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## A Method for Estimating Materials Fluxes from Coastal Wetlands into the Great Lakes, with an Example from Lake Erie

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#### Abstract

A method is presented in detail for the quantitative estimation of the flux of water and dissolved or suspended materials through coastal wetlands of the Great Lakes where the juncture of the wetland and lake is constricted by a narrow opening. Application of the method requires frequent (e.g., hourly or daily) data on atmospheric deposition, precipitation, evaporation, upstream discharges from tributaries, wetland water level changes, and water chemistry data at upstream and downstream locations, as well as a detailed knowledge of wetland depth-area and depth-volume relationships. Data collected during October 1989 are used to demonstrate the method.

## **Table of Contents**

Page

	i
Abstract	iti
List of Tables	iv
List of Figures	v
List of Appendices	¥ 1
INTRODUCTION	1
Background	1
METHODS OF DATA COLLECTION FOR DISCHARGE	
AND WATER CHEMISTRY	4
METHOD FOR CALCULATING MATERIALS FLUXES	6
Determination of Stage Intervals	6
Assignment of Chemical Samples to Intervals	6
Calculation of Discharge for Each Stage Interval	9
EXAMPLE OF MATERIALS FLUX CALCULATIONS	15
	18
SUMMARY AND CONCLUSIONS	18
ACKNOWLEDGMENTS	
LITERATURE CITED	19

## List of Tables

1	Stage intervals, volume changes, precipitation, evaporation, and total discharge at the Old Woman Creek wetland during October 1989	10
2	Specific conductance and concentrations of total phosphorus, soluble reactive phosphorus and total suspended solids in water samples collected at the mouth of Old Woman Creek wetland in October 1989	12
3	Measured amounts and calculated volumes of precipitation and evaporation during October 1989 at the Old Woman Creek wetland	13
4	Sample times, discharge intervals, total discharge per interval and fluxes of materials in Old Woman Creek at Berlin Road in October 1989	14
5	Specific conductance and concentrations of total phosphorus, soluble reactive phosphorus and total suspended solids in water samples collected from Old Woman Creek at Berlin Road in October 1989	16
6	Water and materials fluxes through the Old Woman Creek wetland calculated for October 1989	17

## List of Figures

1	Conceptual model showing the routes of movement of water and materials	Page
	through a Great Lakes coastal wetland	2
2	Water level changes in the Old Woman Creek wetland and adjacent Lake Erie during October and early November 1989	3
3	Lower end of the 69 km <sup>2</sup> Old Woman Creek watershed, Erie County, Ohio	5
4	Protocol for assignment of samples to stage intervals	
		8
Lí	st of Appendices	
A	Rating table published by U.S.G.S. for Old Woman Creek at Berlin Road, Erie County, Ohio	23
B	Mean daily discharges and hourly stages published by the U.S.G.S. for October 1989 at Old Woman Creek at Berlin Road	23
С	Hourly stages reported by U.S.G.S. for October 1989 at the Old Woman Creek wetland at U.S. Highway 6	24
D	Expanded hypsographic and depth-volume table for Old Woman Creek Wetland	
		26.27

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## A Method for Estimating Materials Fluxes from Coastal Wetlands into the Great Lakes, with an Example from Lake Erie

#### INTRODUCTION

The need to understand the water quality functions of wetlands has long been recognized (Zedler and Kentula 1985, International Joint Commission 1986, Mitsch and Gosselink 1986), and within the past two decades, models have been generated to advance this understanding for various types of wetlands (Mitsch et al. 1988). Along the shores of the Laurentian Great Lakes, coastal wetlands are believed to play an important role in the amelioration of pollution. The riverine wetlands at the flooded mouths of Great Lakes tributaries (so-called "Great Lakes estuaries"; Brant and Herdendorf 1972, Herdendorf 1990, Dyer 1990, Odum 1990) may be especially important in reducing the loadings of pollutants received from agricultural watersheds. Mass balance studies of pollutants entering and leaving tributary coastal wetlands of the Great Lakes have not been conducted. Sager et al. (1985) approached this type of study when they related carbon, nitrogen and phosphorus concentrations at the mouth of a Green Bay, Lake Michigan, coastal marsh to water levels measured during ebb and flood phases of seiches.

The intent of this paper is to describe a method for the quantitative estimation of the flux of water and dissolved or suspended materials through Great Lakes coastal wetlands, especially those surrounding tributary mouths. The environmental parameters that must be measured simultaneously, the order of the steps that must be followed to calculate the loads, the equations that are used in the calculations, and some loads computed using the method are presented in detail.

#### Background

Wetlands along the shores and extending up the tributaries of the Laurentian Great Lakes are thought to function as sinks for a variety of materials, including suspended sediments and sediment-bound pollutants, such as pesticides, toxic metals and organic chemicals (Klarer and Millie 1989, MacCrimmon 1980). Buchanan (1982) estimated that the Old Woman Creek wetland on the shore of Lake Erie has been accumulating sediment from its agricultural watershed at the rate of approximately

one centimeter per year. It is thought that such wetlands also transform some pollutants from more biologically active forms to forms that have less impact on the receiving lake. For example, orthophosphate, which derives from natural sources as well as from treated sewage effluent and agricultural runoff, can cause undesirable algal growth in the Great Lakes. The uptake of orthophosphate by algae in the coastal wetlands, which intercept surface runoff from Great Lakes tributaries, transforms the orthophosphate into organic phosphorus within the algae. The algae in the wetland are transported into the lake during high rates of tributary flow (Klarer 1989), but the phosphorus is no longer available directly to stimulate algal growth in the lake. Depending on their individual chemical and physical properties, materials moving into and through coastal wetlands follow most or all of the hydrologic pathways shown in Figure 1.

Substantiation of the ameliorating effects of coastal wetlands on the pollutant loads carried down rivers from Great Lakes watersheds has been lacking, except by the application of indirect evidence from differences in pollutant concentrations between the upstream and downstream ends of wetlands (Klarer and Millie 1989, MacCrimmon 1980). In order to use chemical concentrations (mg L-1) as surrogates to determine mass transport (usually reported in kilograms or metric tons of material), such estimates have necessarily assumed that the instantaneous discharge (flow rate,  $m^3 s^{-1}$ ) of water into a wetland is equal to that discharging from the wetland into the lake, which probably is rarely the case because the water levels of the wetland and lake do not maintain an equilibrium. This is illustrated by Figure 2, which compares the constantly changing levels of the Old Woman Creek coastal wetland in northern Ohio and contiguous Lake Erie.

The only valid estimation of the differences between input and output of materials in a wetland is derived by measuring both concentration and discharge at each end of the system. With that information, the estimated load of a substance during a selected interval of time is calculated as:

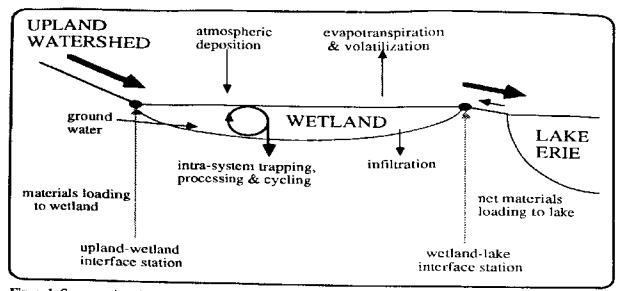


Figure 1 Conceptual model showing the routes of movement of water and materials through a Great Lakes coastal wetland. The thickness of the arrows is an approximate indication of the expected relative importance of each pathway. The dashed arrows indicate that flow between the wetland and lake is sometimes interrupted by a barrier beach. The present study examines the loading of materials into a coastal wetland and Lake Erie by means of discharge and concentration measurements acquired at an upland site and a site near the juncture of the wetland and the lake.

Load = Concentration x Discharge x Time =  $kg/m^3 x m^3/sec. x sec.$  (1)

In estimating loading in free-flowing streams, the time interval represented by each chemical sample in a series of samples is typically determined by calculating the midpoint in time between it and the previous sample and the midpoint between it and the following sample. Thus, for a series of samples that were collected at equal intervals, the time at which the sample was collected lies at the midpoint of the interval represented by that sample. For a series where the samples were collected at unequal intervals, the time of sample collection will not coincide with the midpoint. (Refer to Baker (1988) for a more detailed discussion.)

At discharge measurement sites on upland streams, such as most of the gaging stations operated by the U.S. Geological Survey (U.S.G.S.) (see, for example, Shindel et al. 1990), discharge is a function of the river stage (water depth); the higher the stage, the greater the amount of discharge. At such sites, the U.S.G.S. develops a "rating curve" that plots the discharge at each stream stage. Rating tables are produced that list the discharge (in ft<sup>3</sup>/s) for each stage in increments of 0.01 foot (0.305 cm). A rating table (Appendix A) has been produced for discharge in the most downstream free-flowing reach of Old Woman Creek, a second-order tributary to Lake Erie in northern Ohio. On the basis of the rating curve and a record of the stream stage gathered at 15-minute intervals, the U.S.G.S. annually publishes the computed mean daily discharges (in ft<sup>3</sup>/s) for each year. The mean daily discharges published for Old Woman Creek for October 1989 are shown in Appendix B.

