

¹ **Supporting Information for “Channel-island
connectivity affects water exposure time distributions
in a coastal river delta”**

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8 **Introduction**

9 The Supplemental Information contains information regarding the determination of the
 10 horizontal eddy viscosity for use in the hydrodynamic model (Text S1), a derivation of
 11 the equations used to calculate exposure time distributions (text S2), the comparison of
 12 the hydrodynamic model with field measurements (Text S3), and the comparison of the
 13 mass-flux curves generated by particle tracers versus the advection-diffusion-based tracer
 14 (Text S4). The results from the field measurements are included (Table S1).

15 **Text S1 - Determination of horizontal eddy viscosity**

16 The horizontal eddy viscosity, ν_e , can be estimated for shallow flows as [Cea *et al.*,
 17 2007]:

$$18 \quad \nu_e = \frac{1}{6} \kappa u_* h \quad (S1)$$

19 where $\kappa = 0.41$ is the von Karman's constant, u_* is the bed friction velocity (m s^{-1}), and
 20 h is the local water depth (m). The bed friction velocity can be estimated by using the
 21 "law of the wall" logarithmic velocity profile:

$$22 \quad \frac{U}{u_*} = \frac{1}{\kappa} \ln \left(\frac{h}{ez_0} \right) \quad (S2)$$

23 where U is the depth-averaged velocity (m s^{-1}), e is the base of the natural log, and z_0 is
 24 the bed roughness. The bed roughness can be calculated as:

$$25 \quad z_0 = \frac{aD_{90}}{30} \quad (S3)$$

26 where $a = 2.85$ is an optimization coefficient and D_{90} is the grain size for which 90%
 27 is finer (m) [Wilcock, 1996]. Combining Eqs. S1-S3 and using typical velocities within
 28 the channel at WLD [Hiatt and Passalacqua, 2015] and measurements of D_{90} [Shaw and
 29 Mohrig, 2014], the horizontal eddy viscosity is $O(0.01 - 0.1) \text{ m}^2 \text{ s}^{-1}$. We select a value of
 30 $\nu_e = 0.01 \text{ m}^2 \text{ s}^{-1}$ for our model simulations.

31 **Text S2 - Derivation of exposure time distribution calculation**

32 Here we provide the full derivation for calculating exposure time distributions in the
 33 steady-state case. Some lines are repeated from the main text to maintain the flow of the
 34 derivation.

35 A generic travel time distribution calculated for a system at steady-state can be derived
 36 as follows. The differential travel time distribution for a pulse injection at $t = 0$ calculated
 37 at the domain boundary at time t is given as [e.g., Benjamin and Lawler, 2013]:

$$38 \quad E(t) = \frac{dN(t)/dt}{N_{Total}} \quad (1)$$

39 where $dN(t)/dt$ is the rate at which material exits the domain at time t and N_{Total} is the
 40 cumulative amount of material that has passed through the system at $t = \infty$. Integrating
 41 over time gives the cumulative travel time distribution:

$$42 \quad F(t) = \int_0^t E(\tau)d\tau \quad (2)$$

43 where τ is a dummy variable and $F(t = \infty) = 1$, rendering $E(t)$ and $F(t)$ a probabil-
 44 ity density function (pdf) and cumulative distribution function (cdf) of the travel time
 45 distribution, respectively.

46 In a discrete case, the mass (or particle) flowrate of tracer per unit length at any location
 47 along the exit boundary of the system is given as:

$$48 \quad \frac{dm(x_B, t)}{dt} = u_{\perp}(x_B, t) \cdot c(x_B, t) \cdot H(x_B, t). \quad (3)$$

49 where m is the mass per unit length ($kg\ m^{-1}$), u_{\perp} is the component of velocity perpen-
 50 dicular to the plane of interest ($m\ s^{-1}$), c is the concentration of tracer ($kg\ m^{-3}$), x_B is
 51 the system boundary coordinate, and H is the water depth (m). Defining the volumetric
 52 flowrate of water per unit length as $q_{\perp} = u_{\perp}H$ and integrating spatially over the domain
 53 boundary gives the mass flowrate of tracer exiting the system at time t :

$$54 \quad \frac{dN(t)}{dt} = \int_{x_{b,0}}^{x_{b,n}} \frac{dm(x_B, t)}{dt} dx_B = \int_{x_{b,0}}^{x_{b,n}} q_{\perp}(x_B, t) \cdot c(x_B, t) dx_B \quad (4)$$

