (+0.416 m or +0.15 % Lpp/2)
 CENTROID TO REAR SECTION : 146.568 m (+3.769 m or +1.32 % Lwl/2)

DISPLACEMENT : VOLUME : 217682 m³
 BLOCKCOEFFICIENT (Lpp) : 0.8434
 BLOCKCOEFFICIENT (Lwl) : 0.8236
 CENTROID TO A.P.P. : 147.407 m (+7.961 m or +2.85 % Lpp/2)
 CENTROID TO REAR SECTION .. : 154.113 m (+11.314 m or +3.96 % Lwl/2)
 CENTROID TO WATERLINE : 8.870 m
 CENTROID TO KEELLINE : 9.420 m
 MIDSHP SECTION COEFFICIENT : 0.9970
 LONG. PRISMATIC COEFFICIENT : 0.8459
 VERT. PRISMATIC COEFFICIENT : 0.9220
 RATIO Lpp/B : 5.512
 RATIO Lwl/B : 5.645
 RATIO B/T : 2.766
 WETTED SURFACE HULL : 21184 m²

STABILITY PARAMETERS

KB : 9.420 m
 KG : 16.800 m
 BM-TRANSVERSE . : 11.366 m
 GM-TRANSVERSE . : 3.986 m
 BM-LONGITUDINAL : 334.977 m
 GM-LONGITUDINAL : 327.597 m

San Diego class tanker motions with spring
 SEAWAY-4.12

Execution: 18-04-1994 , 18:44

SECTIONAL HULLFORM DATA

STATION NUMBER	X-APP (-)	HALF CL-CL (m)	HALF WIDTH (m)	HALF DRAFT (m ²)	AREA COEFF (-)	AREA (m)	AREA (m)	KB (m)	BO WETTED LENGTH (m)
-6.71	-6.706	0.000	5.876	4.320	28.6977	0.5652	16.777	1.513	14.683
-3.66	-3.658	0.000	7.396	5.084	42.6789	0.5676	16.448	1.842	18.054

-0.61	-0.610	0.000	8.858	5.883	59.4725	0.5706	16.194	2.096	21.405
0.50	2.438	0.000	9.869	6.664	76.8099	0.5839	16.066	2.224	24.116
1.00	5.486	0.000	11.215	7.342	98.2492	0.5966	15.664	2.626	27.184
2.00	8.534	0.000	12.920	8.697	122.8398	0.5466	15.340	2.950	31.352
3.00	12.192	0.000	14.305	18.290	181.2260	0.3463	13.722	4.568	49.443
4.00	18.136	0.000	16.490	18.290	290.7200	0.4820	11.928	6.362	52.077
5.00	24.232	0.000	18.508	18.290	396.6815	0.5859	11.096	7.194	55.617
6.00	34.900	0.000	21.487	18.290	565.9597	0.7201	10.419	7.871	62.097
7.00	45.568	0.000	23.590	18.290	707.2645	0.8196	10.003	8.287	68.457
8.00	59.284	0.000	24.792	18.290	835.6859	0.9215	9.554	8.736	75.811
9.00	73.000	0.000	25.291	18.290	904.0583	0.9772	9.296	8.994	81.533
10.00	86.716	0.000	25.298	18.290	921.0693	0.9953	9.184	9.106	84.506
11.00	100.432	0.000	25.298	18.290	922.6645	0.9970	9.171	9.119	85.031
12.00	127.864	0.000	25.298	18.290	922.6645	0.9970	9.171	9.119	85.031
13.00	155.296	0.000	25.298	18.290	922.6645	0.9970	9.171	9.119	85.031
14.00	182.728	0.000	25.298	18.290	922.6645	0.9970	9.171	9.119	85.031
15.00	210.160	0.000	25.298	18.290	922.4080	0.9968	9.173	9.117	84.991
16.00	223.876	0.000	25.295	18.290	920.7101	0.9950	9.187	9.103	84.440
17.00	237.592	0.000	25.117	18.290	902.6983	0.9825	9.256	9.034	82.071
18.00	249.022	0.000	23.708	18.290	834.1783	0.9619	9.361	8.929	77.039
19.00	260.452	0.000	19.651	18.290	673.5606	0.9370	9.558	8.732	67.274
19.50	266.852	0.000	15.921	18.290	524.3181	0.9003	9.769	8.521	58.800
19.60	272.948	0.000	10.409	17.985	323.5187	0.8641	10.229	8.061	47.811
19.70	275.971	0.000	6.607	16.614	198.3256	0.9034	10.579	7.711	40.672
19.80	278.892	0.000	3.814	1.278	4.3194	0.4431	17.911	0.379	8.062

San Diego class tanker motions with spring
SEAWAY-4.12

Execution: 18-04-1994 / 18:44

TWO-PARAMETER LEWIS CONFORMAL MAPPING COEFFICIENTS

STATION	X-APP	HALF	DRAFT	AREA	M(S)	A(-1)	A(1)	A(3)	RMS
NUMBER	WIDTH	COEFF							TO LEWIS
(-)	(m)	(m)	(m)	(-)	(m)	(-)	(-)	(m)	
-6.71	-6.706	5.876	4.320	0.5652	4.4761	+1.0000	+0.1739	+0.1390	0.100
CONVENTIONAL									
-3.66	-3.658	7.396	5.084	0.5676	5.4934	+1.0000	+0.2104	+0.1359	0.121
CONVENTIONAL									
-0.61	-0.610	8.858	5.883	0.5706	6.5054	+1.0000	+0.2287	+0.1330	0.136
CONVENTIONAL									
0.50	2.438	9.869	6.664	0.5839	7.3483	+1.0000	+0.2181	+0.1250	0.222
CONVENTIONAL									
1.00	5.486	11.215	7.342	0.5966	8.3125	+1.0000	+0.2330	+0.1162	0.187
CONVENTIONAL									
2.00	8.534	12.920	8.697	0.5466	9.4092	+1.0000	+0.2244	+0.1487	0.356
CONVENTIONAL									
3.00	12.192	14.305	18.290	0.3463	12.7238	+1.0000	-0.1566	+0.2809	0.660
REENFRANT Cm:0.3591									
4.00	18.136	16.490	18.290	0.4820	14.5149	+1.0000	-0.0620	+0.1981	0.746
CONVENTIONAL									

1.00	+8.3004	+1.0000	+0.2436	+0.0968	-0.0083	+0.0210	-0.0020	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.067								
2.00	+9.3781	+1.0000	+0.2433	+0.1182	-0.0133	+0.0343	-0.0049	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.115								
3.00	+12.6285	+1.0000	-0.1523	+0.3011	-0.0159	-0.0105	+0.0104	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.443								
4.00	+14.7506	+1.0000	-0.0998	+0.2053	+0.0407	-0.0264	-0.0019	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.301								
5.00	+16.5655	+1.0000	-0.0562	+0.1305	+0.0642	-0.0199	-0.0014	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.173								
6.00	+19.2021	+1.0000	+0.0216	+0.0350	+0.0614	+0.0007	+0.0002	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.102								
7.00	+21.4104	+1.0000	+0.0790	-0.0291	+0.0417	+0.0071	+0.0031	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.157								
8.00	+23.5495	+1.0000	+0.1213	-0.0900	+0.0185	+0.0048	-0.0018	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.066								
9.00	+24.8596	+1.0000	+0.1433	-0.1269	-0.0003	+0.0034	-0.0022	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.047								
10.00	+25.3491	+1.0000	+0.1472	-0.1475	-0.0106	+0.0073	+0.0017	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.036								
11.00	+25.4068	+1.0000	+0.1473	-0.1502	-0.0117	+0.0080	+0.0022	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.047								
12.00	+25.4068	+1.0000	+0.1473	-0.1502	-0.0117	+0.0080	+0.0022	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.047								
13.00	+25.4068	+1.0000	+0.1473	-0.1502	-0.0117	+0.0080	+0.0022	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.047								
14.00	+25.4068	+1.0000	+0.1473	-0.1502	-0.0117	+0.0080	+0.0022	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.047								
15.00	+25.3995	+1.0000	+0.1474	-0.1498	-0.0118	+0.0079	+0.0023	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.044								
16.00	+25.3400	+1.0000	+0.1473	-0.1471	-0.0110	+0.0071	+0.0019	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.034								
17.00	+24.9161	+1.0000	+0.1412	-0.1348	-0.0051	+0.0058	+0.0009	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.055								
18.00	+23.6883	+1.0000	+0.1118	-0.1174	+0.0026	+0.0039	0.0000	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.038								
19.00	+21.0235	+1.0000	+0.0255	-0.0897	+0.0043	-0.0079	+0.0026	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.107								
19.50	+18.4766	+1.0000	-0.0736	-0.0703	+0.0027	-0.0040	+0.0068	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.083								
19.60	+14.8984	+1.0000	-0.2541	-0.0497	-0.0066	+0.0026	+0.0064	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.051								
19.70	+12.3611	+1.0000	-0.4066	-0.0603	-0.0057	-0.0004	+0.0075	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.033								
19.80	+2.1695	+1.0000	+0.6015	+0.1370	-0.0201	+0.0364	+0.0030	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.038								

San Diego class tanker motions with spring
SEAWAY-4.12

Execution: 18-04-1994 / 18:44

NATURAL ROLL AND COEFFICIENTS AT FIXED AMPLITUDE

FORWARD SHIP SPEED . (kn) : 0.00
 MEAN ROLL AMPLITUDE (deg) : 5.000

NATURAL ROLL PERIOD . (s) : 22.731
 NATURAL FREQUENCY . (r/s) : 0.276

LINEAR EQUIVALENT GM (m) : 4.005

MASS, k-phi-phi (m) : 22.672
 COMPONENTS k-phi-phi:
 SOLID MASS PART .. (m) : 20.981
 2-D POTENTIAL PART (m) : 8.591

DAMPING. kappa (-) : 0.0042
 COMPONENTS kappa:
 2-D POTENTIAL PART (-) : 0.0007
 SPEED EFFECT PART (-) : 0.0000
 SKIN FRICTION PART (-) : 0.0002
 EDDY MAKING PART . (-) : 0.0033
 LIFT MOMENT PART . (-) : 0.0000
 BILGE KEEL PART .. (-) : 0.0000

(NON)LINEAR DAMPING COEFFICIENTS:
 Kappa-1 (-) : 0.0001
 Kappa-2 (-) : 0.0394

NATURAL HEAVE AT ZERO FORWARD SPEED

NATURAL HEAVE PERIOD (s) : 11.542
 NATURAL FREQUENCY (r/s) : 0.544

NATURAL PITCH AT ZERO FORWARD SPEED

NATURAL PITCH PERIOD (s) : 10.633
 NATURAL FREQUENCY (r/s) : 0.591

San Diego class tanker motions with spring
 SEAWAY-4.12

Execution: 18-04-1994 / 18:44

FREQUENCY CHARACTERISTICS OF BASIC MOTIONS FORWARD SPEED = 0.00 kn
 ----- WAVE DIRECTION = +180 deg off stern

WAVE SQRT ENC ...SURGE... ...SWAY... ...HEAVE... ...ROLL... ...PITCH...
 ...YAW... ADDED RESISTANCES
 FREQ SL/WT. FREQ AMPL PHASE AMPL PHASE AMPL PHASE AMPL PHASE
 AMPL PHASE AMPL PHASE GER/BEU BOESE

1.067	2.269	1.067	0.011	108.8	0.000	76.5	0.003	325.1	0.000	295.0	0.009	119.0	0.000
130.1	2.04E+02	4.14E+00											
1.100	2.340	1.100	0.011	165.6	0.000	203.8	0.004	0.6	0.000	266.4	0.003	210.0	0.000
136.4	2.00E+02	7.40E-01											
1.133	2.411	1.133	0.009	211.3	0.000	219.0	0.002	27.0	0.000	232.4	0.006	267.1	0.000
281.7	1.97E+02	2.18E+00											
1.167	2.482	1.167	0.007	283.8	0.000	230.4	0.002	143.3	0.000	132.0	0.004	284.5	0.000
307.9	1.91E+02	1.84E+00											
1.200	2.553	1.200	0.008	340.9	0.000	31.5	0.003	171.8	0.000	83.9	0.002	32.2	0.000
307.0	1.90E+02	4.09E-01											
1.233	2.624	1.233	0.005	32.2	0.000	36.1	0.001	204.3	0.000	46.8	0.004	79.7	0.000
144.1	1.88E+02	1.35E+00											
1.267	2.695	1.267	0.005	120.7	0.000	28.4	0.001	328.3	0.000	305.4	0.002	102.4	0.000
129.0	1.85E+02	6.50E-01											
1.300	2.766	1.300	0.005	168.5	0.000	220.6	0.001	354.4	0.000	256.0	0.001	230.7	0.000
117.8	1.83E+02	3.75E-01											
1.333	2.837	1.333	0.003	242.9	0.000	214.2	0.000	55.0	0.000	207.1	0.002	264.2	0.000
323.8	1.81E+02	6.63E-01											
1.367	2.908	1.367	0.004	324.6	0.000	83.0	0.001	162.7	0.000	106.0	0.001	300.8	0.000
306.8	1.79E+02	1.38E-01											
1.400	2.979	1.400	0.003	17.7	0.000	39.9	0.001	187.2	0.000	65.0	0.001	80.6	0.000
253.6	1.77E+02	2.84E-01											
1.433	3.049	1.433	0.003	112.5	0.000	21.2	0.000	334.8	0.000	325.2	0.001	106.0	0.000
133.9	1.75E+02	9.79E-02											
1.467	3.120	1.467	0.003	180.5	0.000	225.2	0.001	10.2	0.000	252.0	0.000	230.9	0.000
111.8	1.74E+02	7.78E-02											
1.500	3.191	1.500	0.002	259.5	0.000	208.0	0.000	40.4	0.000	199.2	0.001	290.7	0.000
323.4	1.71E+02	-2.55E-03											
1.533	3.262	1.533	0.002	339.0	0.000	56.9	0.000	194.5	0.000	71.0	0.000	317.9	0.000
297.1	1.69E+02	5.15E-02											
1.567	3.333	1.567	0.002	67.4	0.000	28.6	0.000	202.9	0.000	32.4	0.001	107.3	0.000
164.8	1.67E+02	-4.85E-02											
1.600	3.404	1.600	0.002	135.3	0.000	269.1	0.000	13.2	0.000	255.2	0.001	116.1	0.000
118.6	1.65E+02	3.92E-02											
1.633	3.475	1.633	0.002	240.4	0.000	208.4	0.000	7.9	0.000	213.3	0.001	287.2	0.000
16.0	1.63E+02	-6.96E-02											
1.667	3.546	1.667	0.001	309.0	0.000	119.8	0.000	209.1	0.000	75.8	0.001	280.8	0.000
298.0	1.61E+02	3.21E-02											
1.700	3.617	1.700	0.005	221.1	0.000	26.5	0.000	184.2	0.000	28.9	0.000	121.4	0.000
209.2	1.58E+02	-5.55E-02											

San Diego class tanker motions with spring
SEAWAY-4.12

Execution: 18-04-1994 / 18:44

FREQUENCY CHARACTERISTICS OF MOTIONS POINTS FORWARD SPEED = 0.00 kn
----- WAVE DIRECTION = +180 deg off stern
POINT NR = 01
X-APP = 278.000 m
Y-CL = 0.000 m
Z-BL = 10.000 m

.....ABSOLUTE MOTIONS..... ..REL MOT..
WAVE SQRT ENC ..X..... ..Y..... ..Z..... ..Z.....