The downstream reaches of most, if not all, Great Lakes tributaries possess a much more complex hydrology than the upstream, free-flowing reaches. The complexity arises from the interaction of stream and lake hydrologic processes in the stream reach which lies at or below the lake stage. In these regions, the discharge is no longer proportional to stream stage; therefore, the discharge cannot be calculated in the same manner as for free-flowing streams. The hydrology of these "flooded" stream reaches (and their associated wetlands) is controlled by six primary interacting processes, one or more or more of which dominates at any given time. They include (1) upland stream discharge; (2) seiches and storm surges, which irregularly raise and lower the lake stage and thus fill and drain the wetland and stream channel, much as the regular ocean tides do in estuaries; (3) periodic and somewhat seasonal "fill-and-drain" cycles in small tributaries, whereby a barrier beach develops from lake surf action and dams the tributary mouth, followed by a rising wetland water level behind the dam and eventual breaching of the dam, which allows the water level to drop suddenly (see Figure 2A); (4) direct precipitation on the wetland water surface; (5) evapotranspiration; and (6) seasonal, as well as long-term, oscillations in lake level, which affect the extent of downstream tributary flooding. Seiches and storm surges frequently reverse the normal flow of the tributary into the lake, instead forcing

an intrusion of lake water up the channel and into the surrounding wetland. The rapid and ephemeral events involving the fill-and-drain cycles and storm surges, seldom observed by most visitors to Great Lakes tributary mouths, have been documented on videotape (Krieger and Wright 1990).

Methods are available for estimating discharge in backwater areas subject to reversing flows with the use of various types of multidirectional velocity meters, including mechanical, electromagnetic and acoustic velocity meters. The characteristics of each

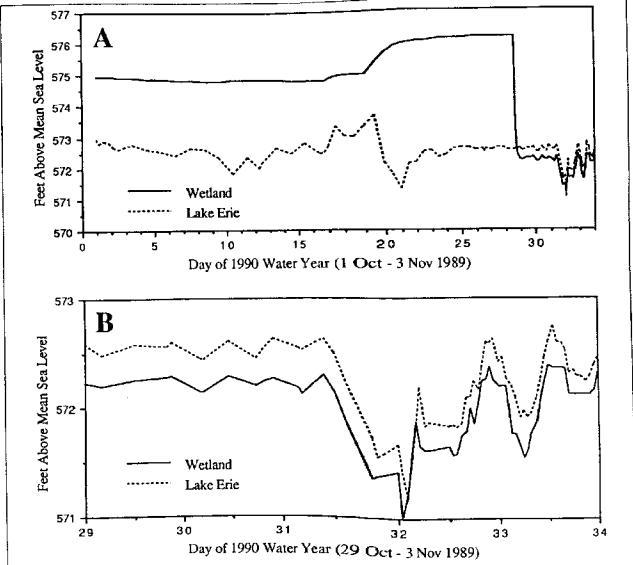


Figure 2 Water level changes in the Old Woman Creek wetland (U.S.G.S. Station 04199165) and adjacent Lake Erie (Ruggles Beach, U.S.G.S. Station 04199175) during October and early November 1989. Fig. 2A shows the higher water level in the wetland, which broke through the barrier beach beginning at about 1100 hours on 28 October 1989 and quickly fell to the level of Lake Erie. Fig. 2B shows detail of Fig. 2A for the period 29 October to 3 November 1989. The graph shows from five to seven data points each day on 29, 30 and 31 October and hourly data points on 1 and 2 November (days 32 and 33) in order to demonstrate the greater number of rising and falling stages which are detected with more detailed data sets. The lake stage appears to be calibrated at a slightly higher elevation.

type of meter and the complexities of their use are described by Kennedy (1984).

Difficulties have been encountered in the use of single meters for measuring discharge at the drowned mouths of tributaries and in the outlets of coastal wetlands. Adequate measurement in some circumstances may require that multiple meters be employed at several points horizontally and vertically within a cross-section of the outlet. During periods of low flow, the velocity is highly variable over time and space, and flow may simultaneously be in opposite directions at different points in the cross-section. Such flow reversals are only detectable by a battery of velocity meters strategically placed in the cross-section. These requirements make the measurement of stream discharge at lake level expensive and computationally complex.

Because of the difficulties and expense in estimating discharge with velocity meters in areas under the influence of lake stage, and when meters are not available, it is desirable to employ an alternative approach that is independent of velocity measurements. One approach that has been attempted makes use of the differences, at any given point in time, in the stages at three geographic points-one located in the wetland a few hundred meters upstream of the discharge point, another at the discharge point-and the third a few hundred meters offshore in the lake. The slope of the differences is used to calculate the expected flow rate of water due to gravitation past the discharge point, taking into account frictional forces and other factors.

The present report describes an alternative method for estimating discharge and materials fluxes between a coastal wetland and a lake. The method relies on a detailed knowledge of the bathymetry of the wetland and changes in area and volume of the wetland as described by hypsographic (depth-area) and depth-volume curves. The method was developed specifically to determine the loads of sediment, nutrients and pesticides moving through the Old Woman Creek wetland into Lake Erie (Figure 3). The downstream loads estimated by this method were subtracted from the upstream loads estimated by the standard approach described above for free-flowing streams. The difference was the amount estimated to be retained within the

wetland or changed to other chemical states before leaving the wetland.

## Methods of Data Collection for Discharge and Water Chemistry

The data were collected at two instrumented stations on Old Woman Creek, one located upstream above the influence of Lake Erie and the other downstream at a constriction near the mouth, where fluctuating lake levels and the shifting barrier beach continuously modify flows and water exchange between the wetland and lake (Figure 3). At the upstream site (Berlin Road, U.S.G.S. station ()4199155), where the creek drains 83 percent of the total watershed, stream stage was recorded and water samples were collected. Electrical service to a winterized building permitted the year-round operation of automated, refrigerated water samplers (Isco Model 2700) for the collection of samples for sediment, nutrient and pesticide analysis. A submersible pump was permanently anchored in the creek and pumped water into a plastic washtub continuously for about one hour before and after each water sample was scheduled to be collected by the autosampler. The autosampler was timed to collect a water sample from the washtub every eight hours (0400, 1200, 2000 hours). From April through August, a second autosampler collected pesticide samples from the same washtub. Details of the collection, analytical and quality control procedures are described by Baker (1988).

An identical sampling protocol was followed at the downstream site, where samples were collected from the middle of a wetland constriction about 30 meters upstream of the U.S. Highway 6 bridge (Figure 3). An intake pipe with a submersible pump at the end was suspended in the water column by a large float, which kept the pump above the bottom and about 0.5 meters below the surface.

It was not desirable nor feasible to analyze all of the samples, since three were collected each day. During low flow periods at both sites, only a single sample per day (usually at 1200 hours) was analyzed, and when no surface flow was present at a site, several days were often allowed to elapse between analyzed samples. When storm runoff was present upstream, as demonstrated by increased

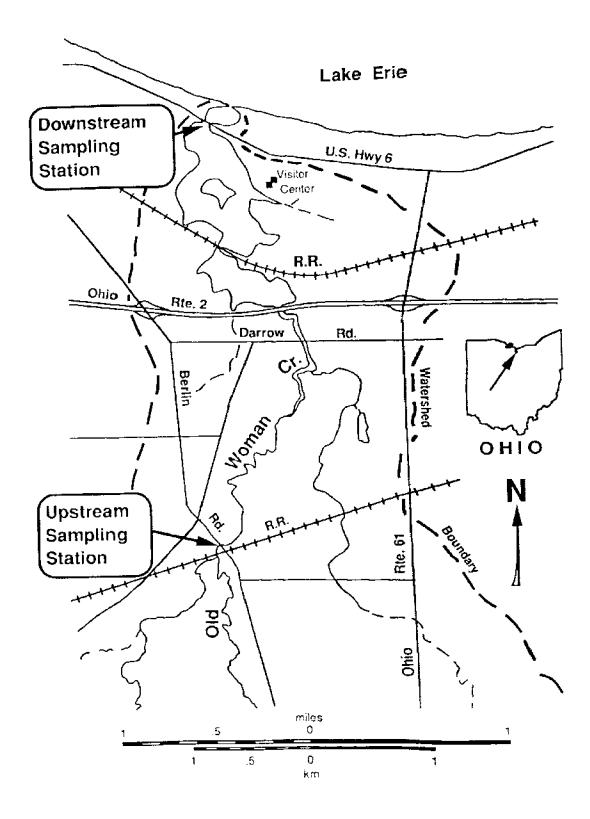


Figure 3 Lower end of the 68.9 km<sup>2</sup> Old Woman Creek watershed. Eric County, Ohio. The two arrows show the locations of the upstream and downstream transport stations.

turbidity in the samples, all three samples taken each day were analyzed to the point where turbidity returned to background levels; afterwards, analysis of one sample per day was resumed until the next storm event.

Conductivity (specific conductance), as an indirect indicator of the ionic content of water, is a sensitive marker of water masses from different sources. For instance, groundwater entering Old Woman Creek is rich in dissolved minerals, and during low flow, the conductivity of the creek water sometimes exceeds 100 mS m<sup>-1</sup> (1,000 µmhos cm<sup>-1</sup>) at 25°C. During storm runoff events, groundwater is diluted by rain water, occasionally yielding creek water conductivities below 40 mS m<sup>-1</sup>. The conductivity of Lake Erie water is almost always below 35 mS m<sup>-1</sup>, with a western basin mean of 28.2 mS m<sup>-1</sup> (Krieger 1989). By contrast, the conductivity of Old Woman Creek wetland water has varied from 17.6 to greater than 100 mS m<sup>-1</sup> (Krieger 1989; Krieger, unpublished Ohio Sea Grant data).