55 The cumulative mass of tracer that exits the system at $t = \infty$ is thus:

$$56 \quad N_{\infty} = \int_0^{\infty} \int_{x_{b,0}}^{x_{b,n}} q_{\perp}(x_B, \tau) \cdot c(x_B, \tau) dx_B d\tau \quad (5)$$

57 where N_{∞} is also equal to the initial mass of tracer released for a pulse input with no return
 58 flow. Substituting Eqs. 4 & 5 into Eq. 1 yields the differential travel time distribution:

$$59 \quad E(t) = \frac{\int_{x_{b,0}}^{x_{b,n}} q_{\perp}(x_B, t) \cdot c(x_B, t) dx_B}{\int_0^{\infty} \int_{x_{b,0}}^{x_{b,n}} q_{\perp}(x_B, \tau) \cdot c(x_B, \tau) dx_B d\tau} \quad (6)$$

60 Eq. 6 is solved discretely at the domain boundary at each time step for model runs
 61 without return flows.

62 The mass flux of tracer at any transect within the domain can also be calculated fol-
 63 lowing a formulation similar to that of Eq. 6. Following the derivation of Eq. 6, the

64 fractional mass flux $e_i(t)$ at any transect x_i is given as:

$$65 \quad e_i(t) = \frac{\int_{x_{i,0}}^{x_{i,n}} q_{\perp}(x_i, t) \cdot c(x_i, t) dx_i}{\int_{0}^{\infty} \int_{x_{b,0}}^{x_{b,n}} q_{\perp}(x_B, \tau) \cdot c(x_B, \tau) dx_B d\tau} \quad (7)$$

66 Note that the denominator of Eq. 7 is the same as that of Eq. 6, which renders $e_i(t)$
 67 a local breakthrough curve normalized by the total amount of tracer exiting the system
 68 domain.

69 **Text S3 - Hydrodynamic model assessment**

70 The Frehd model was assessed by comparing to calculated discharges from ADCP tran-
 71 sects collected in the major distributary channels at WLD. The velocity transects com-
 72 prised a range of tidal conditions from comparison. A summary of the results from each
 73 field trip is contained in Table S1.

74 Discharge was measured by traversing transects in the major WLD distributary channels
 75 (Figure S1 - Locations Apex, A, B, C, D, E, CL, and CR) with a 600 kHz Teledyne
 76 RD Instruments Workhorse Rio Grande Acoustic Doppler Current Profiler (ADCP) in
 77 conjunction with differential GPS. The ADCP was mounted to the *R/V Lake Itasca* and
 78 sat 0.4 m below the water surface. The bin size was set to 0.50 m, the blanking distance
 79 was 0.44 m, and the boat speed was maintained at less than 1.0 m s⁻¹. The velocity
 80 transects were measured on 15 February 2013 from 08:00 to 12:00 and from 12:00 to 16:00
 81 CDT in an attempt to capture the falling and rising tides, respectively. The measurements
 82 coincided with a steady seasonal hydrograph at the USGS gage at Calumet, LA on the
 83 WLO. Flows had been near 1000 m³ s⁻¹ for much of the winter before rising and topping
 84 4200 m³ s⁻¹ by 1 February 2013. On 15 February 2013, the station recorded a maximum
 85 flowrate of 4530 m³ s⁻¹ and a minimum of 3850 m³ s⁻¹. For each cross section, two

86 consecutive passes (right bank to left bank, then left bank to right bank) at each transect
87 location were conducted during the predicted falling and rising tides. The field trip was
88 conducted during a neap tide with relatively high river discharge.

89 In June 2014, discharge was again measured in the major distributary channels at
90 WLD (Figure 1a - Locations A', B, C, D, E, and F). Coinciding with predictions for the
91 spring and neap tides, velocity transects were sailed on 15 June 2014 and 20 June 2014,
92 respectively. The ADCP measurement set up coincides with the methods of *Hiatt and*
93 *Passalacqua* [2015]. We measured velocity profiles along the transects with the 2 MHz
94 RDI StreamPro with the long-range upgrade measuring in water mode 12sp. Due to depth
95 limitations associated with the ADCP, transects Apex and A could not be measured since
96 they were greater than seven meters in depth. The ADCP was floated alongside the bow of
97 the *R/V Bluerunner* and the transect was traversed four times at an average boat speed of
98 about 1.0 m s^{-1} . The data output rate was maintained at 1 Hz and each collected velocity
99 ping was averaged from eight subpings. The ADCP transducer was 0.15 – 0.20 m below
100 the water surface, depending on channel depth and surface roughness conditions. The
101 blanking distance was 0.27 m. Depth profiles were linearly extrapolated to the channel
102 banks at a distance estimated from satellite imagery. ADCP transects were collected
103 during both rising and falling tides. The flow entering the WLD at transect A was 2880
104 $\text{m}^3 \text{ s}^{-1}$ during falling tide on 20 June 2014 and the average from the Calumet gage was
105 about $3450 \text{ m}^3 \text{ s}^{-1}$ [USGS, 2016]. To calculate discharge for both field trips, the measured
106 velocities were projected onto the average flow direction for each transect. Teledyne RDI's
107 WinRiver II software was used to process the GPS and ADCP data and output water