FREQ (r/s)	SL/WL (-)	FREQ (r/s)	AMPL (m/m)	PHASE (deg)	AMPL (m/m)	PHASE (deg)	AMPL (m/m)	PHASE (deg)	AMPL (m/m)	PHASE (deg)
0.200	0.452	0.200	1.053	90.3	0.000	120.3	1.138	30.4	0.193	188.7
0.233	0.510	0.233	0.947	90.6	0.000	125.6	1.207	36.4	0.300	190.6
0.267	0.574	0.267	0.860	91.1	0.000	122.6	1.293	42.4	0.454	192.7
0.300	0.641	0.300	0.774	91.9	0.000	148.6	1.389	47.9	0.664	194.9
0.333	0.710	0.333	0.677	93.3	0.000	151.3	1.479	52.4	0.934	197.3
0.367	0.780	0.367	0.563	95.3	0.000	156.1	1.534	55.9	1.241	200.0
0.400	0.851	0.400	0.435	98.3	0.000	161.1	1.527	58.3	1.551	203.0
0.433	0.922	0.433	0.300	103.3	0.000	166.0	1.434	59.9	1.809	206.6
0.467	0.993	0.467	0.168	113.7	0.000	171.2	1.242	60.8	1.950	211.2
0.500	1.064	0.500	0.069	156.1	0.000	177.3	0.954	62.5	1.903	218.1
0.533	1.135	0.533	0.091	234.2	0.000	186.9	0.600	65.9	1.659	229.0
0.567	1.206	0.567	0.146	252.7	0.000	212.9	0.190	95.2	1.204	250.5
0.600	1.277	0.600	0.155	257.3	0.000	293.1	0.385	178.0	1.154	298.6
0.633	1.347	0.633	0.113	264.0	0.000	331.6	0.539	169.4	1.555	323.2
0.667	1.418	0.667	0.059	292.8	0.000	345.3	0.388	155.7	1.486	338.0
0.700	1.489	0.700	0.049	357.0	0.000	355.7	0.112	143.0	1.096	8.1
0.733	1.560	0.733	0.061	32.6	0.000	13.5	0.104	319.5	1.011	58.0
0.767	1.631	0.767	0.058	55.5	0.000	68.9	0.196	314.0	1.194	97.3
0.800	1.702	0.800	0.042	84.6	0.000	132.8	0.174	312.4	1.230	129.1
0.833	1.773	0.833	0.030	133.4	0.000	155.8	0.085	317.6	1.098	166.7
0.867	1.844	0.867	0.031	182.8	0.000	175.5	0.022	66.0	1.025	214.0
0.900	1.915	0.900	0.030	217.1	0.000	222.3	0.068	112.0	1.079	260.9
0.933	1.986	0.933	0.022	256.5	0.000	294.4	0.069	121.1	1.096	304.4
0.967	2.057	0.967	0.017	316.2	0.000	324.2	0.034	140.4	1.041	351.6
1.000	2.128	1.000	0.018	3.5	0.000	353.7	0.019	239.0	1.026	43.4
1.033	2.198	1.033	0.014	43.7	0.000	76.5	0.031	279.5	1.045	95.0
1.067	2.269	1.067	0.010	107.7	0.000	124.7	0.022	302.2	1.030	147.4
1.100	2.340	1.100	0.011	164.4	0.000	150.1	0.010	17.7	1.016	203.2
1.133	2.411	1.133	0.009	207.2	0.000	238.3	0.016	79.8	1.024	260.1
1.167	2.482	1.167	0.006	283.7	0.000	299.9	0.012	110.3	1.016	318.1
1.200	2.553	1.200	0.008	339.7	0.000	321.6	0.006	195.9	1.010	18.7
1.233	2.624	1.233	0.005	28.4	0.000	75.3	0.009	253.9	1.014	80.6
1.267	2.695	1.267	0.005	121.7	0.000	124.1	0.006	291.3	1.008	144.0
1.300	2.766	1.300	0.005	167.0	0.000	142.9	0.004	33.0	1.006	209.5
1.333	2.837	1.333	0.003	241.0	0.000	291.3	0.005	82.0	1.008	276.4
1.367	2.908	1.367	0.004	325.1	0.000	309.8	0.002	134.6	1.004	345.1
1.400	2.979	1.400	0.003	15.6	0.000	355.9	0.003	246.7	1.005	55.6
1.433	3.049	1.433	0.002	112.9	0.000	124.9	0.003	290.1	1.004	127.5
1.467	3.120	1.467	0.003	180.1	0.000	134.9	0.001	29.1	1.002	201.4
1.500	3.191	1.500	0.002	257.9	0.000	300.4	0.002	107.1	1.004	276.9
1.533	3.262	1.533	0.002	339.5	0.000	307.2	0.001	157.8	1.002	353.9
1.567	3.333	1.567	0.002	65.8	0.000	116.7	0.002	277.6	1.003	72.9
1.600	3.404	1.600	0.002	136.1	0.000	122.4	0.001	311.1	1.002	153.3
1.633	3.475	1.633	0.002	238.8	0.000	308.2	0.001	89.9	1.002	235.7
1.667	3.546	1.667	0.001	310.4	0.000	298.9	0.001	111.3	1.002	319.5
1.700	3.617	1.700	0.005	221.6	0.000	223.0	0.001	272.4	1.001	45.2

San Diego class tanker motions with spring
SEAWAY-4.12

Execution: 18-04-1994 / 18:44

STATISTICS OF BASIC MOTIONS

FORWARD SPEED = 0.00 kn

 WAVE DIRECTION = +180 deg off stern

.....SEA.....	SIGNIFICANT VALUES OF BASIC MOTIONS.....													
	MEAN ADDED.....													
.....INPUT.....CALCULATED.....SURGE.....	SWAY.....	HEAVE.....	ROLL.....	PITCH.....	YAW.....	RESISTANCE.....	
HEIGHT PER AMPL	HEIGHT PER AMPL	AMPL PER GER/BEU	AMPL PER BOESE	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER	AMPL PER
(m)	(s)	(m)	(s)	(m)	(s)	(m)	(s)	(m)	(s)	(deg)	(s)	(deg)	(s)	(deg)	(s)
4.76	12.00	4.75	12.16	0.76	17.20	0.00	7.66	0.91	16.35	0.00	10.99	1.00	15.25	0.00	
10.32	536.1	242.7													
6.06	13.00	6.05	13.14	1.18	18.22	0.00	18.23	1.37	17.54	0.00	22.02	1.35	15.85	0.00	
15.87	717.1	344.9													
7.53	14.00	7.52	14.12	1.71	19.17	0.00	19.14	1.95	18.59	0.00	22.61	1.73	16.41	0.00	
16.45	906.9	454.8													
8.84	15.00	8.83	15.10	2.27	20.09	0.00	20.03	2.55	19.55	0.00	22.91	2.04	16.94	0.00	
17.02	1021.4	527.2													

San Diego class tanker motions with spring SEAWAY=4.12

Execution: 18-04-1994 / 18:44

STATISTICS OF MOTIONS IN POINTS

FORWARD SPEED = 0.00 kn
 WAVE DIRECTION = +180 deg off stern

 POINT NR = 01
 X-APP = 278.000 m
 Y-CL = 0.000 m
 Z-BL = 10.000 m

.....SIGNIFICANT VALUES

OF.....	DISPLACEMENTS.....											VELOCITIES.....					
	ACCELERATIONS.....																	
.....SEA.....X.....Y.....Z.....X.....Y.....Z.....X.....Y.....Z.....X.....Y.....Z.....X.....Y.....Z.....				
HEIGHT PER AMPL	HEIGHT PER AMPL	AMPL PER AMPL	AMPL PER AMPL	AMPL PER AMPL	AMPL PER AMPL	AMPL PER AMPL	AMPL PER AMPL	AMPL PER AMPL	AMPL PER AMPL	AMPL PER AMPL	AMPL PER AMPL	AMPL PER AMPL	AMPL PER AMPL	AMPL PER AMPL	AMPL PER AMPL				
(m)	(s)	(m)	(s)	(m)	(s)	(m)	(s)	(m/s)	(s)	(m/s)	(s)	(m/s)	(s)	(m/s ²)	(s)				
4.76	12.00	0.85	17.2	0.00	15.5	2.67	15.4	0.32	16.2	0.00	10.3	1.10	14.7	0.11	12.7				
0.00	6.28	0.48	13.8																
6.06	13.00	1.30	18.1	0.00	16.3	3.65	16.1	0.46	17.1	0.00	15.3	1.44	15.2	0.14	13.0				
0.00	6.28	0.60	14.3																
7.53	14.00	1.88	19.0	0.00	17.0	4.73	16.8	0.63	18.0	0.00	15.9	1.80	15.7	0.16	13.3				
0.00	9.25	0.73	14.7																
8.84	15.00	2.47	19.9	0.00	17.8	5.69	17.4	0.79	18.7	0.00	16.5	2.09	16.2	0.17	13.5				
0.00	16.1	0.82	15.1																

.....VERTICAL RELATIVE MOTIONS.....	SLAMMING DEFINED BY.....					
	SIGNIFICANT VALUES OF... EXCEEDING... ..BOW EMERGENCE AND....					
.....SEA.....DISPLACEMENT.....VELOCITY.....Z-BL.....VELOCITY.....PRESSURE.....PROB NR/H.....PROB NR/H.....
HEIGHT PER AMPL	AMPL PER AMPL	AMPL PER AMPL	PROB NR/H	PROB NR/H	PROB NR/H	PROB NR/H	PROB NR/H
(m)	(s)	(m)	(s)	(m/s)	(s)	(%)(1/h)	(%)(1/h)
4.76	12.00	3.54	12.58	5.03	13.08	0.0	0.0
6.06	13.00	4.34	13.05	6.15	13.39	0.1	0.2

7.53	14.00	5.12	13.44	7.19	13.62	0.5	1.5	0.0	0.0	1.5	11.9
8.84	15.00	5.65	13.77	7.85	13.79	1.3	3.6	0.0	0.0	3.2	24.6

San Diego Light Ship Environment Input File

4.12
 San Diego class tanker (light ship) motions with spring
 +1 +1 0 +1 0
 6.098 0.000 0.000 304.878 1.025E+00
 123456 1 6 +5 0
 1
 0.0
 1
 180.0
 2.500 1 0.200 1.700 0.033333
 1.474
 +16.800 +20.981 69.750 69.750
 0
 3
 5.000
 0.0 61.250 105.000
 0
 1
 278.00 0.0 0.0
 20.4 0.0 0.0
 -1
 278.000 0.000 10.000
 6
 +2
 3.00 12.00
 4.00 13.00
 5.00 14.00
 3.50 12.50
 4.50 13.50
 2.00 11.00
 0
 *** End of file ***

San Diego Light Ship Output File

```
#####
# Program: SEAWAY Journee #
# #
# STRIPTHEORY CALCULATIONS OF MOTIONS AND LOADS IN A SEAWAY #
# #
# Release 4.12 #
# (31-07-1993) #
#####
```

User: University of California, Berkeley, U.S.A.
 INPUT DATA

 San Diego class tanker (light ship) motions with spring

PRINT-CODE INPUT DATA KPR(1): 1
 PRINT-CODE GEOMETRIC DATA KPR(2): 1
 PRINT-CODE HYDRODYNAMIC COEFFICIENTS KPR(3): 0
 PRINT-CODE FREQUENCY CHARACTERISTICS KPR(4): 1
 PRINT-CODE SPECTRAL DATA KPR(5): 0