At the downstream site, the conductivity of every sample collected throughout the year was analyzed. Abrupt differences in conductivity between two adjacent samples indicated a change in water masses due to changing water levels or the influx of upland storm water. Thus, two or three samples per day were analyzed as indicated by conductivity changes and storm events.

### Method for Calculating Materials Fluxes

Determination of Stage Intervals The approach used at the free-flowing upstream site to calculate the time interval represented by each sample, in which the midpoints in time between the previous and following samples delimit the interval, cannot be applied to the downstream site at lake level. During the time between samples at the lake level site, the wetland stage may rise and fall several times, usually reflecting short-term intrusions of lake water into the wetland. Water sampled at that site derives from the lake or the wetland, depending on whether flow is directed into or out of the wetland at the time of sampling. Because the sampling site is situated about 100 meters upstream of the mouth, water flowing upstream past the sam-

pling point could be wetland water rather than lake water, which may not yet have traveled that far upstream. However, the identity of the water mass sampled can be determined because the wetland and lake water masses have different conductivities, as noted above. In calculating materials loading, a sample can represent only the water mass from which it was taken; the alternate water mass must be represented by a different sample. Thus, it is necessary to know all the time intervals when the stage was rising and falling (hereafter called "stage intervals") and to apply the correct water chemistry data to each stage interval.

Two questions must be answered before water samples can be assigned to stage intervals. First, because the sampling site is slightly inland from the mouth of the wetland (Figure 3), was the magnitude of the stage change sufficient for the water mass previously sampled at the end of the pump intake line to be replaced by a different water mass? For example, during a rising stage, was the wetland water mass displaced by a mass of incoming lake water at the intake point? Second, what resolution is desirable for recording stage changes? Over a period of several days, a greater number of rising and falling stage intervals will be observed in stage data recorded every 15 minutes than in stage data recorded hourly. Likewise, hourly data will yield more stage intervals than will daily data (Figure 2B). The largest stage changes will usually be apparent in the daily data. Because water samples for this project were collected every eight hours, it makes little sense to be concerned with changes occurring during intervals as short as 15 minutes. Many stage changes observed even in hourly data are small (<0.10 ft.) and of less than three hours duration (see sample stage data sheet, Appendix C). In the computations that follow, only stage oscillations of at least 0.10 foot (30.5 cm) as recorded in the hourly data were used. Even so, five or more stage intervals may be found within some 24-hour periods. The stage intervals of at least 0.10 foot for October 1989, as recorded in the hourly data shown in Appendix C, are characterized in Table 1 and appear in Figure 2.

Assignment of Chemical Samples to Intervals Once the stage intervals have been determined, it is necessary to assign a water sample to each interval 7

so that sediment and chemical fluxes can be calculated. Because the number of analyzed samples varies from less than one to three per day, it is often necessary to apply a particular sample to more stage intervals than the one during which it was collected. Table 2 lists the dates and times when samples were collected in October 1989.

A protocol for assigning samples to stage intervals has been developed to ensure consistency of the process. This protocol is presented in Figure 4 in a form similar to a dichotomous key. That is, at each step, or couplet, a decision about the stage interval or the sample, if one was collected during the interval, must be made. This protocol was followed in the flux calculations, which are presented later in this report. An explanation of each step is provided here.

Step 1 requires knowing whether a water sample was collected during the interval under consideration. If so, then it may be possible for that sample to be used for the flux calculations for that interval, as determined in steps 2, 3 and 4. If not, a sample taken during another interval must be applied to this interval in order to provide concentration data for computing fluxes.

In Step 2, the direction of the stage oscillation must be determined. If the water sample was collected while the stage was *falling*, the sample invariably will represent water that has passed through the wetland. Thus, that sample can be used to calculate materials flux. On the other hand, if the stage was *rising*, the sample may have been taken from either a lake water mass or a wetland water mass (because the sampling point is slightly upstream of the mouth).

Step 3 provides for determining whether the water sample can be used to calculate the flux of materials entering from the lake. Water may be flowing out into the lake, despite the fact that the wetland stage is rising in response to a rising lake stage. This occurs when the total water input into the wetland from precipitation and discharge from the upland tributaries exceeds the change in volume resulting from the rising stage, after accounting for evaporation. An example of this is seen in the last stage interval for the month of October 1989 in Table 1. In such instances, the water sample collected during the interval should be used to calcu-

late materials flux, and the flux should be added to the total loading to the lake.

If water was flowing into the wetland from the lake, as determined by the calculation in Step 3, then the identity of the water mass (lake or wetland) at the time of sampling must be determined before a decision can be made whether to use the water sample taken during that interval. This decision is made in Step 4 by observing the conductivity of the sample, since this property provides a "signature" for lake water. Values below about 35 mS m<sup>-1</sup> in the wetland result either from the intrusion of lake water or from the influx of low-conductivity storm water from upstream. If the sample conductivity is less than 35 mS m<sup>-1</sup> during a rising stage with water flowing in from Lake Erie, the sampled water mass is assumed to be lake water. If higher conductivity values are encountered under that set of conditions, it is assumed that a wetland water mass was sampled and that lake water was entering the wetland but had not yet progressed to the sample intake line. Therefore, the present sample represents wetland water, not lake water, and it cannot be used to calculate materials flux into the wetland from the lake. Step 5 then requires that the concentration values of the nearest sample to that interval (either before or after) with a conductivity less than 35 mS m<sup>-1</sup> during an inflow from the lake be used to characterize the fluxes during that interval.

Under circumstances when the upstream tributaries are discharging through the wetland at a high rate after a rainstorm, the conductivity of the wetland water sometimes drops to values below 35 mS  $m^{-1}$  as a result of the dilution of high-conductivity groundwater with rainwater. The water mass is unmistakably wetland or creek water under those circumstances because of the strong outflow of water into the lake.

In those instances when no water sample was collected during a stage interval (Step 1), it is necessary to use a surrogate water sample. In Step 6 it is determined whether the stage rose or fell during the interval. If the stage fell, the values of the last sample taken with a conductivity greater than  $35 \text{ mS m}^{-1}$  are used (Step 7a), since the wetland was discharging to the lake. If it was a rising stage, once again the direction of flow must be determined (Step 7b). If the discharge was into Lake

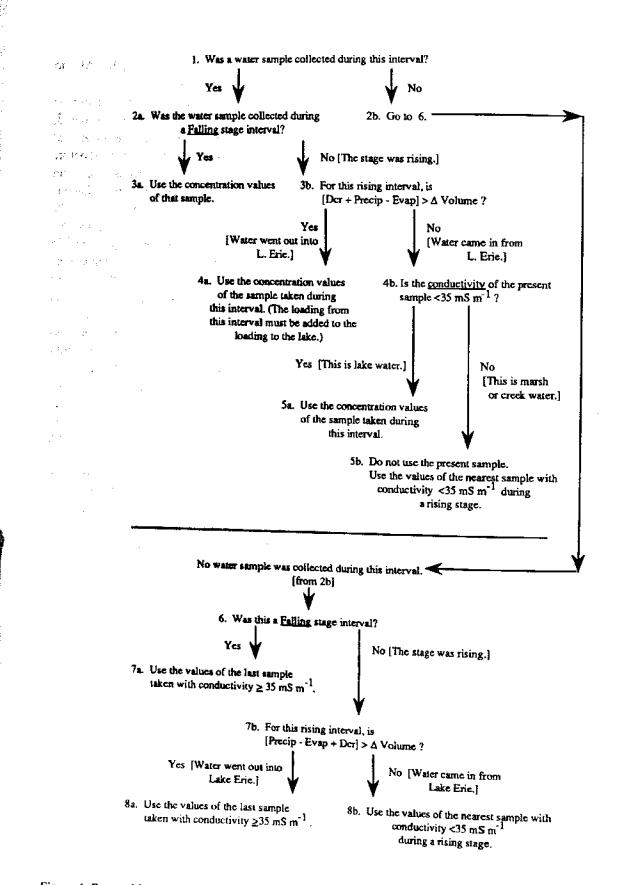


Figure 4 Protocol for assignment of samples to stage intervals.

Erie, the values of the last sample taken with a conductivity greater than or equal to 35 mS m<sup>-1</sup> are used (Step 8a). If the discharge was from Lake Erie, the values of the nearest sample with a conductivity less than 35 mS m<sup>-1</sup> during a rising stage are used (Step 8b). The results of this protocol for the month of October 1989 are shown in the far right column for each interval in Table 1.

Calculation of Discharge for Each Stage Interval In order to complete the protocol discussed above for rising stage intervals (steps 3b and 7b), and to compute fluxes for all intervals, data are needed on precipitation, evaporation, surface discharge into the wetland from its watershed, groundwater exchange, and the change in volume of the wetland. In essence, a water budget must be calculated for each stage interval, and it must take into account each of the pathways in the conceptual model shown in Figure 1.

Discharge for falling stages is calculated differently than discharge from rising stages, although the same parameters are used. The following equations assume negligible groundwater interaction in the model (Figure 1). An equation similar to Eq. (2) below was constructed independently by Mitsch et al. (1989), who also disregarded groundwater interchange in their computations.