108 discharge within each cross section. The average measured discharge through the apex of
109 WLD was $3300 \text{ m}^3 \text{ s}^{-1}$.

110 The discharges measured in the field are normalized for comparison with the modeled
111 discharges. The normalized transect discharge (\hat{Q}) is obtained by dividing the transect
112 discharge Q by the discharge entering WLD through the Apex transect. Since, the Apex
113 discharge was not measured during the June 2014 field trip, the sum of discharges passing
114 through B, C, D, E, and F was used for normalization. This assumption is reasonable
115 considering the good agreement obtained among the measurements at A', B, C, D, and
116 E on 20 June (ratio = 1.03). The modeled flow partitioning is compared to the field
117 discharge results in Fig. S2 for the river-only and tidal cases during spring and neap tide.

118 In general, \hat{Q} does not significantly vary for each transect across the model results and
119 field observations. The average variability among the measurements at each transect is
120 5% and the maximum variability occurs at transects A and D (8%). Transect D consis-
121 tently receives the largest allocation of flow among the major bifurcates downstream of
122 the Apex and transect A. Transect C receives about 25% of the total flow through the
123 system followed by B, E, and F. A large flow asymmetry exists at the CR-CL bifurcation,
124 with CL receiving the majority of flow from transect C. The cross-sectional area is a good
125 predictor of discharge (Table 1), which agrees well with the control of depth on bifurcation
126 flow asymmetry.

127

128 **Text S4 - Particle tracer breakthrough curves compared to advection diffusion**
129 **tracer**

130 As discussed in the main text, we use a particle tracking code to determine ETD over
131 many model runs and at each grid cell within the domain of interest. The particle tracking
132 code reduces the computational effort that would be required to do so with the diffusive
133 tracer. We compare the mass flux distributions from both the particle tracking and
134 advection-diffusion tracers for the WLD model runs. Both the particles and the passive
135 tracer are released at the delta apex. The particle tracer mass flux curves compare well
136 to those generated with the diffusive tracer (Fig. S3). A Wilcoxon rank-sum test was
137 performed to test the statistical similarity of the breakthrough curves generated by the
138 diffusive tracer and the particle tracking. The null hypothesis of equivalent distributions
139 was not rejected at the 5% significance level for the river-only ($p = 0.28$), the spring tide
140 ($p = 0.89$), and the neap tide ($p = 0.65$) model runs. Therefore, the particle tracers are a
141 sufficient representation of the transport within the system.