ACTUAL MIDSHIP DRAFT DRAFT : 6.098 m
 ACTUAL TRIM BY STERN TRIM : 0.000 m
 DUMMY VALUE, FOR THE TIME BEING DIST : 0.000

WATER DEPTH DEPTH : 304.9 m
 DENSITY OF WATER RHO : 1.025 ton/m³

DEGREES OF FREEDOM CODE MOT : 123456
 VERSION-CODE OF STRIP THEORY METHOD ... KTH : 1
 NUMBER OF TERMS IN POTENTIAL SERIES .. MSER : 6
 CODE OF USED 2-D APPROXIMATION KCOF : 5
 NUMBER OF "FREE-CHOICE" SECTIONS NFR : 0

NUMBER OF FORWARD SPEEDS NV : 1
 FORWARD SPEEDS (kn) VK(NV) : 0.00

NUMBER OF WAVE DIRECTIONS NWD : 1
 WAVE DIRECTIONS (deg off stern) WAVDIR(NWD) : 180.0

MAX. FREQ. OF ENCOUNTER IN SERIES . FREQMAX : 2.500 rad/sec (range = 0.000 - 3.125 rad/sec)

CODE FOR WAVE FREQUENCY INPUT KOMEQ : 1
 MINIMUM CIRCULAR WAVE FREQUENCY OMMIN : 0.200 rad/sec
 MAXIMUM CIRCULAR WAVE FREQUENCY OMMAX : 1.700 rad/sec
 INCREMENT IN WAVE FREQUENCIES OMINC : 0.033 rad/sec

WAVE AMPLITUDE FOR LINEARISATION ... WAVAMP : 1.474 m

INPUT DATA (continued)

 BASE LINE TO CENTRE OF GRAVITY ... +GKGM=KG : 16.800 m

MASS-GYRADIUS k-xx GYR(1) : 20.981 m
 MASS-GYRADIUS k-yy GYR(2) : 69.750 m
 MASS-GYRADIUS k-zz GYR(3) : 69.750 m

NUMBER OF LOAD-CALCULATION SECTIONS .. NBTM : 0

CODE OF ROLL DAMPING INPUT KRD : 3
 AVERAGE ROLL AMPLITUDE ROLAMP : 5.000 deg
 HEIGHT OF BILGE KEEL HBK : 0.000 m
 DISTANCE OF A.P.P. TO AFT END B.K. ... XBKA : 61.25 m
 DISTANCE OF A.P.P. TO FORWARD END B.K. XBKF : 105.00 m

CODE OF ANTI-ROLLING DEVICES KARD : 0

NUMBER OF LINEAR SPRINGS NCAB : 1

COORDINATES AND LINEAR SPRING COEFFICIENTS :278.000 0.000 0.000 2.040E+01
0.000E-01 0.000E-01

NUMBER OF DISCRETE POINTS NPTS : -1
COORDINATES OF POINTS (m) .. PTSXYZ(NPTS,3) : 278.00 0.00 10.00

NUMBER OF SEA STATES NSEA : 6
CODE OF IRREGULAR SEA DESCRIPTION KSEA : 2
WAVE HEIGHTS (m) HW(K) / PERIODS (s) TW(K) : 3.00 12.00
4.00 13.00
5.00 14.00
3.50 12.50
4.50 13.50
2.00 11.00

INPUT-CODE OF CRITERA FOR SHIPMOTIONS KRIT : 0

San Diego class tanker motions with spring
SEAWAY-4.12

Execution: 18-04-1994 , 18:53

GEOMETRICAL HULLFORM DATA

ACTUAL MIDSHIP DRAFT (T) : 6.098 m
ACTUAL TRIM BY STERN : 0.000 m

LENGTH BETWEEN PERPENDICULARS (Lpp) : 278.892 m
REAR SECTION TO A.P.P. : 6.706 m

WATERLINE : LENGTH (Lwl) : 285.598 m
BEAM (B) : 50.596 m
AREA : 11676 m²
AREA COEFFICIENT (Lpp) : 0.8274
AREA COEFFICIENT (Lwl) : 0.8080
CENTROID TO A.P.P. : 150.548 m (+11.102 m or +3.98 % Lpp/2)
CENTROID TO REAR SECTION : 157.254 m (+14.455 m or +5.06 % Lwl/2)

DISPLACEMENT : VOLUME : 68354 m³
BLOCKCOEFFICIENT (Lpp) : 0.7944
BLOCKCOEFFICIENT (Lwl) : 0.7757
CENTROID TO A.P.P. : 150.967 m (+11.521 m or +4.13 % Lpp/2)
CENTROID TO REAR SECTION .. : 157.673 m (+14.874 m or +5.21 % Lwl/2)
CENTROID TO WATERLINE : 2.984 m
CENTROID TO KEELLINE : 3.114 m
MIDSHIP SECTION COEFFICIENT : 0.9911
LONG. PRISMATIC COEFFICIENT : 0.8015
VERT. PRISMATIC COEFFICIENT : 0.9600
RATIO Lpp/B : 5.512
RATIO Lwl/B : 5.645
RATIO B/T : 8.297
WETTED SURFACE HULL : 13934 m²

STABILITY PARAMETERS

KB : 3.114 m
 KG : 16.800 m
 BM-TRANSVERSE . : 32.494 m
 GM-TRANSVERSE . : 18.808 m
 BM-LONGITUDINAL : 792.786 m
 GM-LONGITUDINAL : 779.100 m

San Diego class tanker motions with spring
 SEAWAY-4.12

Execution: 18-04-1994 , 18:53

SECTIONAL HULLFORM DATA

STATION NUMBER	X-APP (m)	HALF CL-CL (m)	HALF WIDTH (m)	HALF DRAFT (m)	DRAFT (m2)	AREA COEFF (-)	AREA (m)	AREA (m)	KB (m)	BO (m)	WETTED LENGTH (m)
-6.71	-6.706	0.000	0.003	0.003	0.0000	0.7500	6.097	0.001	0.009	0.009	0.009
-3.66	-3.658	0.000	0.003	0.003	0.0000	0.7500	6.097	0.001	0.009	0.009	0.009
-0.61	-0.610	0.000	0.003	0.003	0.0000	0.7500	6.097	0.001	0.009	0.009	0.009
0.50	2.438	0.000	0.003	0.003	0.0000	0.7500	6.097	0.001	0.009	0.009	0.009
1.00	5.486	0.000	0.003	0.003	0.0000	0.7500	6.097	0.001	0.009	0.009	0.009
2.00	8.534	0.000	0.003	0.003	0.0000	0.7500	6.097	0.001	0.009	0.009	0.009
3.00	12.192	0.000	1.441	6.098	11.9976	0.6825	3.738	2.360	12.694	12.694	12.694
4.00	18.136	0.000	4.837	6.098	46.6412	0.7906	3.389	2.709	17.625	17.625	17.625
5.00	24.232	0.000	8.016	6.098	80.9939	0.8285	3.304	2.794	23.183	23.183	23.183
6.00	34.900	0.000	13.320	6.098	138.5482	0.8528	3.247	2.851	32.687	32.687	32.687
7.00	45.568	0.000	17.992	6.098	191.0340	0.8706	3.218	2.880	41.506	41.506	41.506
8.00	59.284	0.000	22.502	6.098	250.4236	0.9125	3.173	2.925	50.882	50.882	50.882
9.00	73.000	0.000	24.877	6.098	289.3329	0.9536	3.123	2.975	57.119	57.119	57.119
10.00	86.716	0.000	25.298	6.098	304.2029	0.9860	3.081	3.017	60.122	60.122	60.122
11.00	100.432	0.000	25.298	6.098	305.7981	0.9911	3.072	3.026	60.647	60.647	60.647
12.00	127.864	0.000	25.298	6.098	305.7981	0.9911	3.072	3.026	60.647	60.647	60.647
13.00	155.296	0.000	25.298	6.098	305.7981	0.9911	3.072	3.026	60.647	60.647	60.647
14.00	182.728	0.000	25.298	6.098	305.7981	0.9911	3.072	3.026	60.647	60.647	60.647
15.00	210.160	0.000	25.298	6.098	305.5416	0.9903	3.074	3.024	60.607	60.607	60.607
16.00	223.876	0.000	25.295	6.098	303.8590	0.9850	3.085	3.013	60.047	60.047	60.047
17.00	237.592	0.000	24.912	6.098	292.7500	0.9635	3.117	2.981	57.684	57.684	57.684
18.00	249.022	0.000	22.965	6.098	262.1276	0.9359	3.160	2.938	52.595	52.595	52.595
19.00	260.452	0.000	18.592	6.098	199.7755	0.8811	3.247	2.851	42.707	42.707	42.707
19.50	266.852	0.000	14.457	6.098	146.7948	0.8325	3.338	2.760	34.120	34.120	34.120
19.60	272.948	0.000	9.196	5.793	79.2156	0.7435	3.656	2.442	23.210	23.210	23.210
19.70	275.971	0.000	6.024	4.422	41.0068	0.7697	4.160	1.938	16.207	16.207	16.207
19.80	278.892	0.000	0.003	0.003	0.0000	0.7500	6.097	0.001	0.009	0.009	0.009

San Diego class tanker motions with spring
 SEAWAY-4.12

Execution: 18-04-1994 / 18:53

TWO-PARAMETER LEWIS CONFORMAL MAPPING COEFFICIENTS

STATION NUMBER	X-APP	HALF DRAFT	AREA	M(S)	A(-1)	A(1)	A(3)	RMS		
HULLFORM REMARKS WITH REGARD TO LEWIS										
CONFORMAL MAPPING										
(-)	(m)	(m)	(m)	(-)	(m)	(-)	(-)	(-)	(m)	
-6.71	-6.706	0.003	0.003	0.7500	0.0025	+1.0000	+0.0000	+0.0225	0.001	
CONVENTIONAL										
-3.66	-3.658	0.003	0.003	0.7500	0.0025	+1.0000	+0.0000	+0.0225	0.001	
CONVENTIONAL										
-0.61	-0.610	0.003	0.003	0.7500	0.0025	+1.0000	+0.0000	+0.0225	0.001	
CONVENTIONAL										
0.50	2.438	0.003	0.003	0.7500	0.0025	+1.0000	+0.0000	+0.0225	0.001	
CONVENTIONAL										
1.00	5.486	0.003	0.003	0.7500	0.0025	+1.0000	+0.0000	+0.0225	0.001	
CONVENTIONAL										
2.00	8.534	0.003	0.003	0.7500	0.0025	+1.0000	+0.0000	+0.0225	0.001	
CONVENTIONAL										
3.00	12.192	1.441	6.098	0.6825	3.6227	+1.0000	-0.6427	+0.0406	0.058	
CONVENTIONAL										
4.00	18.136	4.837	6.098	0.7906	5.4855	+1.0000	-0.1149	-0.0033	0.193	
CONVENTIONAL										
5.00	24.232	8.016	6.098	0.8285	7.2523	+1.0000	+0.1322	-0.0270	0.302	
CONVENTIONAL										
6.00	34.900	13.320	6.098	0.8528	10.0827	+1.0000	+0.3581	-0.0371	0.382	
CONVENTIONAL										
7.00	45.568	17.992	6.098	0.8706	12.5611	+1.0000	+0.4734	-0.0411	0.411	
CONVENTIONAL										
8.00	59.284	22.502	6.098	0.9125	15.1240	+1.0000	+0.5423	-0.0545	0.263	TUNNELED
9.00	73.000	24.877	6.098	0.9536	16.6193	+1.0000	+0.5650	-0.0681	0.175	TUNNELED
10.00	86.716	25.298	6.098	0.9860	17.0732	+1.0000	+0.5623	-0.0805	0.373	TUNNELED
11.00	100.432	25.298	6.098	0.9911	17.1126	+1.0000	+0.5610	-0.0827	0.431	TUNNELED
12.00	127.864	25.298	6.098	0.9911	17.1126	+1.0000	+0.5610	-0.0827	0.431	TUNNELED
13.00	155.296	25.298	6.098	0.9911	17.1126	+1.0000	+0.5610	-0.0827	0.431	TUNNELED
14.00	182.728	25.298	6.098	0.9911	17.1126	+1.0000	+0.5610	-0.0827	0.431	TUNNELED
15.00	210.160	25.298	6.098	0.9903	17.1062	+1.0000	+0.5612	-0.0823	0.426	TUNNELED
16.00	223.876	25.295	6.098	0.9850	17.0641	+1.0000	+0.5625	-0.0801	0.368	TUNNELED
17.00	237.592	24.912	6.098	0.9635	16.7098	+1.0000	+0.5630	-0.0721	0.191	TUNNELED
18.00	249.022	22.965	6.098	0.9359	15.5225	+1.0000	+0.5433	-0.0638	0.094	TUNNELED
19.00	260.452	18.592	6.098	0.8811	12.9321	+1.0000	+0.4830	-0.0454	0.261	TUNNELED
CONVENTIONAL										
19.50	266.852	14.457	6.098	0.8325	10.5419	+1.0000	+0.3965	-0.0251	0.244	
CONVENTIONAL										
19.60	272.948	9.196	5.793	0.7435	7.3094	+1.0000	+0.2328	+0.0253	0.132	
CONVENTIONAL										
19.70	275.971	6.024	4.422	0.7697	5.1725	+1.0000	+0.1548	+0.0097	0.138	
CONVENTIONAL										
19.80	278.892	0.003	0.003	0.7500	0.0025	+1.0000	+0.0000	+0.0225	0.001	
CONVENTIONAL										