During a *falling stage interval*, discharge through the mouth of the wetland is calculated by the equation:

 $D = \Delta V - Evap. + Precip. + D_{cr}$  (2) where:

- D = discharge from the wetland during the interval (in  $m^3$ )
- $\Delta V = |V_b V_c|; V_b = \text{volume of water in the}$ wetland at beginning of the interval and  $V_e = \text{volume at end of the interval}$
- Evap = evaporation during the interval (in m<sup>3</sup>) = total evap (in m) times mean area (in m<sup>2</sup>) of wetland during the interval; beginning and ending areas are determined from the hypsographic table (Appendix D)
- Precip = precipitation during the interval (in m<sup>3</sup>) = total precip (in m) times mean area (in m<sup>2</sup>)
  - of wetland during the interval

- $D_{cr}$  = discharge from the total watershed via the tributaries (in m<sup>3</sup> sec<sup>-1</sup>)
  - = D<sub>Berlin</sub> + D<sub>other</sub> = 1.187(D<sub>Berlin</sub>), where D<sub>Berlin</sub> = mean hourly discharge at the Berlin Road gaging station, based on the hourly stages reported by U.S.G.S. during the interval, and
    - Dother = discharge from the remainder of the watershed, extrapolated from DBerlin.

During a *rising stage interval*, discharge is calculated with the equation:

$$D (in m3) = - [\Delta V + Evap. - Precip. -Dcr]$$
(3)

Several points should be noted in regard to performing the calculations. First, it is of interest to know the total inflows into the wetland from Lake Erie, as well as the total outflows from the wetland into the lake. By adding the inflows and outflows for a period of time, the net water budget for the wetland is derived. Therefore, Eq. (3) results in a negative discharge value when water flows in from Lake Erie and a positive discharge value when water flows out into Lake Erie.

Second, change in volume of the wetland is determined from the depth-volume table by subtracting the volume represented by the stage reading at the beginning of the interval from the volume represented by the stage reading at the end of the interval. The depth-volume table, expanded from one developed by Herdendorf and Hume (1991), is presented in Appendix D.

Third, precipitation and evaporation data add a degree of complexity to the calculations, in that these data are measured on a daily basis, not synchronized with the timing of stage intervals. Although it is possible to extract detailed, continuous data from the chart recordings, the manual effort involved makes this option prohibitive. Thus, when several stage intervals fall within a single day, which is often the case, the daily evaporation and precipitation values must be subjectively apportioned among the intervals. Furthermore, evaporation is measured from the evaporation pan directly as *net* evaporation, rather than total evaporation. Total evaporation (in mm) is calculated as:

Inter- val	Date	Interval (seconds) Began - Ended	Stages Begin.		Rose/ Fell	Vol. Begin.	(10 <sup>3</sup> n End	ι <sup>3</sup> ) Δ	Mean Area (m <sup>2</sup> )		Precip. mm10 <sup>3</sup> m <sup>3</sup>	Ev mm	ap. m <sup>3</sup>	D <sub>cr</sub>	Total Discharge 10 <sup>3</sup> m <sup>3</sup>
1	10/01 - 10/28	1300		6.11	R	• • • • • •		мо	UTH CLOSE	ED					0*
2	10 <b>/29</b>	28/1300-29/0400 (54,000)	6.11	2.19	F	647.6	41.2	-606.4	429.4	0	0	1.27	0.55	4.08	+ 609.9
3	10/29	0400 - 2100 (61,200)	2.19	2.28	R	41.23	49.60	+ 8.37	231.3	0	0	2.5	0.58	4.41	- 4.53
4	10/30	2100-0500 (28,800)	2.28	2.14	F	49.60	38.27	- 11.33	224.2	0	0	0	0	2.08	+ 13.41
5	1 <b>0/3</b> 0	0500 - 1100 (21,600)	2.14	2.28	R	38.27	49.60	+ 11.33	224.2	0	0	2.24/3	0.17	1.49	- 10.01
6	10/30	1100 - 1600 (18,000)	2.28	2.13	F	49.60	37.91	- 11.69	223.0	0	0	2.24*2/3	0.34	1.24	+ 12.59
7	10/30	1600 - 2200 (21,600)	2.13	2.26	R	37.91	47.74	+ 9.83	219.9	0	0	0	0	1. <b>5</b> 6	- 8.27
8	10/31	2200 - 0500 (25,200)	2.26	2.12	F	47.74	37.55	- 10.19	218.6	0	O	0	0	1.82	+ 12.01
9	10/31	0500 - 1000 (18,000)	2.12	2.29	R	37.55	50.53	+12.98	223.3	0	0	0	0	1.30	- 11.68
10	10/31	1000-1900 (32,400)	2.29	1.35	F	50.53	1 <b>3.8</b> 3	- 36.70	144.6	4.57	7 0.66	1.55	0.22	2.56	+ 39.70
11	10/31	1900 - 2400 (18,000)	1.35	1.42	R	13.83	14.57	+ 0.74	42.49	0	0	0	0	1.55	+ 0.81

Table 1 Stage inservals, volume changes, precipitation, evaporation and total discharge at the Old Woman Creek wetland during October 1989.

\* Some wetland water seeped through the sand barrier beach into Lake Erie.

.

$$E_T = Precip. + Net Evap.$$
 (4)

Precipitation and pan evaporation data for October 1989 at the Old Woman Creek wetland are presented in Table 3. Of course, it is not necessary to separate precipitation from evaporation in the water budget model (Figure 1), but instead those two components can be combined into a single net evaporation term in the equations. When precipitation exceeds total evaporation, the net evaporation value will be negative.

To apply precipitation and evaporation to the discharge equations, the measurements (in meters rather than the original inches) must be converted to volumes  $(m^3)$  by multiplying them by the mean surface area  $(m^2)$  of the wetland during the interval (Table 3). The mean surface area is derived from the hypsographic table (Appendix D) by adding the areas of the beginning and ending stages and dividing by two.

Fourth, discharge from the entire watershed via tributaries to the wetland  $(D_{cr})$  includes the mainstem of Old Woman Creek and perennial and intermittent streams that enter the wetland directly around its perimeter. Old Woman Creek, at the gaging station where discharge is measured at Berlin Road (DBerlin), drains 83 percent of the total watershed. Another 15.5 percent of the watershed is drained by tributaries below the gaging station, and the surface of the wetland, when full, accounts for 1.5 percent of the watershed. Thus, all discharge values from the gaging station are multiplied by 1.187 (98.5%/83.0%) to estimate the total surface discharge into the wetland. An additional factor should be built into the equation in the future to account for periods of very low flow in the mainstem of Old Woman Creek, when there may be no surface flow into the wetland from any other tributaries (i.e., D<sub>cr</sub> = D<sub>Berlin</sub>). However, only a small error in the total loading probably results from the lack of inclusion of this factor because a great majority of the discharge, and therefore pollutant and nutrient inputs, occurs during high flow events, when the smaller tributaries, especially the one draining 7.5 percent of the watershed east of Berlin Road, are flowing.

The rating tables provided by the U.S.G.S. report discharge in cubic feet per second. To obtain a discharge for the equations, an average creek stage at Berlin Road is computed for the wetland stage interval by dividing the sum of all hourly creek stages by the number of hourly readings. The average creek stage is converted to cubic feet per second. The total cubic meters of water discharged is then:

$$m^3 = ft^3/sec. \ge 0.0283 m^3/ft^3 \ge sec.$$
 (5)

The discharges of Old Woman Creek at Berlin Road in October 1989 are listed in Table 4 for each sample interval at that site.

Fifth, no direct measurements of exchanges between groundwater and surface water were made during this study. The net groundwater exchange for all intervals is assumed to be zero. Matisoff and Eaker (1989, 1992) showed that diffusional fluxes of solutes between Old Woman Creek wetland sediments and the overlying water were insignificant in comparison to seepage fluxes, and that the seepage fluxes composed an insignificant proportion of the total solute flux values obtained using flux chambers. They found that groundwater input occurred around the perimeter of the wetland, varying in their studies from zero cubic feet per second during a prolonged summer drought to 2.4 cubic feet per second after the drought subsided. They also measured discharge through the barrier beach, which ranged from 0.076 cubic feet per second to 0.42 cubic feet per second for the period of June through December. Thus, it appears that the maximum groundwater input rates could have accounted for a significant proportion of total water exchange in the wetland during some periods of very low surface water input, but that groundwater input is probably insignificant when compared to the very large volumes of water transported through the wetland during high-flow periods (Table 4).

Table 2Conductivity and concentrations of total phosphorus, soluble reactive phosphorus, and total suspended solids in<br/>water samples collected near the mouth of Old Woman Creek wetland from 17 October (891017) through 3 November 1989<br/>(891103). Conductivity was read on all but one of the samples collected; a "-" indicates that the parameter was not measured<br/>in that sample. Letters to the left of sample dates are used to designate those samples discussed in the text.