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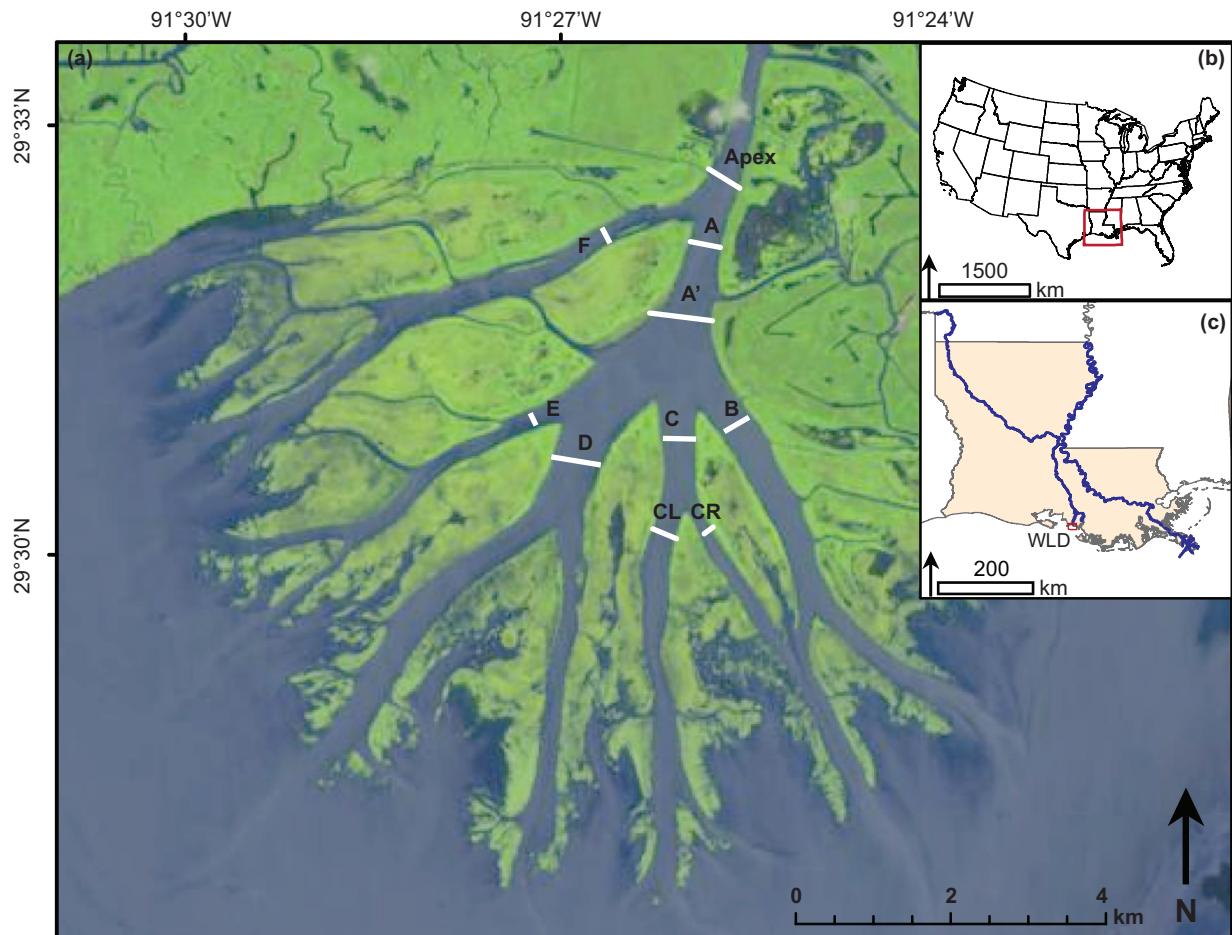
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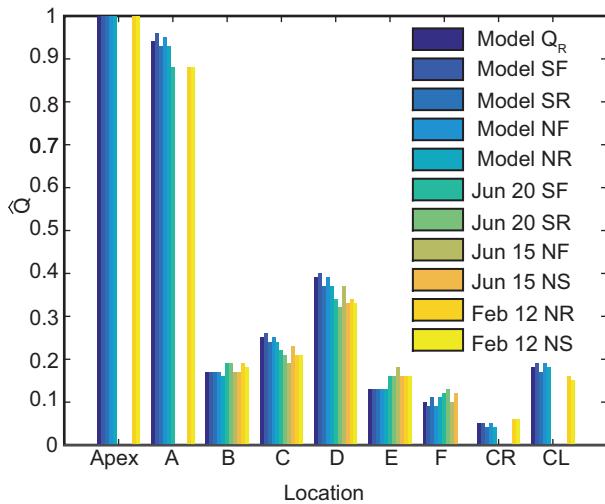
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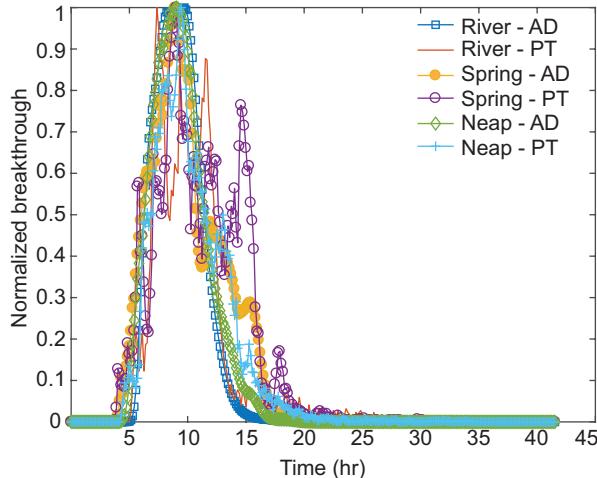


Supplementary Figure 1. Map of Wax Lake delta (WLD). Locations of acoustic Doppler current profiler transects. (b) The location of Louisiana on a United States map. (c) Map of major river systems in Louisiana and the location of WLD.