San Diego class tanker motions with spring SEAWAY-4.12

Execution: 18-04-1994 / 18:53

N-PARAMETER CLOSE-FIT CONFORMAL MAPPING COEFFICIENTS

19.60	+7.2776	+1.0000	+0.2124	+0.0228	+0.0125	+0.0069	+0.0089	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.027								
19.70	+5.1174	+1.0000	+0.1274	+0.0222	+0.0155	-0.0017	+0.0136	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.031								
19.80	+0.0031	+1.0000	-0.0203	-0.1787	+0.0203	+0.0000	+0.0000	+0.0000	+0.0000	+0.0000
+0.0000	+0.0000	0.000								

San Diego class tanker motions with spring
SEAWAY-4.12

Execution: 18-04-1994 / 18:53

NATURAL ROLL AND COEFFICIENTS AT FIXED AMPLITUDE

FORWARD SHIP SPEED (kn) : 0.00
MEAN ROLL AMPLITUDE (deg) : 5.000

NATURAL ROLL PERIOD (s) : 11.922
NATURAL FREQUENCY (r/s) : 0.527

LINEAR EQUIVALENT GM (m) : 18.858

MASS, k-phi-phi (m) : 25.802
COMPONENTS k-phi-phi:
SOLID MASS PART .. (m) : 20.981
2-D POTENTIAL PART (m) : 15.018

DAMPING, kappa (-) : 0.0261
COMPONENTS kappa:
2-D POTENTIAL PART (-) : 0.0227
SPEED EFFECT PART (-) : 0.0000
SKIN FRICTION PART (-) : 0.0003
EDDY MAKING PART . (-) : 0.0031
LIFT MOMENT PART . (-) : 0.0000
BILGE KEEL PART .. (-) : 0.0000

(NON)LINEAR DAMPING COEFFICIENTS:
Kappa-1 (-) : 0.0002
Kappa-2 (-) : 0.0371

NATURAL HEAVE AT ZERO FORWARD SPEED

NATURAL HEAVE PERIOD (s) : 9.211
NATURAL FREQUENCY (r/s) : 0.682

NATURAL PITCH AT ZERO FORWARD SPEED

NATURAL PITCH PERIOD (s) : 8.825

San Diego class tanker motions with spring
SEAWAY-4.12

Execution: 18-04-1994 / 18:53

FREQUENCY CHARACTERISTICS OF MOTIONS POINTS FORWARD SPEED = 0.00 kn
----- WAVE DIRECTION = +180 deg off stern

POINT NR = 01
X-APP = 278.000 m
Y-CL = 0.000 m
Z-BL = 10.000 m

	ABSOLUTE MOTIONS.....			..REL MOT..					
WAVE	SQRT	ENCX.....Y.....Z.....Z.....				
FREQ	SL/WL	FREQ	AMPL	PHASE	AMPL	PHASE	AMPL	PHASE	AMPL	
(r/s)	(-)	(r/s)	(m/m)	(deg)	(m/m)	(deg)	(m/m)	(deg)	(m/m)	
0.200	0.452	0.200	1.094	90.1	0.000	118.0	1.114	30.6	0.152	191.0
0.233	0.510	0.233	0.994	90.2	0.000	123.9	1.173	37.2	0.237	193.4
0.267	0.574	0.267	0.917	90.4	0.000	130.1	1.250	44.2	0.360	196.1
0.300	0.641	0.300	0.841	90.7	0.000	136.2	1.337	51.0	0.529	199.1
0.333	0.710	0.333	0.754	91.2	0.000	141.7	1.423	57.4	0.744	202.5
0.367	0.780	0.367	0.648	91.9	0.000	146.6	1.484	63.2	0.995	206.3
0.400	0.851	0.400	0.523	93.1	0.000	150.8	1.499	68.5	1.257	210.8
0.433	0.922	0.433	0.384	95.0	0.000	154.0	1.446	73.6	1.496	216.1
0.467	0.993	0.467	0.240	98.6	0.000	156.1	1.313	79.2	1.665	222.6
0.500	1.064	0.500	0.105	110.6	0.000	154.1	1.097	86.2	1.719	231.1
0.533	1.135	0.533	0.042	213.1	0.000	209.5	0.822	97.1	1.632	243.0
0.567	1.206	0.567	0.115	255.5	0.000	189.3	0.548	118.4	1.430	261.1
0.600	1.277	0.600	0.161	262.8	0.000	218.6	0.407	159.7	1.233	289.0
0.633	1.347	0.633	0.164	267.1	0.000	326.0	0.461	199.4	1.211	324.6
0.667	1.418	0.667	0.129	272.3	0.000	345.7	0.534	221.6	1.354	357.0
0.700	1.489	0.700	0.072	285.0	0.000	356.6	0.517	238.4	1.465	24.5
0.733	1.560	0.733	0.028	347.3	0.000	12.7	0.427	260.2	1.447	53.2
0.767	1.631	0.767	0.050	55.7	0.000	50.0	0.351	292.0	1.387	87.1
0.800	1.702	0.800	0.062	72.7	0.000	107.4	0.327	325.2	1.381	123.9
0.833	1.773	0.833	0.050	87.2	0.000	141.3	0.296	353.2	1.374	160.4
0.867	1.844	0.867	0.026	120.7	0.000	174.4	0.237	24.6	1.313	199.2
0.900	1.915	0.900	0.021	197.1	0.000	236.5	0.196	64.7	1.263	241.9
0.933	1.986	0.933	0.027	234.8	0.000	281.4	0.181	102.6	1.246	285.8
0.967	2.057	0.967	0.021	264.4	0.000	305.8	0.150	135.7	1.199	330.5
1.000	2.128	1.000	0.013	328.6	0.000	354.2	0.108	177.5	1.139	19.1
1.033	2.198	1.033	0.016	27.8	0.000	81.8	0.091	230.2	1.120	70.0
1.067	2.269	1.067	0.014	59.1	0.000	109.5	0.084	272.4	1.104	121.2
1.100	2.340	1.100	0.007	121.3	0.000	139.3	0.061	313.9	1.065	174.8
1.133	2.411	1.133	0.010	198.9	0.000	226.3	0.044	15.9	1.052	231.2
1.167	2.482	1.167	0.010	229.3	0.000	273.9	0.044	71.7	1.052	288.0
1.200	2.553	1.200	0.006	290.0	0.000	310.2	0.034	117.3	1.033	346.6
1.233	2.624	1.233	0.008	10.5	0.000	31.1	0.024	182.8	1.026	47.5
1.267	2.695	1.267	0.007	50.1	0.000	81.5	0.024	245.8	1.026	109.4
1.300	2.766	1.300	0.005	126.1	0.000	139.2	0.019	301.4	1.018	173.0
1.333	2.837	1.333	0.006	192.1	0.000	228.4	0.016	8.2	1.016	238.4
1.367	2.908	1.367	0.004	252.1	0.000	270.3	0.013	66.4	1.011	305.3
1.400	2.979	1.400	0.005	337.7	0.000	11.7	0.010	142.3	1.009	14.1
1.433	3.049	1.433	0.004	26.3	0.000	68.9	0.010	207.8	1.009	84.1

1.467	3.120	1.467	0.003	145.9	0.000	133.1	0.006	268.5	1.004	156.1
1.500	3.191	1.500	0.004	187.5	0.000	222.6	0.006	6.9	1.007	229.7
1.533	3.262	1.533	0.001	273.3	0.000	274.8	0.005	67.4	1.005	304.7
1.567	3.333	1.567	0.004	0.3	0.000	11.1	0.004	161.1	1.005	21.5
1.600	3.404	1.600	0.002	49.2	0.000	66.2	0.004	229.1	1.004	99.8
1.633	3.475	1.633	0.003	173.6	0.000	164.9	0.003	321.0	1.004	179.9
1.667	3.546	1.667	0.003	231.7	0.000	227.3	0.004	25.6	1.003	261.5
1.700	3.617	1.700	0.002	347.0	0.000	327.4	0.002	111.9	1.002	344.9

San Diego class tanker motions with spring
SEAWAY-4.12

Execution: 18-04-1994 / 18:53

STATISTICS OF BASIC MOTIONS

FORWARD SPEED = 0.00 kn
WAVE DIRECTION = +180 deg off stern

.....SEA.....	SIGNIFICANT VALUES OF BASIC MOTIONS.....												
.....INPUT.....	CALCULATED.....	SURGE.....	SWAY.....	HEAVE.....	ROLL.....	PITCH.....		
.....YAW.....		RESISTANCE		HEIGHT PER AMPL		HEIGHT PER AMPL		AMPL PER GER/BEU		AMPL PER BOESE		AMPL PER (kN)		
(m)	(s)	(m)	(s)	(m)	(s)	(m)	(s)	(deg)	(s)	(deg)	(s)	(deg)	(s)	(kN)
3.00	12.00	2.99	12.16	0.55	17.10	0.00	6.28	0.59	16.87	0.00	13.33	0.70	14.94	0.00
6.28	470.8		17.1											
4.00	13.00	3.99	13.14	0.88	18.05	0.00	9.49	0.94	17.84	0.00	13.67	0.97	15.52	0.00
7.43	659.5		26.5											
5.00	14.00	4.99	14.12	1.27	18.97	0.00	18.92	1.34	18.75	0.00	14.02	1.24	16.07	0.00
12.25	814.4		35.5											
3.50	12.50	3.49	12.65	0.71	17.58	0.00	6.28	0.76	17.37	0.00	13.50	0.84	15.24	0.00
6.28	568.7		21.8											
4.50	13.50	4.49	13.63	1.07	18.51	0.00	13.93	1.13	18.30	0.00	13.85	1.11	15.80	0.00
9.71	741.6		31.1											
2.00	11.00	1.99	11.19	0.29	16.03	0.00	6.28	0.31	15.76	0.00	12.98	0.43	14.32	0.00
6.28	265.1		8.6											

San Diego class tanker motions with spring
SEAWAY-4.12

Execution: 18-04-1994 / 18:53

STATISTICS OF MOTIONS IN POINTS

FORWARD SPEED = 0.00 kn
WAVE DIRECTION = +180 deg off stern

POINT NR = 01
X-APP = 278.000 m
Y-CL = 0.000 m
Z-BL = 10.000 m

.....SIGNIFICANT VALUES OF.....										
.....DISPLACEMENTS.....										
.....VELOCITIES.....										
.....ACCELERATIONS.....										
.....SEA.....X.....Y.....Z.....X.....Y.....Z.....X.....Y.....Z.....Z.....
HEIGHT PER AMPL	PER AMPL	PER AMPL	PER AMPL	PER AMPL	PER AMPL	PER AMPL	PER AMPL	PER AMPL	PER AMPL	PER AMPL
PER	AMPL	PER	AMPL	PER	AMPL	PER	AMPL	PER	AMPL	PER

(m)	(s)	(m)	(s)	(m)	(s)	(m)	(s)	(m/s)	(s)	(m/s)	(s)	(m/s)	(s)	(m/s ²)	(s)	(m/s ²)	(s)
3.00	12.00	0.62	17.0	0.00	15.6	1.72	15.0	0.23	16.0	0.00	6.28	0.73	14.0	0.06	11.5		
0.00	6.28	0.34	12.7														
4.00	13.00	0.98	17.9	0.00	16.3	2.42	15.7	0.35	16.9	0.00	7.87	0.99	14.6	0.08	11.7		
0.00	6.28	0.43	13.2														
5.00	14.00	1.40	18.8	0.00	17.0	3.13	16.4	0.47	17.7	0.00	12.6	1.22	15.1	0.09	11.9		
0.00	6.28	0.52	13.7														
3.50	12.50	0.79	17.4	0.00	15.9	2.07	15.3	0.29	16.5	0.00	6.28	0.86	14.3	0.07	11.6		
0.00	6.28	0.39	13.0														
4.50	13.50	1.18	18.3	0.00	16.6	2.78	16.0	0.41	17.3	0.00	10.1	1.11	14.9	0.08	11.8		
0.00	6.28	0.48	13.5														
2.00	11.00	0.33	16.0	0.00	8.62	1.05	14.3	0.13	15.0	0.00	6.28	0.47	13.3	0.04	11.3		
0.00	6.28	0.23	12.1														

.....VERTICAL RELATIVE MOTIONS.....				...SLAMMING DEFINED BY...				
..SIGNIFICANT VALUES OF...				.EXCEEDING.BOW EMERGENCE AND....				
.....SEA....	DISPLACEMENT	VELOCITY.Z-BL....	VELOCITY.	PRESSURE.	PROB NR/H	PROB NR/H	PROB NR/H
HEIGHT PER	AMPL PER	AMPL PER	AMPL PER	PROB NR/H	PROB NR/H	PROB NR/H	PROB NR/H	PROB NR/H
(m) (s)	(m) (s)	(m/s) (s)	(%) (1/h)	(%) (1/h)	(%) (1/h)	(%) (1/h)	(%) (1/h)	(%) (1/h)
3.00 12.00	2.02 11.91	2.14 11.55	0.1 0.2	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.0
4.00 13.00	2.56 12.36	2.68 11.97	1.0 2.9	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.1	
5.00 14.00	3.00 12.74	3.11 12.28	3.4 10.0	0.0 0.0	0.2 1.1			
3.50 12.50	2.30 12.14	2.42 11.77	0.3 1.0	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.0	
4.50 13.50	2.79 12.55	2.91 12.14	2.0 6.0	0.0 0.0	0.1 0.5			
2.00 11.00	1.40 11.36	1.49 10.98	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.0	0.0 0.0	

Appendix 5 : Calculations of Seismic Motions

Seismic motions were calculated with the use of the PCNSPEC seismic analysis program. The reader is referred to the PCNSPEC manual for a more detailed description of the program. Two input files are required by the program : a ground motion file and a system characteristics file. The characteristics for the SALM system, subjected to 2-, 10-, 100- and 1000-year peak accelerations are given below. The ground motion history is that of the El Centro earthquake, scaled to match the predicted peak ground accelerations.