	Date	Time	Total P mg L <sup>-1</sup>	Sofuble Reactive P mg L-1	Total Suspended Solids, mg L <sup>-1</sup>	Conductivity mS m <sup>-1</sup>
	891017		0.163	0.002	50	53.5
	891017 891017	2000	0.156	0.006	51	52.8 53.0
	891018	0400	-	-	•	52.9
	891018 891018	1200	0.157	0.007	53	52.9
	891019		-	-	-	53.1
	891019	1200	0.118	0.007	87	44,4 35,2
	891019		-	-	-	41.5
	891020		-	-	-	43.5
	891020 891020		0.113	0.015	34	45.8
	891021		-	-	-	47.2
	891021	1200	0.100	0.011	24	48.3 48.9
	891021	2000	-	-	-	48.6
	891022		-	•	-	48.2
	891022 891022		0.087	0.006	17	48.6
	891023		·	-	-	49.2
	891023		0.100	0.003	13	50.3 51.9
	891023		-	-	-	50,1
	891024		-	-	-	51.8
	891024 891024	1200 2000	0.078	0.001	10	51.2
	891025		-	-	-	52.4
	891025	1200	0.078	0.001	- 11	51.9 52.2
	891025		-		-	52.9
	891026		-	-	-	52.2
	891026 891026		0.076	0.001	12	52.1
	891027	0400	•	-	•	52.3
	891027		0.086	0.001	- ]4	52.4
	891027		0.274	0.007	190	58.4
	891028		0.237	0.013	7.4	60.2
A	891028 891028		0.242 0.257	0.012 0.006	44	60.0
в	891029		0.287	0.006	64	61.0
С	891029	1200	0.253	0.007	77 65	61.5 61.2
$\mathbf{D}$	891030		0.258	0.029	75	65.0
	891031		-	-	-	62.2
E	891031 891031	1200 2000	0.314	0.022	-	59.3
-	891101			N. F. M. Faire and	155	65.3
	891101	1200	-	-	-	68.0 69,1
F	891101	2000	0.160	0.019	50	61.0
G	891402		0.138	0.020	48	54.0
H I	891102 891102	1200 2000	0,096 0,135	0.023 0.018	26 64	38.2
J	891102		0.075	0.014	40	45.9
2	891103	F200	0.118	0.015	54	26,4 56,4
	891103	2000	0.075	0.014	38	32.8

·	Mean	Mean	PRECIPI	FATION	EVAPORATION					
Date	Stage (ft)	Area 10 <sup>3</sup> m <sup>2</sup>	Measured mm	Vol. 10 <sup>3</sup> m <sup>3</sup>	Net mm	Total mm	Vol. 10 <sup>3</sup> m <sup>3</sup>			
1	574.92	569.0	0	0	2.6	2.6	1.48			
2	574.91	568.6	0	0	3.86	3,86	2.19			
3	574.88	567.2	0	0	4.04	4.04	2.29			
4	574.86	566.3	0	0	3.23	3.23	1.83			
5	574.85	565.8	0	0	4,22	4.22	2.39			
6	574.84	565.8	0	0	2.49	2.49	1.41			
7	574.81	564.0	0	0	3.12	3.12	1.76			
8	574,80	563.5	0	0	1.61	1.61	0.91			
9	574.78	562.6	0	0	1.61	1.61	0.91			
10	574,80	563.5	13.2	7,44	-10.72	2.48	1.40			
11	574.82	564.4	Ø	0	3.35	3.35	1.89			
12	574.82	564.4	Õ	0	2.95	2.95	1.66			
12	574,81	564.0	Ő	0	3.75	3.75	2,12			
15 ]4	574.80	563.5	ŏ	0	3.75	3.75	2.11			
14	574.79	563.0	Ő	0	3.75	3.75	2.11			
16	574.79	563.0	25.1	14.13	-24.22	0.88	0.50			
16	574.93	569.5	10,2	5.81	-4.66	5.54	3.16			
18	574.97	571.3	2.03	1.16	0	2.03	1.16			
10	575.34	588.4	33.3	*	-23.57	*	*			
20	575.82	620.4	1.27	*	-0,91	*	¥			
21	575.95	629.9	0.25	*	-1.81	*	٠			
22	576.02	635.0	0	0	0	0	0			
22	576.07	638.7	Ō	0	1.70	1.70	1.09			
23 24	576.10	640.8	Ő	0	1.19	1.19	0.76			
24 25	576.11	641.6	0	0	1.80	1.80	1.15			
26	576.11	641.6	0	0 1.32	1.32	1,32	0.85			
20 27	576.11	641.6	0	0	1.45	145	0.93			
28	574.77	562.1	Q	0	1.78	1.78	1.00			
20 29	572.24	232.5	ò	0	2.49	2,49				
30	572.21	223.3	0	0	2.24	2.24				
31	571.90	143.8	4.57	<u>0.66</u>	<u>-3.02</u>	<u>1.55</u>	<u>0.23</u>			
			Totals 89.92	50.54	+10.61	79.31	43.55			

 Table 3 Measured amounts and calculated volumes of precipitation and evaporation during October 1989 at the Old Woman Creek wetland.

\*The data collection times varied between precipitation and evaporation on these dates, yielding incongruent data. Totals for the period 19-21 October 1989 were precipitation 34.82 mm, volume of precipitation 21.34, 10<sup>3</sup> m<sup>3</sup>, net evaporation -26.29 mm, total evaporation 8.53 mm, volume of evaporation 5.23, 10<sup>3</sup> m<sup>3</sup>. Mean area for the period was 612.9, 10<sup>3</sup> m<sup>2</sup>.

 Table 4 Sample times, discharge intervals, total discharge per interval and fluxes of materials in Old Woman Creek at

 Berlin Road in October 1989.

Date	Sample				Stage			Mean	Total		kg of mate	riak
	Time	Beg.	End	Hrs	Beg.	End	Mean	Disch. cfs	Discharge 10 <sup>3</sup> m	1	TP	TSS
890930		2400					· · · · · · · ·		- stonarge to m	JAI		
891001		2400	2400	24	2.51	0.5-		! 	-			
891002	1200	2400	2000	20		2.51	2.51	0.635	1.55	0.0140	0.043	10.9
891003	0400	2000	0800	12	2.51	2.53	2.52	0.672	1.37	0.0096	0.037	
891003	1200	0800	2400		2.53	2.50	2.51	0.635	0.78	0.0070	0.023	
891004	1200	2400	2400	16	2.50	2.49	2.49	0.562	0.92	0.0092	0.027	
891005	1200			24	2.49	2.47	2.48	0.525	1.28	0.0124	0.037	
891006	1200	2400 2400	2400	24	2.47	2.46	2,47	0.491	1.20	0.0084	0.031	9.6
891007	1200		2400	24	2.46	2.45	2.46	0.459	1.12	0.0090	0.034	15.7
891008		2400	2400	24	2.45	2.42	2.43	0.372	0.91	0.0055	0.034	9.1
891009	1200	2400	2400	24	2.42	2.41	2.42	0.347	0.85			
	1200	2400	2000	20	2.41	2.41	2.41	0.323	0.66	0.0026	0.027	7.7
891010	0400	2000	0800	12	2.41	2,41	2.41					
891010	1200	0800	2400	16	2.41	2.76		0.323	0.39	0.0016	0.012	3.9
891011	1200	2400	2400	24	2.76	2.10	2.65	1.333	2.17	0.0087	0.150	60.8
891012	1200	2400	2400	24	2.67		2.71	1.783	4.36	0.1134	0.458	34.9
891013	1200	2400	2400	24	2.61	2.61	2.64	1.268	3.10	0.0372	0.161	18.6
891014	1200	2400				2.59	2.60	1.034	2.53	0.0152	0.111	15.2
891015	1200	2400	2400	24	2.59	2.56	2.58	0.931	2.28	0.0160	0.096	9.1
891016	1200	2400	2400	24	2.56	2.53	2.54	0.751	1.84	0.0184	0.083	7.4
891017	0400		2000	20	2.53	2.65	2.53	0.710	1.45	0.0104	0.080	8.7
391017	1200	2000	0800	12	2.65	3.13	3.06	7.554	9,24	1.3213	3.142	304.9
		0800	1600	8	3.13	3.01	3.07	7.843	6.39	0.1981	2.307	581.5
391017	2000	1600	2400	8	3.01	2.91	2.96	5.169				1
891018	0400	2400	0800	8	2.91	2.87	2.89	3.936	4.21	0.1810	0.960	168.4
891018	1200	0800	2400	16	2.87	2.85	2.86	3.474	3.21	0.1380	0.523	70.6
391019	1200	2400	1600	16	2.85	4.54	3.55	28.23	5.66	0.0906	0.657	67.9
391019	2000	1600	2400	8	4,54	4.17	4.37	26.25 76.81	46.02	1.9328	15.509	6396.8
391020	0400	2400	0800						62.60	2.5040	14.148	3818.6
391020	1200	0800	2400	8 16	4.17	3.59	3.84	43.75	36.66	1.6130	6.929	1099.8
91021	1200	2400	2400	24	3.59	3.16	3.30	16.46	26.83	0.8854	3.783	697.6
391022					3.16	3,06	3.11	9.091	22.23	0.3335	1.867	155.6
91022 91023	1200	2400	2400	24	3.06	2.99	3.02	6.485	15.86	0.3172		
	1200	2400	2000	20	2.99	2.93	2.96	5.169	10.53		0.936	63.4
391024	0400	2000	0800	12	2.93	2.89	2.91	4.264	5.21	0.0632	0.432	42.1
891024	1200	0800	2400	16	2.89	2.86	2.88	3.777	6.16	0.0365	0.234	36.5
891025	1200	2400	2400	24	2.86	2.82	2.84	3.193	7.81	0.0246	0.222	18.5
91026	1200	2400	2400	24	2.82		1			0.0156	0.250	23.4
91026	1300	mouth c	Dened		2.92	2,79	2.81	2.805	6.86	0.0069	0.103	6.9
91027	1200	2400	2400	24	2.79	ריד ל	<b>7</b> 70					
91028	1200	2400	2400	24	2.77	2.77 2.76	2.78	2.458	6.01	0.0120	0.180	18.0
91029	1200	2400	1600	16	2.76		2.76	2.246	5.49	0.0165	0.165	11.0
01020 -						2.75	2.75	2.146	3.50	0.0070	0.102	3.5
91030	2000	1600	0800	16	2.75	2.74	2,74	2.050	3.34	0.0301	0.137	i
91031	2000	0800	0800	24	2,74	2.78	2.76	2.246	5.48	0.6521	1.052	16.7
91101	2000	0800			2.76		;			0.0021	1.052	43.8
					Total	Dischar	ne fo-	1	1			
i						er 1989	SCION		102 67 102 1			
hears 2.1	i			Ì	~~w/0	. 1707	1		323.61 x 10 <sup>3</sup> m <sup>3</sup>			
hrough	1000			i			Ì			i t	i	
)/26	1200			1				ł	:	9.9525	53.465	12 011 0
fter 👘											55.403	13,811.5
/26	1300 .			ļ						0.734		
tal										0.7246	1.739	99.5
89				1				!		10	1	
	:									10.6771	55.204	13 011 4