Supplementary Figure 2. Assessment of the hydrodynamic model on the WLD domain.

The fractional discharge (\hat{Q}) is compared among the various model runs and field data sets. The model is not calibrated to the field results, but provides good qualitative agreement with the actual flow partitioning at WLD.



Supplementary Figure 3. Comparisons among the breakthrough curves for the diffusive tracer (denoted AD in figure) and the particle tracers (PT). In each scenario, the distributions for the AD and the PT are statistically similar according to a Wilcoxon rank-sum test at the 5% significance level (p-values: 0.28, 0.89, and 0.65 for river, spring, and neap, respectively).

Supplementary Table 1. Average discharge, area, and width measured by the ADCP on 15 February 2013 and 15 and 20 June 2014. Transects averaged over fewer than four repeat measurements are italicized for the June 2014 field trip.

Time and Date	Location	Tide	Q ($\text{m}^3 \text{s}^{-1}$)	Area (m^2)	Width (m)
08:02 15 Feb	Apex	Fall	3734 (26)	3951 (49)	437 (61)
08:23 15 Feb	A	Fall	3278 (14)	3696 (11)	396 (1)
08:47 15 Feb	E	Fall	588 (4)	935 (5)	289 (3)
09:08 15 Feb	D	Fall	1279 (7)	2209 (11)	682 (4)
09:46 15 Feb	C	Fall	796 (12)	1244 (7)	428 (2)
10:13 15 Feb	B	Fall	681 (5)	1189 (20)	493 (7)
10:39 15 Feb	CR	Fall	221 (25)	397 (30)	160 (15)
10:54 15 Feb	CL	Fall	572 (1)	940 (23)	309 (6)
12:31 15 Feb	CL	Rise	582 (3)	950 (5)	309 (4)
12:49 15 Feb	CR	Rise	228 (14)	420 (31)	176 (17)
13:38 15 Feb	B	Rise	683 (18)	1248 (1)	523 (4)
13:59 15 Feb	C	Rise	775 (5)	1275 (14)	439 (3)
14:26 15 Feb	D	Rise	1250 (5)	2254 (2)	682 (4)
14:53 15 Feb	E	Rise	580 (3)	926 (1)	285 (3)
15:19 15 Feb	A	Rise	3225 (33)	3755 (18)	411 (19)
15:41 15 Feb	Apex	Rise	3671 (38)	3851 (12)	378 (9)
08:45 15 Jun	B	Fall	430 (5)	1266 (91)	490 (16)
09:18 15 Jun	C	Fall	466 (24)	1309 (128)	455 (42)
10:22 15 Jun	F	Fall	247 (26)	868 (26)	301 (14)
10:59 15 Jun	E	Fall	447 (10)	913 (18)	310 (8)
11:38 15 Jun	D	Fall	914 (5)	2230 (147)	668 (36)
13:16 15 Jun	B	Lower-high	468 (8)	1153 (58)	466 (18)
14:02 15 Jun	C	Lower-high	625 (18)	1217 (28)	423 (8)
14:58 15 Jun	D	Lower-high	880 (74)	2095 (32)	653 (17)
16:06 15 Jun	E	Lower-high	418 (18)	888 (33)	309 (8)
16:53 15 Jun	F	Lower-high	317 (22)	804 (14)	293 (3)
08:52 20 Jun	D	Rise	971 (10)	2161 (27)	640 (6)
09:50 20 Jun	E	Rise	448 (7)	870 (29)	314 (28)
10:29 20 Jun	C	Rise	610 (3)	1274 (18)	465 (9)
11:13 20 Jun	B	High	537 (6)	1198 (22)	501 (7)
12:43 20 Jun	F	Fall	364 (4)	802 (17)	306 (3)
13:25 20 Jun	D	Fall	1118 (14)	2208 (33)	699 (20)
14:11 20 Jun	E	Fall	530 (9)	867 (12)	324 (7)
14:40 20 Jun	A'	Fall	2899 (10)	4359 (214)	928 (8)
15:32 20 Jun	C	Fall	716 (10)	1195 (15)	449 (9)
16:32 20 Jun	B	Fall	619 (9)	1122 (20)	502 (6)
17:11 20 Jun	F	Fall	381 (9)	728 (20)	296 (6)