Input File for 2-year Conditions

```
SALM SYSTEM, EL CENTRO=2-year quake, damping=0.33
2
1.00000, 0.3300,-0.0000, 1, 2, 2, 0, 1.00000, ...
1, 1, 0,
0.1000
7.42
330.0
12, ...
0.02, 0, 200
(8f10.0)
0.0287
```

Input File for 10-year Conditions

```
SALM SYSTEM, EL CENTRO=10-year quake, damping=0.33
2
1.00000, 0.3300,-0.0000, 1, 2, 2, 0, 1.00000, ...
1, 1, 0,
0.1000
7.42
66.0
12, ...
0.02, 0, 200
(8f10.0)
0.144
```

Input File for 100-year Conditions

```
SALM SYSTEM, EL CENTRO=100-year quake, damping=0.33
2
1.00000, 0.3300,-0.0000, 1, 2, 2, 0, 1.00000, ...
1, 1, 0,
0.1000
7.42
18.33
12, ...
```

0.02,0, 200
 (8f10.0)
 0.517

Input File for 1000-year Conditions

SALM SYSTEM, EL CENTRO=1000-year quake, w/damping=0.33
 2
 1.00000, 0.3300,-0.0000, 1, 2, 2, 1, 1.00000,....
 1, 1, 0,
 0.1000
 7.42
 8.25
 12.....
 0.02,0, 500
 (8f10.0)
 1.148

Input File for Ground Motions

-14	-108	-101	-88	-95	-120	-142	-128
-110	-85	-85	-131	-176	-194	-162	-144
-108	-82	-42	-66	-131	-190	-196	-66
30	141	-49	-128	-144	-203	-260	-325
-306	-172	-197	-163	-164	-67	25	150
236	252	336	463	492	419	359	271
235	339	412	530	639	732	652	599
400	400	63	-515	-787	-603	-484	-250
-59	134	308	499	710	995	1219	1529
1449	1155	935	892	926	839	901	993
1209	328	-1475	-2066	-1989	-2034	-1816	-1725
-1752	-1753	-1805	-1630	-1347	-1087	-782	-429
-17	360	785	1164	1598	1960	2412	2729
3036	3200	3417	2821	2324	-1198	-2373	-1640
-1865	-1095	-753	-173	113	533	895	1186
1757	576	-2631	-1547	-1729	-1012	-579	237
-670	-1980	-1641	-1685	-1481	-1231	-1001	-751
-523	-271	-44	188	-95	-433	-838	-951
-716	-599	-334	-108	185	420	673	-97
-372	-40	11	344	565	883	1130	1363
219	241	683	689	1318	1353	2040	-931
-1308	-692	-546	72	675	-1067	-1488	-1071
-1162	-762	-559	-215	-126	-674	-324	-337
-109	17	299	488	608	222	-32	-245
77	211	568	826	1206	1478	1737	421
29	259	293	-55	-147	143	206	499
645	957	1128	1447	1629	1945	1856	1984
1769	1250	-1207	-542	-384	-311	-1118	-1661
-2464	-2025	-1835	-1317	-960	-325	154	816
1319	1818	-58	-169	285	447	983	1424
1853	2456	1685	-1380	-999	-1089	-907	-469
-1250	-2111	-1617	-1692	-1306	-1111	-773	-510
-544	-1200	-1209	-1158	-1145	-717	-546	64
-804	-1634	-859	-961	-396	-147	319	648

876	472	198	-27	292	445	785	1033
1352	1606	1861	1281	640	204	314	373
496	235	-84	-168	-113	-229	-248	-157
-69	147	379	579	255	-41	-428	-133
95	230	-129	-50	80	210	380	510
157	-32	-111	5	76	35	-95	-36
-16	38	85	-56	-304	-421	-244	-236
-177	-129	-18	203	-108	-91	-34	-106
-111	-99	-2	73	235	355	705	779
184	-263	-124	-42	159	48	-219	-467
-428	-216	-43	159	320	419	123	-160
-204	-82	-206	-137	-55	53	134	266
232	79	-8	200	435	492	191	92
-22	-21	52	93	255	368	525	541
425	398	559	756	365	411	98	-204
-249	-405	-413	-471	-433	-458	-57	178
-208	-492	-530	-362	-405	-308	-316	-265
-265	-269	-345	-309	-217	-78	87	281
310	358	341	358	287	305	112	214
136	384	-861	-1349	-1342	-1354	-1193	-1042
-829	-651	-444	-258	-60	-91	-182	-147
85	163	50	264	582	867	1200	1695
1111	-1100	-366	-445	-236	-960	-656	-597
-670	-552	-27	378	1072	1669	947	408
667	132	-95	-520	-827	-1152	-1150	-803
-369	29	545	1178	1610	-270	34	-56
20	146	537	798	-205	-590	-169	-175
-28	74	382	567	753	801	592	304
23	64	-406	-451	-79	168	567	93
-55	44	-123	-282	-437	-352	-255	-111
205	519	854	1144	733	237	-368	-271
-217	-873	-973	-589	-336	77	259	508
361	81	-56	-209	-317	-238	-376	-550
-722	-803	-523	-340	-11	65	-37	-5
-168	-410	-80	79	374	615	665	254
-57	-474	-356	-243	-48	126	379	241
-227	-428	-679	-661	-590	-513	-408	-309
-266	-541	-628	-908	-1107	-881	-770	-582
-473	-333	-199	20	211	432	613	767
933	1066	1130	1187	1247	1334	1594	1797
2037	1236	442	-140	-666	-555	-693	-984
-1246	-1179	-1050	-920	-743	-809	-850	-860
-863	-873	-868	-885	-537	52	215	245
580	314	236	485	589	525	355	197
199	492	343	288	432	239	88	77
-148	-77	-19	75	44	-145	-316	-241
-28	182	426	439	512	466	479	193
222	274	393	504	577	588	822	797
949	345	45	-123	-347	-426	-416	-275
-270	74	428	-231	-387	-83	139	445
27	-697	-796	-251	-135	79	-115	-251
-333	-269	-301	-200	-67	-38	105	296
344	957	898	179	-362	-994	-807	-744
-539	-330	-128	31	148	508	-22	-489

-358	-691	-516	-371	88	632	841	1276
1388	1193	751	225	-88	-227	74	181
544	399	45	-82	-185	-20	6	-117
-210	-303	-512	-727	-579	-266	-178	40
98	137	221	437	91	-548	-555	-243
-81	250	410	182	-27	-243	-15	247
482	783	622	331	-14	-195	-247	-212
-110	50	241	-34	-216	-471	-363	-195
-18	170	-80	5	230	374	601	516
432	344	505	653	683	172	-170	-527
-664	-387	-222	-33	119	-128	-351	-514
-335	-218	-12	142	70	-63	-120	-322
-346	-91	73	309	472	603	576	330
-73	-777	-608	-438	-209	31	350	293
121	338	317	254	206	198	174	21
-144	-343	-339	-145	-28	170	-96	-255
-279	-388	-242	-215	-182	-174	-38	-27
-185	-123	87	343	695	910	853	760
513	186	15	-190	-151	-73	21	129
215	24	-124	-329	-519	-708	-579	-462
-307	-145	-9	-180	-318	-465	-391	-345
-316	-435	-491	-475	-420	-361	-277	-258
-139	-68	507	722	878	782	765	439
80	13	-126	-15	30	104	104	193
205	74	-56	-72	70	106	147	-9
-159	-187	-7	155	105	-115	-302	-309
-95	-58	4	20	50	57	97	134
177	218	261	302	346	386	474	393
238	115	-79	-124	54	27	-250	-566
-630	-591	-413	-68	272	277	-21	-60
-110	-221	-416	-519	-222	30	79	139
171	253	323	391	164	-136	-323	-291
-287	-304	-339	-245	-76	125	376	402
245	156	-40	-153	-289	-316	-111	94
335	576	424	143	-7	-135	-270	-341
-357	-396	-402	-488	-480	-406	-407	-351
-187	-57	44	-19	-72	-169	-115	126
358	654	716	762	739	628	484	264
-44	-288	-384	-492	-428	-416	-276	-52
237	426	604	452	284	126	-54	-275
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184	-48	-303	-531	-708	-928	-863	-631
-376	87	309	589	614	385	351	311
246	19	-198	-158	-15	99	286	408
563	531	314	165	-24	-189	-276	-371
-450	-534	-483	-379	-296	-196	-184	-159
-119	-53	2	59	-23	-112	-205	-322
-388	-341	-287	-328	-407	-487	-563	-644
-555	-446	-8	253	411	644	579	474
384	385	340	357	8	-254	-460	-471
-222	-65	164	355	504	371	280	158
41	-27	21	44	99	135	94	58
22	-31	-31	4	-25	-124	-235	-406
-530	-701	-323	-52	94	328	478	509

358	342	306	285	263	168	-5	-212
-365	-310	-297	-280	-237	-266	-308	-366
-357	-308	-193	19	196	160	129	140
110	108	92	89	19	-131	-247	-436
-432	-300	-192	-48	97	168	148	173
78	-58	-215	-234	60	262	269	84
-41	-227	-76	31	182	310	479	459
166	-58	-396	-444	-241	-101	217	261
121	8	-168	-382	-566	-781	-619	-256
44	456	783	1103	953	489	122	-387
-846	-1226	-864	-616	-228	150	477	323
248	95	227	338	505	594	519	552
595	617	593	466	158	-77	-230	-303
-329	-364	-490	-660	-723	-784	-846	-566
-209	128	381	488	363	250	158	377
594	630	370	217	40	23	-62	-286
-380	-457	-505	-777	-628	-189	189	509
744	749	562	455	525	635	731	851
927	975	929	814	264	-289	-901	-1364
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-380	-347	-20	180	633	1013	1122	1107
940	805	608	508	206	-201	-572	-1021
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577	562	464	300	107	-4	-33	-69
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155	-19	-95	-216	-167	-120	-64	-129
-163	-193	-242	-236	-175	-124	-185	-265
-323	-336	-454	-430	-334	-213	-69	30
3	-93	-89	-150	-164	-238	-323	-421
-457	-397	-349	-258	-172	-20	156	284
362	354	269	101	-45	-125	-245	-229
-126	-68	18	93	200	286	365	311
181	24	-156	-319	-219	-118	15	153
298	243	136	102	24	-10	-23	-36
-48	-89	-155	-148	-76	0	-20	-148
-225	-374	-365	-251	-164	-14	153	302
393	406	385	329	183	125	70	-1
-68	-130	-128	-115	-102	-84	-141	-205
-269	-350	-359	-297	-227	18	99	203
256	199	160	107	156	198	242	141
55	-66	-97	-26	16	100	94	39
0	-66	-99	-109	-115	-131	-168	-199
-104	-10	55	129	200	199	169	143
109	128	152	137	117	97	60	-28
-50	-84	-178	-321	-420	-518	-472	-394
-290	-110	50	132	157	177	195	255
331	328	252	173	45	-85	-190	-229
-304	-277	-227	-174	-121	-126	-129	-80
-26	39	65	55	50	64	126	179
243	307	298	251	216	163	192	234
282	301	196	106	-39	-163	-322	-335
-219	-148	-10	11	-53	-101	-126	-143

-129	-104	-71	-18	33	85	158	239
319	341	318	213	73	9	-15	-47
-75	-121	-156	-116	-56	-6	-3	-3
14	46	72	86	98	124	147	172
200	256	317	287	231	105	-1	-11
-36	-53	-83	-52	-7	37	96	155
205	143	73	21	-22	-70	-100	-75
-54	-29	-15	0	-1	-6	-11	-10
-3	1	16	53	86	126	154	131
102	56	6	-40	-98	-96	-46	-7
31	45	68	52	45	18	-2	-29
-28	-17	-6	-5	-17	-22	-36	-32
-7	14	41	65	63	52	6	-45
-76	-64	-65	-107	-161	-174	-117	-70
-36	-25	-2	4	36	82	115	70
38	-10	-25	-34	-41	-45	-65	-120
-117	-108	-98	-81	-76	-110	-138	-166
-144	-130	-111	-108	-98	-98	-63	-2
54	110	152	197	170	140	103	64
10	-67	-105	-122	-138	-166	-215	-269
-274	-217	-178	-116	-61	0	54	99
144	187	232	262	255	227	171	127
54	-47	-149	-210	-254	-252	-245	-226
-179	-136	-82	-14	44	89	97	100
100	104	136	121	100	72	19	-28
-83	-131	-190	-194	-139	-100	-44	-44
-40	-45	-49	-39	-21	-22	-16	-16
-12	-12	28	68	115	96	56	17
-31	-78	-92	-92	-95	-92	-86	-52
-22	14	38	34	34	29	40	54
67	85	87	77	70	61	75	87
102	111	90	75	53	62	79	97
118	135	105	77	44	12	-21	-42
-68	-87	-120	-159	-198	-233	-258	-270
-278	-287	-278	-270	-258	-251	-219	-173
-129	-109	-105	-95	-91	-72	-59	-40
-23	18	35	42	48	43	43	38
36	32	47	63	85	110	134	160
181	173	168	159	147	108	68	32
9	19	28	40	54	69	95	117
143	143	125	122	124	126	130	133
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18	43	76	103	133	101	62	18
-27	-80	-89	-54	-30	6	12	22
27	32	48	69	89	112	133	140
137	140	135	139	102	30	-32	-63
-85	-112	-157	-197	-188	-182	-170	-156
-142	-129	-125	-120	-116	-114	-106	-64
-21	2	-4	-4	-14	-19	-8	11
27	50	53	42	30	11	-5	-24
-20	-7	4	19	33	48	57	65
74	81	91	127	169	144	105	70
35	2	-32	-64	-95	-91	-83	-73
-51	-30	-4	33	52	44	40	30