## E xample of Materials Flux Calculations

This method for calculating fluxes of materials through wetlands was developed in order to specifically characterize the functioning of the Old Woman Creek wetland as a sink, source or modifier of the major pollutants entering it. As an example, the fluxes of total phosphorus, soluble reactive phosphorus and total suspended solids in the samples collected from Old Woman Creek at Berlin Road in October 1989 are listed in Table 4, and the concentrations from which those fluxes were derived are shown in Table 5. The fluxes of water and materials through the wetland are shown in Table 6. Eleven stage intervals were recorded during the month. The first interval represented the period 1-28 October at 1300 hours, when the mouth of the wetland was closed and there was no interchange of surface water between the wetland and Lake Erie, except for some unmeasured seepage through the barrier beach into the lake and perhaps some overtopping of surf over the beach. Stage intervals 2 through 11 represented alternating periods of falling and rising water levels (also shown in Table 1), beginning with the opening of the mouth at 1300 hours on 28 October and ending at 2400 hours on 31 October.

The volume of precipitation and surface inflow from upstream are shown in Table 6 for interval 1. Upstream inflow is not shown for the remaining intervals because the individual upstream sample intervals did not correspond with the downstream stage intervals. The ultimate objective was to obtain flux estimates for the entire month, rather than for individual sample or stage intervals. Total upstream inflow is shown in the bottom part of Table 6 for the two periods before and after 28 October at 1300 hours.

Discharge at the wetland mouth is shown in Table 6 for each stage interval, negative values indicating inflow from Lake Erie. Water flowed from the wetland into the lake during the last rising stage interval in October 1989 (interval 11) because the volume of upstream discharge into the wetland exceeded the volume of evaporation plus the increase in volume of the wetland during the interval.

The samples used to calculate materials fluxes during each stage interval are shown in Table 2. Two samples were collected during interval 2, both representing the wetland water mass. Thus, the average concentrations of the two samples (A and B) were used. No samples were collected during the next three falling stage intervals, so it was necessary to use samples A and B again to calculate fluxes during those intervals. Sample C (Table 2) was never used because it was collected during a rising stage while lake water was entering the wetland, yet it was taken from the wetland water mass, as indicated by its high conductivity. Sample D, collected during falling interval 10, represented wetland water, and so was applied to fluxes for that interval. Sample E also represented wetland water but was collected during a rising stage (interval 11). However, because water was flowing out of the wetland into the lake during the interval, it was appropriate to use sample E for interval 11. During the first four rising stage intervals after 28 October (Table 1), water entered the wetland from Lake Erie (Table 6); however, no samples collected during those intervals represented lake water (as judged by their conductivities), so the next lake water sample encountered in the data (Table 2, sample J on 3 November 1989) was applied to each of those intervals.

The movement of soluble reactive phosphorus, total phosphorus and total suspended solids into and out of the wetland and the percentage loss of each material between the upstream and downstream ends are shown in Table 6. During the entire month of October 1989, 11.29 kilograms of soluble reactive phosphorus entered the Old Woman Creek wetland via atmospheric deposition or surface flow, while 4.58 kilograms (net) moved out of the wetland into Lake Erie, equivalent to a loss of 41 percent of the input amount. The lost SRP (Soluble Reactive Phosphorus) was either stored in that form somewhere in the wetland ecosystem or it was changed to another form of phosphorus, such as intracellular phosphorus in algae. The fate of the lost SRP should be the subject of additional investigation. Unlike the case for SRP, much (322 %) more total phosphorus left the wetland than entered it during October 1989. The difference, which far

Date	Time	Total P mg L <sup>-1</sup>	Soluble Reactive P mg L <sup>-1</sup>	Total Suspended Solids, mg L <sup>-1</sup>	Conductivity mS m <sup>-1</sup>
891001	1200	0.028			
891002	1200	0.027	0.009	7	88.9
891003	0400	0.029	0.007	5	88.1
	0,00	0.029	0.009	7	89.0
891003	1200	0.029	0.010		
891004	1200	0.029	0.008	17	88.5
891005	1200	0.026		10	88.7
		0.020	0.007	8	89.1
891006	1200	0.030	0.008	14	
891007	1200	0.034	0.006		89.3
891008	1200	0.032	0.003	10	90.1
891009	1000		4,000	9	90.5
B91010	1200	0.033	0.004	10	<b>~</b> ~ ~
<b>391010</b>	0400	0.031	0.004		91.0
37 IVIU	1200	0.069	0.004	10 28	91.3
91011	1000			20	82.1
91012	1200	0.105	0.026	Q	<u> </u>
91012	1200	0.052	0.012	8	97.1
	1200	0.044	0.006	6 6	90.5
91014	1000	_		O	90.3
91015	1200	0.042	0.007	4	<b>_</b> .
91015	1200	0.045	0.001	4 4	91.5
-1010	1200	0.055	0.000	4 6	91,9
91017	0400	<b>.</b> -		D	92.4
91017	0400	0.340	0.143	33	
91017	1200	0.361	0.031	33 91	88.9
	2000	0.228	0.043	40	76.0
91018	0400	0.1/2		70	79.1
91018	1200	0.163	0.043	22	<b>A</b> <i>c</i> -
91019	1200	0.116	0.016	12	83.0
	1.000	0.337	0.042	139	86.2
91019	2000	0.007		137	58.6
91020	0400	0.226	0.040	61	
91020	1200	0.189	0.044	30	54.7
		0.141	0.033	26	54.6
1021	1200	0.00+			55.7
1022	1200	0.084	0.015	7	<i></i>
1023	1200	0.059 0.041	0.020	4	65.1
1024	0400	0.041	0.006		70.4
1024	1200	0.045	0.007	7	75.2
1025	1200	0.038	0.004	3	74.2
	·· - •	V.U32	0.002	4 7 3 3	76.9
1026	1200	0.030	0.0	~	77.8
1027	1200	0.030	0.002	2	70 -
1028	1200	0.030	0.002	2 3	79.1
		0.00 <b>0</b>	0.003	2	80.5
1029	1200	0.029	0.000	—	81.6
030	2000	0.041	0.002	1	PA -
031	2000	0.192	0.009	5	82.5
		·····	0.119	8	81.9
101	2000	0.042	0.016		84.7
102	2000	0.034	0.016 0.011	5	<b>01</b> 1
03	2000	0.035	0.011	5	83.3 82.2
			0.015	5	877

**Table 5** Conductivity and concentrations of total phosphorus soluble reactive phosphorus and total suspended solids in water samples collected from Old Woman Creek at Berlin Road, October through early November 1989.

	Densia	Surface	Discharge	Sample	Soluble R	eactive 1	Phosphorus		FLUXI al Phosp		Total	Suspendeo	t Solids
Stage Interval	10 <sup>3</sup> m <sup>3</sup>	10 <sup>3</sup> m <sup>3</sup>	10 <sup>3</sup> m <sup>3</sup>	Used <sup>*</sup>	In	Out	% Loss	In	Out	% Loss	In	Out	% Loss
1	49.9	296.4	0†	.†	0.59 p <sup>@</sup> 9.95 s <sup>@</sup>	0	0%	1.9 p 53.5 s	0	0%	150 p 13,812 s	0	0%
2 3 4 5 6 7 8 9 10 11	0.7		+ 609.9 - 4.5 + 13.4 - 10.0 + 12.6 - 8.3 + 12.0 - 11.7 + 39.7 + 0.8	(A+B)/2 J (A+B)/2 J (A+B)/2 J (A+B)/2 J D E	0.02 p	+ 3.66 - 0.06 + 0.08 - 0.14 + 0.08 - 0.12 + 0.07 - 0.16 + 1.15 + 0.02		0.1 p	+ 165.9 - 0.3 + 3.7 - 0.8 + 3.4 - 0.6 + 3.3 - 0.9 + 10.2 + 0.3		6 p	+ 42,998 - 181 + 945 - 400 + 888 - 331 + 847 - 467 + 2,978 +126	
Before <sup>†</sup> 10/28 1300	49.9	296.4	0		10.54	0	0%	55.4	0	0%	13,962	0	0%
After 10/28 1300	0.7	27.3	Net 653.9		0.74	+4.58	619%	1.8	+184.2	10,233%	106	47,403	44,720%
Total fo Oct. '89		323.7	Out 688.4 <u>In - 34.5</u> Net +653.9		0.61 p <u>10.68 s</u> 11.29 total	+4.58	41%	2.0 p <u>55.2 s</u> 57.2 u		322%	156 р <u>13.912 s</u> 14,068 ко		337%

 Table 6
 Water and material fluxes through the Old Woman Creek wetland calculated for October 1989.

\*See Table 2 for sample characteristics. <sup>†</sup>Mouth was closed throughout the interval 1-28 October 1989 at 1300 hours, with some unmeasured see page through barrier beach.  $^{\textcircled{m}}p = precipitation$  and other atmospheric deposition; s = surface flow

exceeds the amount of the lost SRP, most likely represents phosphorus stored within plankton and adsorbed to particulate matter during the course of the summer, which was exported primarily during the opening of the mouth of the wetland (interval 2, Table 6).