20	10	1	-10	-14	2	16	30
23	16	16	26	32	45	46	21
-3	-30	-60	-87	-116	-107	-94	-78
-68	-63	-57	-50	-41	-33	-24	-14
-5	4	13	21	1	-18	-40	-64
-87	-110	-129	-151	-165	-174	-184	-194
-202	-213	-198	-173	-135	-65	-14	20
55	82	89	102	93	80	61	33
3	-24	-57	-45	-13	13	33	48
54	58	62	65	69	72	75	77
73	69	64	58	54	48	61	79
96	114	126	139	131	123	114	103
87	72	56	40	24	7	4	8
9	23	42	52	61	72	79	89
95	95	93	89	25	0	-5	-20
-24	-33	-48	-64	-80	-97	-114	-128
-106	-89	-66	-43	-19	-11	-9	-5
-4	5	14	24	35	43	44	39
13	-8	-36	-47	-55	-62	-68	-73
-80	-75	-64	-28	5	46	65	60
60	54	49	44	39	29	0	-26
-52	-42	-37	-25	-15	-2	2	0
-1	-4	-7	-6	-1	3	7	-1
-7	-15	-24	-29	-28	-28	-25	-24
-21	-27	-44	-58	-76	-84	-82	-83
-75	-50	-28	-2	18	18	21	21
21	31	43	49	49	49	48	47
52	67	79	95	97	96	96	93
91	89	87	84	82	80	77	62
47	35	36	34	35	36	32	19
15	14	12	12	11	0	-10	-20
-25	-29	-33	-37	-48	-63	-77	-92
-107	-122	-138	-143	-120	-103	-78	-67
-63	-58	-56	-53	-50	-48	-45	-42
-39	-31	-26	-11	10	31	46	48
54	55	63	84	101	84	71	53
33	28	29	28	27	12	-1	-10
-11	-16	-15	-31	-68	-99	-102	-105
-106	-99	-89	-79	-74	-72	-68	-67
-63	-40	-13	13	43	71	99	86
63	41	13	-14	-35	-37	-44	-46
-49	-51	-54	-56	-58	-46	-29	-13
7	17	19	25	25	35	50	64
80	94	97	102	105	108	110	104
99	93	86	79	72	63	55	46
38	31	27	21	17	12	7	3
8	13	19	22	9	-3	-17	-31
-47	-53	-48	-43	-38	-25	-9	7
17	22	35	61	83	112	107	87
71	47	27	4	-16	-24	-33	-46
-69	-84	-88	-93	-95	-97	-99	-101
-102	-98	-95	-90	-87	-89	-91	-93
-96	-97	-88	-81	-71	-68	-68	-67
-67	-66	-66	-64	-56	-50	-43	-34

-23	-13	-2	10	17	9	3	-5
-14	-7	6	16	13	12	8	5
0	2	9	14	21	22	10	0
-12	-24	-24	-19	-16	-18	-22	-27
-30	-26	-22	-22	-32	-41	-51	-46
-43	-38	-32	-26	-20	-19	-18	-15
13	41	67	70	76	78	80	81
84	78	61	48	31	29	30	32
35	37	24	7	-8	-28	-35	-22
-13	1	3	1	-1	-5	-8	-11
-15	-12	-8	-4	1	5	8	8
9	10	22	33	46	59	72	79
62	50	32	16	-4	0	30	52
87	82	31	-2	-62	-28	58	130
221	185	94	91	77	90	16	-84
-200	-148	-7	107	99	48	3	-69
-107	-15	69	138	67	20	35	90
86	52	24	16	13	-11	-34	-67
-54	-20	14	48	97	58	-48	-119
-95	-60	-24	32	38	14	-1	-7
6	10	24	13	4	-8	-27	-55
-84	-93	-61	-36	-2	33	37	-14
-45	-103	-77	-23	31	41	26	13
15	61	73	74	76	73	71	57
38	21	-2	-33	-64	-68	-51	-54
-54	-57	-33	0	33	70	96	85
80	68	75	77	85	61	37	7
-1	4	5	-10	-29	-46	-69	-65
-44	-28	-7	-23	-33	-51	-60	-56
-54	-50	-66	-91	-114	-140	-138	-140
-121	-96	-74	-46	-34	-42	-45	-55
-63	-72	-80	-73	-52	-37	-26	-14

Output File for 2-year Conditions

PCNSPEC

A PROGRAM
BY
R. BOROSCHEK

A MODIFIED VERSION OF
NONSPEC
BY
S. A. MAHIN

ASSISTED BY
R. HERRERA

J. LIN

JULY 10, 1991

NOTE: INPUT FILE WITH FIRST LINE CONTAINING THE WORD "HELP"
TO OBTAIN INPUT FILE FORMAT

LIKE pdspec < foo.file

AND foo.file : help

SALM SYSTEM, EL CENTRO=2-year quake, w/damping=0.33

ANALYSIS TYPE : 2
 1-SINGLE STRUCTURE
 2-SPECTRUM REPOSE

SYSTEM MASS : 1.00000
 SYSTEM DAMPING : .50000
 POST YIELD STIFF FACTOR . : .00000
 ELEMENT TYPE : 1
 1-BILINEAR MODEL
 2-DEGRADING MODEL

PROPERTY PARAMETER : 2
 1-STIFFNESS
 2-PERIOD

YIELD PARAMETER : 2
 1-YIELD DISPL.
 2-ETA VALUE
 3-TARGET

P-DELTA EFFECT : 0
 0-ONLY POST YIELD
 1-FULL EFFECT

YIELD PARAMETER SCALE ... : 1.00000

NUMBER OF STIFF/PERIODS . : 1
 NUMBER OF YIELD DISP/ETA : 1
 COMBINATION OF RESULTS .. : 0
 0-TOTAL COMBINATION
 1-ONE-TO-ONE COMBINATION

MAXIMUM TIME STEP : .10000

STIFF/PERIOD VALUES :
): 1): 7.4200

YIELD DISPL/ETA/TARGET VALUES :
): 1): 330.0000

LOAD TYPE : 12
 INPUT FUNCTION TIME STEP. : .02000
 INPUT FUNCTION TYPE : READ OTHERGM
 INPUT FUNCTION LENGTH ... : 200
 INPUT FUNCTION FORMAT ... : (8f10.0)
 INITIAL VELOCITY : .00000
 INITIAL DISPLACEMENT : .00000
 LOAD FACTOR : .02870

ELEMENT PROPERTY SUMMARY :

stiffness	natural frequency	period	yield displacement	yield shear
.717054E+00	.846790E+00	.742000E+01	.451324E+05	.323624E+05

EXECUTION STARTS
 GROUND MOTION MAXIMA

	ground displacement	ground velocity	ground acceleration
maximum	.4358E+01	.1094E+02	.9807E+02
time	2.6600	2.1800	2.1200
minimum	-.1379E+01	-.3872E+01	-.7551E+02
time	1.0400	2.9400	2.4400

ENERGY MAXIMA

	input energy	kinetic energy	recoverable strain energy	hysteretic energy	damping energy
maximum	.4701E+02	.7238E+01	.3966E+01	.0000E+00	.4614E+02
time	3.9600	2.6400	2.5800	.0000	3.9800

RELATIVE RESPONSE MAXIMA

	displacement	velocity	acceleration	resistance
maximum	1.5609	6.8683	81.9621	1.1192
time	3.9600	2.9400	2.4400	3.9600
minimum	-3.3261	-9.3652	-91.9137	-2.3850
time	2.5800	2.1800	2.1200	2.5800

ABSOLUTE RESPONSE MAXIMA

displacement	velocity	acceleration
--------------	----------	--------------

maximum	9.1455	3.8048	3.7393
time	2.1800	2.6400	3.9800
minimum	-4.6384	-1.0930	-.8424
time	2.9600	1.0200	1.5400

DUCTILITY ENVELOPES

maximum positive ductility ratio	:	.0000
minimum negative ductility ratio	:	-.0001
cyclic ductility ratio	:	1.0000
accumulative ductility ratio	:	1.0000
normalized hysteretic energy	:	1.0000

RESPONSE ENVELOPES

number of positive yield excursions	:	0
number of negative yield excursions	:	0
number of yield reversals	:	0
number of zero crossings	:	2
residual displacement	:	-.1292E-15

EXECUTION ENDS

WRITING RESULTS

Output File for 10-year Conditions

PCNSPEC

A PROGRAM
BY
R. BOROSCHEK

A MODIFIED VERSION OF
NONSPEC
BY
S. A. MAHIN

ASSISTED BY
R. HERRERA
J. LIN

JULY 10, 1991

NOTE: INPUT FILE WITH FIRST LINE CONTAINING THE WORD "HELP"
TO OBTAIN INPUT FILE FORMAT
LIKE pdspec < foo.file
AND foo.file : help

 SALM SYSTEM, EL CENTRO=10-year quake, w/damping=0.33

ANALYSIS TYPE : 2
 1-SINGLE STRUCTURE
 2-SPECTRUM REPONSE

SYSTEM MASS : 1.00000
 SYSTEM DAMPING : .33000
 POST YIELD STIFF FACTOR .. : .00000
 ELEMENT TYPE : 1
 1-BILINEAR MODEL
 2-DEGRADING MODEL

PROPERTY PARAMETER : 2
 1-STIFFNESS
 2-PERIOD

YIELD PARAMETER : 2
 1-YIELD DISPL.
 2-ETA VALUE
 3-TARGET

P-DELTA EFFECT : 0
 0-ONLY POST YIELD
 1-FULL EFFECT

YIELD PARAMETER SCALE ... : 1.00000

NUMBER OF STIFF/PERIODS .. : 1
 NUMBER OF YIELD DISP/ETA .. : 1
 COMBINATION OF RESULTS ... : 0
 0-TOTAL COMBINATION
 1-ONE-TO-ONE COMBINATION

MAXIMUM TIME STEP : .10000

STIFF/PERIOD VALUES :

) : 1): 7.4200

YIELD DISPL/ETA/TARGET VALUES :

) : 1): 66.0000

LOAD TYPE : 12

INPUT FUNCTION TIME STEP. : .02000
 INPUT FUNCTION TYPE : READ OTHERGM
 INPUT FUNCTION LENGTH ... : 200

INPUT FUNCTION FORMAT ... : (8f10.0)

INITIAL VELOCITY : .00000

INITIAL DISPLACEMENT : .00000

LOAD FACTOR : .14400

ELEMENT PROPERTY SUMMARY :

stiffness	natural frequency	period	yield displacement	yield shear
.717054E+00	.846790E+00	.742000E+01	.452897E+05	.324752E+05

EXECUTION STARTS

GROUND MOTION MAXIMA

	ground displacement	ground velocity	ground acceleration
maximum	.2187E+02	.5490E+02	.4920E+03
time	2.6600	2.1800	2.1200
minimum	-.6917E+01	-.1943E+02	-.3789E+03
time	1.0400	2.9400	2.4400

ENERGY MAXIMA

	input energy	kinetic energy	recoverable strain energy	hysteretic energy	damping energy
maximum	.8878E+03	.1310E+03	.1270E+03	.0000E+00	.8530E+03
time	3.9600	2.7200	2.6000	.0000	3.9800

RELATIVE RESPONSE MAXIMA

	displacement	velocity	acceleration	resistance
maximum	7.9294	34.4778	407.0706	5.6858
time	3.9800	2.9400	2.4400	3.9800
minimum	-18.8176	-49.2111	-468.8034	-13.4932
time	2.6000	2.1800	2.1200	2.6000

ABSOLUTE RESPONSE MAXIMA

	displacement	velocity	acceleration
maximum	34.1961	16.1868	18.9277
time	2.1800	2.7200	3.9800
minimum	-16.5668	-4.5784	-3.7553
time	3.7000	1.0600	1.6000

DUCTILITY ENVELOPES

maximum positive ductility ratio : .0002
 minimum negative ductility ratio : -.0004
 cyclic ductility ratio : 1.0000
 accumulative ductility ratio : 1.0000
 normalized hysteretic energy : 1.0000

RESPONSE ENVELOPES

number of positive yield excursions : 0
 number of negative yield excursions : 0
 number of yield reversals : 0
 number of zero crossings : 2

residual displacement : .1019E-15

EXECUTION ENDS
 WRITING RESULTS

Output File for 100-year Conditions

PCNSPEC

A PROGRAM
 BY
 R. BOROSCHEK

A MODIFIED VERSION OF
 NONSPEC
 BY
 S. A. MAHIN

ASSISTED BY
 R. HERRERA
 J. LIN

JULY 10, 1991

NOTE: INPUT FILE WITH FIRST LINE CONTAINING THE WORD "HELP"
 TO OBTAIN INPUT FILE FORMAT
 LIKE pdspec < foo.file
 AND foo.file : help

SALM SYSTEM, EL CENTRO=100-year quake, w/damping=0.33

ANALYSIS TYPE : 2
 1-SINGLE STRUCTURE
 2-SPECTRUM REPOSE

SYSTEM MASS : 1.00000
 SYSTEM DAMPING : .33000
 POST YIELD STIFF FACTOR . : .00000
 ELEMENT TYPE : 1
 1-BILINEAR MODEL
 2-DEGRADING MODEL