## **Summary and Conclusions**

A method has been presented for the estimation of the fluxes of water and materials between lakes and their coastal wetlands where the two are separated by a constricted passage or "mouth." The method was developed specifically to determine annual and seasonal fluxes at the mouth of the Old Woman Creek wetland on the south shore of Lake Erie, but it should be equally applicable to large lakes throughout the world possessing similar types of coastal wetlands. A quantitative knowledge of the capacities of such wetlands to store, modify and release various materials, especially sediment, nutrients and toxic pollutants, is essential to our progress in understanding the functional values of these shallow aquatic ecosystems and the nature of their interactions with the receiving lakes.

To calculate a water budget and materials fluxes for a single month is a very time-consuming and tedious task. Before the method can be applied efficiently to estimate seasonal and annual mass balances, it will be necessary to computerize the database and the mathematical calculations. This should be the next step in the further development and refinement of the method.

#### Acknowledgments

The work presented here would not have been possible without a great amount of field assistance from Dr. David M. Klarer, Scott Hoffman, Gary Obermiller and Gene Wright of the Old Woman Creek National Estuarine Research Reserve and State Nature Preserve, who helped construct instrumentation houses, monitored the sampling equipment and collected precipitation and evaporation data. A detailed bathymetric map and hypsographic and depth-volume curves for the wetland were developed by Drs. Charles E. Herdendorf of The Ohio State University and Terry M. Hume of the National Institute of Water and Atmo-

spheric Research Ltd of New Zealand. Drs. Herdendorf and Hume, as well as Dr. Sharon Fitzgerald of the U.S. Geological Survey, Madison, Wisconsin, critically reviewed an earlier version of this report. The U.S.G.S. provided continuous stage data at the upstream and downstream sites and developed a discharge rating curve for the upstream site through a cooperative agreement with the Ohio Department of Natural Resources, Division of Natural Areas and Preserves. This work is a result of research sponsored in part by the Ohio Sea Grant College Program, project R/ES-4 under grant NA89AA-D-SG132 of the National SeaGrant College Program, National Oceanic and Atmospheric Administration, U.S. Department of Commerce, as well as with funding from the Ohio Department of Natural Resources, Division of Natural Areas and Preserves and additional grants to the Water Quality Laboratory of Heidelberg College.

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Gage				h's East	DOT SOCOL	h	(Expanded Precision)							
Height	.00	Disch .01	arge in C .02	ubic Feet .03	.04	.05	.06	.07	.08	.09				
(feet)	.00								.000	.000				
2.00	.000	.000	.000	.000	.000	.000	.000	.000	.001	.001				
2.00	.000	.001	.001	.001	.001	.001	.001	.001	.058	.094				
	.000	.002	.003	.005	.008	.013	.022	.036	.038	.281				
2.20		.161	.173	.186	,200	.214	.230	.246		.562				
2.30	.150	.323	.347	.372	.399	.428	.459	,491	.525	.982				
2.40	.300	.525	.672	.710	.751	.793	.837	.883	.931	1.621				
2.50	.600	1.089	1.146	1.206	1,268	1.333	1.401	1.471	1.544	2.569				
2.60	1.034		1.868	1.957	2,050	2.146	2.246	2.350	2.458	3.936				
2.70	1,700	1.783	2.930	3.059	3.193	3.331	3.474	3.623	3.777	5.783				
2.80	2.685	2.805	2.930 4,434	4.609	4,790	4.976	5,169	5.367	5.572	8.448				
2.90	4.100	4.264		6,740	7.003	7.274	7.554	7,843	8.140	11.97				
3.00	6,000	6.239	6.485	9.774	10.13	10.50	10.85	11.22	11.59	16.05				
3.10	8,764	9.091	9.428	13.61	14.05	14.50	14.88	15.26	15.65					
3.20	12.37	12.77	13.19	17.73	18.17	18.62	19.08	19.55	20.02	20.51				
3.30	16.46	16.88	17.30		22.77	23.23	23.70	24.17	24.66	25.14				
3.40	21.00	21.43	21.87	22.32	27.69	28.23	28.76	29.31	29.87	30.43				
3.50	25.64	26.14	26.65	27.17	32.92	33.41	33.91	34.41	34.92	35.43				
3.60	31.00	31.47	31.95	32.43	38.07	38.61	39.16	39.72	40.28	40.84				
3.70	35.95	36.47	37.00	37.53		44.35	44.95	45.56	46.17	46.79				
3.80	41.41	41.99	42.57	43,16	43.75	50.64	51.30	51.96	52.64	53.32				
3.90	47.42	48.05	48.69	49.33	49.98	56.80	57.37	57.94	58.52	59,10				
4.00	54.00	54,55	55.11	55.67	56.23	62.66	63.26	63.87	64.49	65,10				
4.10	59.69	60.27	60.86	61.46	62.06	68.87	69.51	70.16	70.81	71.46				
4.20	65.72	66.34	66.97	67.60	68.23	75.45	76.12	76.81	77.49	78.18				
4.20	72.11	72.77	73.44	74.10	74.77	75.45 82.39	83.10	83.82	84.54	85.27				
4.30	78.87	79.57	80.27	80.97	81.68	82.39	89.75	90.39	91.03	91.66				
	86.00	86.62	87.24	87.87	88.49		96.20	96.86	97,52	98.19				
4.50	92.31	92.95	93.60	94.24	94.90	95.55	102.9	103.6	104.3	104.9				
4.60	92.31 98.85	99,52	100.2	100.9	101.5	102.2	109.8	110.5	111.2	112.0				
4,70		106.3	107.0	107.7	108.4	109.1	117.0	117.7	118.5	119.2				
4.80	105.6	113.4	114.1	114.8	115.5	116.3	124.4	125.2	125.9	126.7				
4.90	112.7	120.7	121.4	122.2	122.9	123.7	132.1	132.9	133.7	134.4				
5.00	119.9		129.0	129.8	130.5	131.3	140.0	140.8	141.6	142.4				
5.10	127.5	128.2	136.8	137.6	138.4	139.2	140.0	149.0	149.8	150.7				
5.20	135.2	136.0	144.9	145.7	146.5	147.3	146.4	157.4	158.3	159.1				
5.30	143.2	144.0	153.2	154.0	154.9	155.7		165.6	166.5	167.3				
5,40	151.5	152.3	161.6	162.4	163.2	164.0	164.8 173.1	173.9	174.7	175.6				
5.50	160.0	160.8	169.7	170.6	171.4	172.2		182.3	183.2	184.0				
5.60	168.1	168.9	109.1	178.9	179.8	180.6	181.5	191.0	191.8	192.7				
5.70	176.4	177.2	186.6	187.5	188.4	189.2	190.1	199.8	200.7	-201.6				
5.80	184.9	185.8		196.2	197.1	198.0	198.9	208.8	209.8	210.7				
5.90	193.6	194.5	195.4 204.3	205.2	206.1	207.0	207.9	218.1	219.0	220.0				
6.00	202.5	203.4		214.4	215.3	216.2	217.2	227.5	228.5	229.4				
6.10	211.6	212.5	213.4	223.7	224.7	225.6	226.6	237.2	238.1	239.1				
6.20	220.9	221.8	222.8	233.3	234.3	235.2	236.2	247.0	248.0	249.0				
6.30	230.4	231.4	232.3	233.0	244.0	245.0	246.0	257.0	258.1	259.				
6.40	240.1	241.1	242.1	243.0	254.0	255.0	256.0	267.3	268.3	269.				
6.50	250.0	251.0	252.0	253.0	264.2	265.2	266.3	207.5						
6.60		261.1	262.1	203.2										

**Appendix A** Part of rating table published by U.S.G.S. for Old Woman creek at Berlin Road, U.S.G.S. Station 04199155, Erie County, Ohio.

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## **Appendix B** Mean daily discharges ( $ft^3$ /sec.) and hourly stages (ft.) published by the U.S.G.S. for October 1989 at Old Woman Creek at Berlin Road.