PROPERTY PARAMETER : 2
 1-STIFFNESS
 2-PERIOD

YIELD PARAMETER : 2
 1-YIELD DISPL.
 2-ETA VALUE
 3-TARGET

P-DELTA EFFECT : 0
 0-ONLY POST YIELD
 1-FULL EFFECT

YIELD PARAMETER SCALE ... : 1.00000

NUMBER OF STIFF/PERIODS . : 1
 NUMBER OF YIELD DISP/ETA : 1
 COMBINATION OF RESULTS ... : 0
 0-TOTAL COMBINATION
 1-ONE-TO-ONE COMBINATION

MAXIMUM TIME STEP : .10000

STIFF/PERIOD VALUES :
) : 1: 7.4200

YIELD DISPL/ETA/TARGET VALUES :
) : 1: 18.3300

LOAD TYPE : 12

INPUT FUNCTION TIME STEP . : .02000
 INPUT FUNCTION TYPE : READ OTHERGM
 INPUT FUNCTION LENGTH ... : 200

INPUT FUNCTION FORMAT ... : (8f10.0)

INITIAL VELOCITY : .00000

INITIAL DISPLACEMENT : .00000

LOAD FACTOR : .51700

ELEMENT PROPERTY SUMMARY :

stiffness	natural frequency	period	yield displacement	yield shear
.717054E+00	.846790E+00	.742000E+01	.451592E+05	.323816E+05

EXECUTION STARTS
GROUND MOTION MAXIMA

	ground displacement	ground velocity	ground acceleration
maximum	.7851E+02	.1971E+03	.1767E+04
time	2.6600	2.1800	2.1200
minimum	-.2484E+02	-.6975E+02	-.1360E+04
time	1.0400	2.9400	2.4400

ENERGY MAXIMA

	input energy	kinetic energy	recoverable strain energy	hysteretic energy	damping energy
maximum	.1144E+05	.1689E+04	.1636E+04	.0000E+00	.1100E+05
time	3.9600	2.7200	2.6000	.0000	3.9800

RELATIVE RESPONSE MAXIMA

	displacement	velocity	acceleration	resistance
maximum	28.4689	123.7848	1461.4964	20.4137
time	3.9800	2.9400	2.4400	3.9800
minimum	-67.5603	-176.6814	-1683.1346	-48.4444
time	2.6000	2.1800	2.1200	2.6000

ABSOLUTE RESPONSE MAXIMA

	displacement	velocity	acceleration
maximum	122.7733	58.1150	67.9558
time	2.1800	2.7200	3.9800
minimum	-59.4794	-16.4377	-13.4825
time	3.7000	1.0600	1.6000

DUCTILITY ENVELOPES

maximum positive ductility ratio	:	.0006
minimum negative ductility ratio	:	-.0015
cyclic ductility ratio	:	1.0000
accumulative ductility ratio	:	1.0000
normalized hysteretic energy	:	1.0000

RESPONSE ENVELOPES

number of positive yield excursions	:	0
number of negative yield excursions	:	0

number of yield reversals : 0
 number of zero crossings : 2

 residual displacement : .2422E-14
 EXECUTION ENDS
 WRITING RESULTS

Output File for 1000-year Conditions

PCNSPEC

 A PROGRAM
 BY
 R. BOROSCIEK

A MODIFIED VERSION OF
 NONSPEC
 BY
 S. A. MAHIN

ASSISTED BY
 R. HERRERA
 J. LIN

JULY 10, 1991

NOTE: INPUT FILE WITH FIRST LINE CONTAINING THE WORD "HELP"
 TO OBTAIN INPUT FILE FORMAT
 LIKE pdspec < foo.file
 AND foo.file : help

SALM SYSTEM, EL CENTRO=1000-year quake, w/damping=0.33

ANALYSIS TYPE : 2
 1-SINGLE STRUCTURE
 2-SPECTRUM REPONSE

 SYSTEM MASS : 1.00000
 SYSTEM DAMPING : .33000
 POST YIELD STIFF FACTOR . : .00000
 ELEMENT TYPE : 1

```

1-BILINEAR MODEL
2-DEGRADING MODEL
PROPERTY PARAMETER ..... :      2
 1-STIFFNESS
 2-PERIOD
YIELD PARAMETER ..... :      2
 1-YIELD DISPL.
 2-ETA VALUE
 3-TARGET
P-DELTA EFFECT ..... :      1
 0-ONLY POST YIELD
 1-FULL EFFECT
YIELD PARAMETER SCALE ... :    1.00000

NUMBER OF STIFF/PERIODS .. :      1
NUMBER OF YIELD DISP/ETA :      1
COMBINATION OF RESULTS .. :      0
 0-TOTAL COMBINATION
 1-ONE-TO-ONE COMBINATION

MAXIMUM TIME STEP ..... :    .10000

STIFF/PERIOD VALUES :

): 1):      7.4200

YIELD DISPL/ETA/TARGET VALUES :

): 1):      8.2500

LOAD TYPE ..... :      12

INPUT FUNCTION TIME STEP. :    .02000
INPUT FUNCTION TYPE .... :    READ OTHERGM
INPUT FUNCTION LENGTH ... :      500

INPUT FUNCTION FORMAT ... :    (8f10.0)
INITIAL VELOCITY ..... :    .00000      f
INITIAL DISPLACEMENT .... :    .00000

LOAD FACTOR ..... :      1.14800

ELEMENT PROPERTY SUMMARY :

stiffness  natural  period  yield  yield
          frequency          displacement  shear

.717054E+00 .846790E+00 .742000E+01 .451324E+05 .323624E+05

EXECUTION STARTS .....
    
```

GROUND MOTION MAXIMA

	ground displacement	ground velocity	ground acceleration
maximum	.5973E+03	.4377E+03	.3923E+04
time	9.9800	2.1800	2.1200
minimum	-.5515E+02	-.2211E+03	-.3020E+04
time	1.0400	5.3800	2.4400

ENERGY MAXIMA

	input energy	kinetic energy	recoverable strain energy	hysteretic energy	damping energy	
maximum	.1061E+06	.9876E+04	.8069E+04	.0000E+00	.1026E+06	
time	9.2200	8.5800	2.6000	.0000	9.9800	

RELATIVE RESPONSE MAXIMA

	displacement	velocity	acceleration	resistance
maximum	63.2153	323.5411	3245.2570	45.3288
time	3.9800	5.3800	2.4400	3.9800
minimum	-150.0178	-392.3216	-3737.4052	-107.5709
time	2.6000	2.1800	2.1200	2.6000

ABSOLUTE RESPONSE MAXIMA

	displacement	velocity	acceleration
maximum	272.6186	140.5419	649.0404
time	2.1800	8.5800	9.9800
minimum	-169.3432	-36.5000	-29.9378
time	5.4000	1.0600	1.6000

DUCTILITY ENVELOPES

maximum positive ductility ratio	:	.0014
minimum negative ductility ratio	:	-.0033
cyclic ductility ratio	:	1.0000
accumulative ductility ratio	:	1.0000
normalized hysteretic energy	:	1.0000

RESPONSE ENVELOPES

number of positive yield excursions	:	0
number of negative yield excursions	:	0
number of yield reversals	:	0
number of zero crossings	:	6

residual displacement	:	.1811E-14
-----------------------	---	-----------

EXECUTION ENDS

WRITING RESULTS

Appendix 6 : Calculations of Fatigue

The following fatigue calculations are based on the relations given in Chapters 4 and 5. The calculations were carried out with the use of Microsoft Excel spreadsheets, which are reproduced here. Fatigue calculations were done for the tubular riser, wire rope, connections, articulations, and chain components.

Fatigue Reliability : Tubular Riser

Input Parameters, Mean Fatigue Life	
100 year wave height (Hfd, Ho)	46
Accumulated Fatigue Dam. (D)	1.00
Wave Period (sec)	15
Stress cycles (No)	2.10E+08

S-N m value	4.38
Biased K value	1.50E+12
Alpha	1.00
Stress range bias	0.80

Out - Mean Fatigue Life	
Mean Fatigue Life (yrs)	98034

Cd	0.3
Ck	0.73
Cb	0.5

Removing Bias from K	
Deviation in K	0.65
Unbiased K (S-N)	5.54E+12

Input Parameters, Stress Range Para.	
Rainflow correction (y(m))	1.00
Average freq of stress (1/yr)	2.10E+06
Epsilon 0	1.00
1+m/eps0	5.38
Gamma (1+m/eps0)	46.69
Largest stress in 100 years	21.12

Output	
Omega	1.50E+08

Reliability	
Sigma ln T	1.57
Median T (yrs)	9.80E+04
Service Life (yrs)	20
Beta	5.41
Probability of Fail	3.27E-08

Fatigue Design Stress	
Design Time Period	100
Fatigue Life F.S.	3
Yo	4.55E+09
SfD	63.36
SCF	3.00
Nom. Allow. D Stress	21.12

Fatigue Reliability : Wire Rope

Input Parameters, Mean Fatigue Life	
100 year wave height (Hfd, Ho)	46
Accumulated Fatigue Dam. (D)	1.00
Wave Period (sec)	15
Stress cycles (No)	2.10E+08

S-N m value	4.09
Biased K value	1.30E+10
Alpha	1.00
Stress range bias	0.80

Out - Mean Fatigue Life	
Mean Fatigue Life (yrs)	66820

Cd	0.3
Ck	0.73
Cb	0.5

Removing Bias from K	
Deviation in K	0.65
Unbiased K (S-N)	4.80E+10

Input Parameters, Stress Range Para.	
Rainflow correction (y(m))	1.00
Average freq of stress (1/yr)	2.10E+06
Epsilon 0	1.00
1+m/eps0	5.09
Gamma (1+m/eps0)	30.56
Largest stress in 100 years	7.99

Output	
Omega	1.79E+06

Reliability	
Sigma ln T	1.53
Median T (yrs)	6.68E+04
Service Life (yrs)	20
Beta	5.31
Probability of Fail	5.79E-08

Fatigue Design Stress	
Design Time Period	100
Fatigue Life F.S.	3
Yo	2.31E+09
S/D	23.96
SCF	3.00
Nom. Allow. D Stress	7.99

Fatigue Reliability : Connections

Input Parameters, Mean Fatigue Life	
100 year wave height (Hfd, Ho)	46
Accumulated Fatigue Dam. (D)	1.00
Wave Period (sec)	15
Stress cycles (No)	2.10E+08

S-N m value	3.74
Biased K value	1.79E+10
Alpha	1.00
Stress range bias	0.80

Out - Mean Fatigue Life	
Mean Fatigue Life (yrs)	42072.49

Cd	0.3
Ck	0.73
Cb	0.5

Removing Bias from K	
Deviation in K	0.65
Unbiased K (S-N)	6.62E+10

Input Parameters, Stress Range Para.	
Rainflow correction (y(m))	1.00
Average freq of stress (1/yr)	2.10E+06
Epsilon 0	1.00
1+m/eps0	4.74
Gamma (1+m/eps0)	18.82
Largest stress in 100 years	10.11

Output	
Omega	3.62E+06

Reliability	
Sigma ln T	1.477313
Median T (yrs)	4.21E+04
Service Life (yrs)	20
Beta	5.18
Probability of Fail	1.15E-07

Fatigue Design Stress	
Design Time Period	100
Fatigue Life F.S.	3
Yo	1.05E+09
SfD	30.34
SCF	3.00
Nom. Allow. D Stress	10.11

Fatigue Reliability : Articulations

Input Parameters, Mean Fatigue Life	
100 year wave height (Hfd, Ho)	46
Accumulated Fatigue Dam. (D)	1.00
Wave Period (sec)	15
Stress cycles (No)	2.10E+08

S-N m value	3.74
Biased K value	1.79E+10
Alpha	1.00
Stress range bias	0.80

Out - Mean Fatigue Life	
Mean Fatigue Life (yrs)	42072.49

Cd	0.3
Ck	0.73
Cb	0.5

Removing Bias from K	
Deviation in K	0.65
Unbiased K (S-N)	6.62E+10

Input Parameters, Stress Range Para.	
Rainflow correction (y(m))	1.00
Average freq of stress (1/yr)	2.10E+06
Epsilon 0	1.00
1+m/eps0	4.74
Gamma (1+m/eps0)	18.82
Largest stress in 100 years	10.11

Output	
Omega	3.62E+06

Reliability	
Sigma ln T	1.477313
Median T (yrs)	4.21E+04
Service Life (yrs)	20
Beta	5.18
Probability of Fail	1.15E-07

Fatigue Design Stress	
Design Time Period	100
Fatigue Life F.S.	3
Yo	1.05E+09
SfD	30.34
SCF	3.00
Nom. Allow. D Stress	10.11

Fatigue Reliability : Chain

Input Parameters, Mean Fatigue Life	
100 year wave height (Hfd, Ho)	46
Accumulated Fatigue Dam. (D)	1.00
Wave Period (sec)	15
Stress cycles (No)	2.10E+08

S-N m value	3.36
Biased K value	4.60E+09
Alpha	1.00
Stress range bias	0.80

Out - Mean Fatigue Life	
Mean Fatigue Life (yrs)	25460.46

Cd	0.3
Ck	0.73
Cb	0.5

Removing Bias from K	
Deviation in K	0.65
Unbiased K (S-N)	1.70E+10

Input Parameters, Stress Range Para.	
Rainflow correction (y(m))	1.00
Average freq of stress (1/yr)	2.10E+06
Epsilon 0	1.00
1+m/eps0	4.36
Gamma (1+m/eps0)	11.52
Largest stress in 100 years	8.23

Output	
Omega	1.41E+06

Reliability	
Sigma ln T	1.42
Median T (yrs)	2.55E+04
Service Life (yrs)	20
Beta	5.04
Probability of Fail	2.43E-07

Fatigue Design Stress	
Design Time Period	100
Fatigue Life F.S.	3
Yo	4.59E+08
SfD	24.68
SCF	3.00
Nom. Allow. D Stress	8.23

Cumulative Annual Probability of Failure

The cumulative annual probabilities of failure are given in Table A.6.1. These values were obtained by varying the service life in the fatigue reliability calculations for chain. The values are plotted in Figure 5.1.

Year	End-of-Year	
	Beta	Probability of Failure
2	6.66	4.46E-10
4	6.17	4.84E-09
6	5.89	1.86E-08
8	5.68	4.71E-08
10	5.53	9.56E-08
12	5.40	1.69E-07
14	5.29	2.72E-07
16	5.20	4.08E-07
18	5.11	5.82E-07
20	5.04	7.98E-07
22	4.97	1.06E-06
24	4.91	1.37E-06
26	4.85	1.73E-06
28	4.80	2.14E-06
30	4.75	2.61E-06

Table A.6.1 : Fatigue Reliability versus Service Life (Chain)

Appendix 7 : Calculations of CALM Line and Anchor Tensions

The calculations of CALM line and anchor tensions were carried out by Wei Ma, and the reader is referred to his report for a detailed discussion of this analysis [Ma, 1994]. A table of Offset versus Line and Anchor Tensions is provided in Table A.7.1..

Offset (ft) (feet)	Steady Force (pounds)	Line Tension (pounds)	Anchor Tension (pounds)
0	0	52907	19537
40	37812	67282	30735
80	78708	88156	47640
120	129952	117780	72652
160	198253	160205	109855
200	293530	222052	165910
240	431631	314840	252353

Table A.7.1 : CALM Offset versus Tension

Appendix 8 : Calculations of SALM Line and Anchor Pile Tensions

The design of the SALM system was carried out using Microsoft Excel spreadsheets, which are reproduced here. The SALM system consists of three major components : the buoy, the anchor leg, and the pile anchor. The anchor leg for this system is composed of three solid risers. The hoses were also examined in this design process. Based on the weight and buoyancy of all system components, a buoy draft could be determined. A pretension was then added by increasing the draft of the buoy. The effectiveness of a pretension at providing restoring force is calculated in the next section.

SALM Component Design

<i>Buoy</i>			
<i>Input</i>		<i>Output</i>	
Diameter (ft)	15	Volume (ft ³)	12369.66
Depth (ft)	70	Weight (LT)	47.35
Steel Volume Ratio (ft ³ /ft ³)	0.0175		
Steel Density (kip/ft ³)	0.49		

<i>Top Tubular Riser (watertight)</i>			
<i>Input</i>		<i>Output</i>	
Diameter (in)	48	Steel Margin	3.20
Thickness (in)	0.5	Cross-section (in ²)	74.61
Length (ft)	300	Total Volume (ft ³)	3770
Weight/ft (LT/ft)	0.113	Weight (LT)	108.81
Steel Density (kip/ft ³)	0.49	Buoyancy (LT)	107.71
Steel Young's mod (kip/ft ²)	29000	Stiffness (LT/ft)	50.09
		Net Weight (LT)	1.10

<i>Middle Tubular Riser (watertight)</i>			
<i>Input</i>		<i>Output</i>	
Diameter (in)	48	Steel Margin	1.80
Thickness (in)	0.75	Cross-section (in ²)	111.33
Length (ft)	350	Total Volume (ft ³)	4398
Weight/ft (LT/ft)	0.169	Weight (LT)	106.54
Steel Density (kip/ft ³)	0.49	Buoyancy (LT)	125.66
Steel Young's mod (kip/ft ²)	29000	Stiffness (LT/ft)	64.06
		Net Weight (LT)	-19.12

<i>Lower Tubular Riser (watertight)</i>			
<i>Input</i>		<i>Output</i>	
Diameter (in)	48	Steel Margin	1.40
Thickness (in)	1	Cross-section (in ²)	147.65
Length (ft)	320	Total Volume (ft ³)	4021
Weight/ft (LT/ft)	0.224	Weight (LT)	100.48
Steel Density (kip/ft ³)	0.49	Buoyancy (LT)	114.89
Steel Young's mod (kip/ft ²)	29000	Stiffness (LT/ft)	92.92
		Net Weight (LT)	-14.40

<i>Hoses (submerged sections only)</i>			
<i>Input</i>		<i>Output</i>	
Outer Diameter (in)	24	Cross-section (in ²)	36.91
Thickness (in)	0.5	Steel Vol (ft ³)	281.97
Length (ft)	1100	Steel Wt (LT)	15.42
Steel Density (kip/ft ³)	0.49	Buoyancy (LT)	8.06
Steel % in Volume	25	Net Weight (LT)	7.36

Buoyancy refers only to solid hose, not interior

<i>Pile Anchor</i>			
<i>Input</i>		<i>Output</i>	
Length (ft)	140	Weight (LT)	77
Diameter (ft)	5	Holding Power (LT)	2001
Soil U.S.S (ksf)	2.0	Min.Thickness (in)	0.84
		Thickness (in)	1.00

SALM System Design

<i>No Pretension in System</i>		<i>Pretension Added</i>	
<i># Hoses</i>		<i>2 Pretension Force (LT)</i>	<i>300</i>
Net Weight (LT)	29.66	Tension/tendon (LT)	329.66
Required Buoy Buoyancy (LT)	29.66	Elong : Pretension (ft)	0.000
Buoy Draft (ft)	5.87	Delta Buoy Draft (ft)	59.42
Buoy Freeboard (ft)	64.13	Buoy Draft (ft)	65.29
		Buoy Freeboard (ft)	4.71

With the system characteristics determined, the horizontal environmental force versus the horizontal restoring force could be iterated to find an equilibrium offset position. This procedure is given in the spreadsheets below for the SALM system without a moored tanker for the 2-year, 10-year and 100-year return period conditions. The procedure was also carried out for the SALM system with a connected tanker, for the tanker in both the full-load and lightship conditions. The procedure was then carried out given total offset (steady force offset plus peak oscillating offset), iterating to find forces and tensions based on equilibrium offset.

SALM System, 2-year Return Period Calculations

<i>Elongation</i>	
Tendon Angle (deg)	0.88
Environ. Force (LT)	4.20
Cable Tension (LT)	273.07
Tension/tendon (LT)	273.07
Elongation (offset) (ft)	5.452

<i>Added Buoyancy (with Elongation)</i>	
Tendon Angle (deg)	0.88
Offset (ft)	15.38
Del Draft (ft)	0.12
Del Draft, with Elong. (ft)	-5.33
Added Buoyancy (LT)	-26.93
Added Restoring (LT)	-0.41

<i>Pretension Restoring Force</i>	
Tendon Angle (deg)	0.88
Restoring Force (LT)	4.61

<i>Output</i>	
Tendon Angle (deg)	0.881
Offset (ft)	15.38
Buoy Draft (ft)	59.96
Buoy Freeboard (ft)	10.04
Tension/tendon (LT)	273.07
Vertical Stiffness (LT/ft)	50.09

<i>Input / Iteration</i>	
Environ. Force (LT)	4.2
Tendon Angle Gss (deg)	0.88128
Restoring Force (LT)	4.20
Force Differential	2.69E-08

SALM System, 10-year Return Period Calculations

<i>Elongation</i>	
Tendon Angle (deg)	1.71
Environ. Force (LT)	8.20
Cable Tension (LT)	274.57
Tension/tendon (LT)	274.57
Elongation (offset) (ft)	5.482

<i>Added Buoyancy (with Elongation)</i>	
Tendon Angle (deg)	1.71
Offset (ft)	29.88
Del Draft (ft)	0.45
Del Draft, with Elong. (ft)	-5.04
Added Buoyancy (LT)	-25.43
Added Restoring (LT)	-0.76

<i>Pretension Restoring Force</i>	
Tendon Angle (deg)	1.71
Restoring Force (LT)	8.96

<i>Output</i>	
Tendon Angle (deg)	1.711
Offset (ft)	29.88
Buoy Draft (ft)	60.26
Buoy Freeboard (ft)	9.74
Tension/tendon (LT)	274.57
Vertical Stiffness (LT/ft)	50.09

<i>Input / Iteration</i>	
Environ. Force (LT)	8.2
Tendon Angle Gss (deg)	1.71136
Restoring Force (LT)	8.20
Force Differential	9.33E-09

SALM System, 100-year Return Period Calculations

<i>Elongation</i>	
Tendon Angle (deg)	2.80
Environ. Force (LT)	13.60
Cable Tension (LT)	278.03
Tension/tendon (LT)	278.03
Elongation (offset) (ft)	5.551

<i>Pretension Restoring Force</i>	
Tendon Angle (deg)	2.80
Restoring Force (LT)	14.67

<i>Input / Iteration</i>	
Environ. Force (LT)	13.6
Tendon Angle Gss (deg)	2.80383
Restoring Force (LT)	13.60
Force Differential	4.54E-08

<i>Added Buoyancy (with Elongation)</i>	
Tendon Angle (deg)	2.80
Offset (ft)	48.98
Del Draft (ft)	1.20
Del Draft, with Elong. (ft)	-4.35
Added Buoyancy (LT)	-21.97
Added Restoring (LT)	-1.07

<i>Output</i>	
Tendon Angle (deg)	2.804
Offset (ft)	48.98
Buoy Draft (ft)	60.94
Buoy Freeboard (ft)	9.06
Tension/tendon (LT)	278.03
Vertical Stiffness (LT/ft)	50.09

Appendix 9 : Approximations to the Standard Normal Distribution

Several formulas have been proposed for approximating the standard normal distribution. A handy form of estimation was necessary in this paper due to the number of iterations run which made use of the standard normal distribution. Two approximations were examined, one by Abramowitz [Melchers, 1987] and one from [Bea, 1990]. These approximations are :

$$\Phi(\beta) = 1 - \left[\frac{1}{\beta(2\pi)^{1/2}} \right] \exp\left[-\frac{1}{2}\beta^2 \right] \quad (\text{Abramowitz})$$

$$\Phi(\beta) = 1 - 0.475 \exp(-\beta^{1.6}) \quad (\text{Bea})$$

The values calculated by the Abramowitz approximation were found to more closely approximate the standard normal distribution for the values of beta encountered in this project, and this approximation was used.

Appendix 10 : Calculations of Reliability

The calculation of reliability was carried out using Microsoft Excel spreadsheets. The relations used in the reliability analysis are detailed in Chapter 5. For the sake of conciseness, only one reliability calculation is presented here for each system, as only the load will vary for calculation of different return periods, and the results for other return periods can be found in Chapter 5.

Calculation of CALM Reliability

CALM Components

	Wire	Chain	Connect.	Anchor
Capacity (LT)	447	321	100	274
Variance on Capacity	0.1	0.1	0.1	0.4
Natural Log Deviation	0.100	0.100	0.100	0.385
Mean (2-yr) Tension (LT)	31.70	31.70	31.70	15.6
Ln Dev. Loading	0.115	0.115	0.115	0.115
Variance, Type II	0.100	0.100	0.100	0.100
Correlation	0.0	0.0	0.0	0.0
Beta	18.74	16.39	8.13	7.20
Pf	1.29E-78	1.12E-60	2.11E-16	3.06E-13

CALM System

	Number	Individual Beta	Individual Pf
Chain Anchor Legs, Upper	8	16.39	1.12E-60
Chain Anchor Legs, Lower	8	16.39	1.12E-60
Wire Rope Anchor Legs	8	18.74	1.29E-78
Connections	32	8.13	2.11E-16
Anchors	8	7.20	3.06E-13

<u>Pf of single leg</u>	
Rho =	0.569429
Pf =	3.06E-13
Beta =	8.13

<u>Pf of System (all legs in parallel)</u>	
Beta =	10.3
Pf =	3.5E-25

Calculation of Reliability for SALMSALM Components

	Riser	Joint	Pile
Capacity (LT)	700	600	2000
Variance on Capacity	0.1	0.1	0.4
Natural Log Deviation	0.100	0.100	0.385
Mean (2-yr) Tension (LT)	283.4	283.4	283.4
Ln Dev, Loading	0.015	0.015	0.015
Variance in Loading	0.015	0.015	0.015
Variance, Type II	0.100	0.100	0.100
Correlation	0.0	0.0	0.0
Beta	6.37	5.28	4.91
Pf	9.93213E-11	6.65106E-08	4.8291E-07

SALM System

	Number	Individual Beta	Individual Pf
Riser	3	6.37	9.93213E-11
Pile	1	4.91	4.8291E-07
Joints	4	5.28	6.65106E-08

Pf of system

Rho =	0.022007
Pf, Storm	4.8291E-07
Beta, Sum	5.16

Appendix 11 : SPMS evaluated by ABS Factors of Safety

The following is a list of single point moorings which have been classed by ABS [Jones, 1992].

Year Installed	Country/Location	System Name
1978	Abu Dhabi	Tropical Lion
1981	United States	Santa Ynez
1981	Phillipines	FPSO II
1981	Thailand	Erawan
1986	Nigeria	FPSO VI
1986	Indonesia	Kakap Natuna
1986	Colombia	Convenas
1988	Yemen	Safer
1989	China	Bo Hai You Yi Hao
1989	China	Nanghai Faxian
1990	China	Chang Qing Hao
1990	Indonesia	Anoa Natuna
1990	China	Ayer Biru
1990	Malaysia	Puteri Dulang
1992	Australia	NA
1992	Indonesia	Belida
1992	China	NA

Table A.11.1 : SPMS Evaluated by ABS