	<b>MAE 6</b> 4							5146		MUND	REDT	3 04	fEtt,	41 1	N01C4	TED H	Duts	
paté 	4015CH 4015CH	/925Cm <11m2>	MË A L G H	. NEAV 035C++	\$41FT 40j	CORA CORA	1300	1400	1500	1400	1700	0600 1800	1900	2030	21.00	2200	1330	2100
0/01/89	2.51 .04 40013>	7.50 .44 4415>	2.51		-0.23	0.42	231	250	231		251	250 251	251	230	251	251	251	
0195/84	2.53 .09 <1865>	2.30 .02 <geoo></geoo>	2.91	.05	-0.244	0.02	251	81	231 232	251 252	251	250 252	251 253	253		252	252	212
0/03/80	2.12 04 <0013>	7,49 ′ _01 <1230>	1.10		0.348.	0.02	249	244	251	247	251 249	249	250	250	249	250 249	249	249
0/64/67	2.6* _01 <0613>	2.47 .00 <14005	2.48	-01	-0.24#	D.02	248 248	" 248 247	248	24 B 24 B	248 248	248	248	- 245 248	247	Z47	247	
0/05/44	2.47 .00 <0615>	2.14 ,00 <0500>	2.66		-D.24W	0.02	247 246	247 246	247	247 246	-246 246	247 247	246 247	246 246	248	246	245	249
0/06/84	2.44	2.45	2.43	-00	-0.24w	0.02	246 243	246 245	245	246	245 245	246 245	246 245	246 245	245 245		243	
0/07/ <b>8</b> 9	<0015> 1.45 .00	<04155 2,42 .00	2.43	-90	-Q.24W	0.02	246 263	246 243	244 243	216 243	244	246 243	244 243	243 243		243		243
6708764	40C33> 2.42 .00	<1100> 2.41 .00	2,41	-50	-0.244	0.02	242 241	242 241	242	242 241	247 241	242 241	742 241	242 241		241 241		241
9/69/89	41 2_41 _06	41000+ 2.40 .00	2.+1	-0-	-0.214	0.02	241 240	249 240	241 247	243 240	241 241	269 241	241 241	241 241		261		240
0/10/89	×00153 2.80 1.7 <1015>	<0700> 2.60 .00 <0015>	2.\$7	. \$ ?	-0.19x	0.02	240 256	240 257		240 279	240 250		240 277	241 277	242 276			252 275
10/11/49	2.76	2.67 40	2.81		-0.154	D*05	275 270	274 270					213 269	277 269	27? 268	272 265		271
10/12/44	40615> 2.47 .40	<2215> 2.61 .32	2.64	. 4 .	-0.784	0.02	267	267 263	767 242	266 262		266 262	205 261	265 261	265 261	265 251	264 261	
16/13/89	40015> 2,41 ,12 40C15>	<1900> 3.59 .26 <2230>	\$. 40	.1	-0.204	0.02	261 240	264 260		260 260			2 60 2 60	260 220	260 260			
10/14/69	41.5 41.50>	2.1m .20 (240)>	2.56	. 20	-0.21+	0.02	239 756	25 0 25 4		259 255			258 259	258	255 257	256 257	257	
10715787	2,50 ,20 406132	4 6 6 5 5 5 4 0 2 0 5 5 7 4 0 2 0 5 7	2.15	-14	-0.232	0.02	250 251	255 255		236 254			255 256	255 254	235 255	235 253	255 253	
10/16/89	3.14 10 (2315)	1.52 .04 (0745)	2.1*	4.	I -0,21v	0.02	253 257	253 252	253			252	212 213	252 245	252	252 251	- 252 313	
16217244		2.91	5.04	7.1	-0.01=	0.02	312 306	505 904		374 301			314 291	313 295	312 293	292 292	30V 292	308 291
10/18/48		2,86	2.86	2.4	-0.0/*		212	289 289					- 268 284	287 244	287 284	287		286
\$0419769		2.84 2.2	3.70	46	-0.01		420	264 478			294 454		325	315 437.	347 431	- 57Ó 430	*25 *25	1 402 5 417
10/20/64	4,76 43 400150	34	3. 50	27	0*00M	0.01	410									347 319		3 330 316
10/21/49	11	7.+	3.10	0.9	a.064	0-02	515 309					301	312	31) 307	311 307	310 307		3 310 6 300
10/22/29	P. 6	2,79 5.8	5.02	6.6	0,004	0.02	30a 302							303 300	303 300			
10/23/45	5.4	2.01 3.5	7.45	4.3	-0.03W	9.02	291						295	296 293	296 292	295	Z Ø S	5 295
10/24/29	3.5	2.5	2.66	1,0	~0. Gew	0.02	241										2 a l	5 200 6 786
10/25/84	+0015> 2.85 2.5 <0015>	2.82 2.0	2.46	2.1	-0.094	a . 02	207 263	28 26	2 2 8 3 2 8	20	s 20 3 78	3 744 3 783	284	236	214 203	284 282	28	3 283 2 262
10/26/81	2.82 2,0 <0015×	1.5	2.40	1.7	-0.100	0-02	282		2 201 2 201	2 201 2 201	1 28 1 28	1 281 280	- 287	230	230 280	- 260	-280	0 200 9 279
10/77/85	2.79 1.5 40615×	2.77 1.3	2.78	1.4	-0.11	0.02	279				27 4 27			278 278	273	- 278 - 277	- 21	9 - 277 7 - 277
10726784	2.77 1.3 (0015>	2.74	2.76	1.3	+0.17w	\$0.0	211 276	271	7 27 6 27	27	6 27 7 27	276	271 271	27£ 271	276 277	276	27	6 276 6 276
10/29/69	2.7a 7.5	2.75	2.75	1.7	-0,15	¢-02	274 273	27	4 27 5 27		\$ 21 5 27				275 275	275	27	5 275 5 275
16733748	<0013> 2.75 1.1 <0015>	2.24	2.74	1.1	-0,136	U.02			27			274 5 274		276	276 275	275	27	L 276 5 275
10/31/84	3.90	2-74	2,76	1.3	+0.12	₩ <sup>~</sup> 0+02	275	27	27	5 27 7 27	3 27 7 27	4 27: 8 27:	27	5 - 274 7 - 280	275	279	27	5° 275 9 279

**Appendix** C Hourly stages (ft.) reported by the U.S.G.S. for October 1989 at the Old Woman Creek weiland at U.S. Highway 6, U.S.G.S. Station 04199165, Erie County, Ohio. Addition of 560.00 feet to the recorded stage yields the mean feet above sea level.

		STUR STAGE, IN HUNDROTHS OF ALET, AT EXDICATED HOURS
8476	MEX SH ATH GH MEAN Crthur (11Me) gh	Cola 1 2 3 4 5 4 7 9 4 10 11 12
10-31	16.82 [6.82 ]6.82 405152 400152	.00 4H 1452 1467 1467 1467 1452 1452 1452 1452 1452 1467 1467 1467 1467 1467 1467 1467 1467
10-02	16,92 94,90 14,91	PH 1498 5499 1699 1691 1691 1697 FAFE 1699 9476 1698 1698 1698
10-95	<0013> <8930> 14,90 14,97 14,89	
15-06	«CC13» «1430» 14.57 14.66 14.84	-DO AM 1487 1487 1487 1487 1488 1434 1434 1414 1414 1414 1414
10-03	(0015> 40415> 14.36 16.85 14.55	00 1446 1446 1448 1496 1486 1486 1486 1486 1416 1416 1496 1495 1495 .00 AN 1496 1416 1448 1496 1489 1489 1488 1488 1416 1416 1496 1495 1495 .01 AN 1495 1495 1483 1483 1483 1483 1483 1485 1485 1485 1495 1495
	<0013> <1315>	the star the star the star test take but the
10-96	16,95 14,82 18,84 40015> (2230)	Em 6453 1485 1483 1483 1483 1483 1484 4488 4488 4488
19-07	14.57 14.40 14.45 <8015> <2400>	WH 1485 1481 1481 1481 1481 1481 1484 4446 4440 1440 1440 1440
15-09	14.40 14.79 14.40 (2015) <1000	M 1672 3679 1679 1679 1679 1679 1778 1778 1778 1798 1678 1678
1a-a+	36.78 86.76 96.78	BY \$478 1478 TATE TATE THEY THEY THEY THEY THEN THEY THEY
10-19	<pre><pre><pre><pre><pre><pre><pre><pre></pre></pre></pre></pre></pre></pre></pre></pre>	PH 1482 1482 1483 1483 1484 1484 1484 1484 1484
	<1.15> <0013>	.00 AM 2882 7682 1682 1682 1682 1682 1682 1682 1682 1
10-11	16.92 16.91 16.82 20155 460855	00 Am 1462 1462 1468 1468 1468 1468 1468 1468 1468 1468
\$0-12	16,33 56,48 16,82 235135 41300P	PR 1686 1681 1688 1681 1681 1681 1681 168
10-13	16,61 16,00 16,87 (0015) <1830)	55 1440 1410 1400 1410 1410 1410 1410 14
12-94	14.60 14.60 14.60 (0015) (0013)	ed 1440 1460 1480 1480 1490 1470 1 14 146 146 146 146
13-15	14,40 14.78 14.79	AN TARD 1430 1480 1470 1470 1474
·	c0013> <1400>	-00 an 1478 1478 1478 1478 1478 1478 1478 1478
13-14	<21007 <0013>	
10-17	<2133> <80933>	
10-11	(gC15) <0015)	PH 1407 1407 1906 1907 1908 1912 1997 1920 1930 1930 1930
10-11	0 05.46 14.47 13.34 c2343> 40015>	-30 pe 1558 1942 1934 1947 1930 1914 1918 1917 1940 1942 1953 1994 -00 im 1944 1971 1975 1975 1974 1974 1974 1974 1974 1974 -00 im 1944 1971 1975 1976 1977 1984 1986 1996 1990 1992 1975
10-21		PM 1315 1384 1384 1986 1987
		· · · · · · · · · · · · · · · · · · ·
12-5	«2100» <0015»	10 AN 1993 1403 1403 1403 1404 1404 1404 1404 140
12-5	+2230> +0015>	30 48 1605 1403 1803 1804 1808 1808 1808 1808 1808 1808 1808
10-1	3 16.04 14.33 14.07 221939 400197	100 100 1000 1000 1000 1000 1000 1000
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10-2		PR 1411 1411 1411 1411 1411
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10-3	20011) CD013>	OD AR 1411 1411 1411 1411 1411 1411 1411 1
10-		DC AF 1611 1011 1611 1611 1611 1611 1611 161
10-		00 44 1218 1221 1221 1219 1219 1225 1227 1227 1228 1226 1225 1227
10-	24 12.29 52.16 12.24	m an 1225 1225 1216 1214 1214 1215 1215 1228 1226 1324 1222 1220
10-	10 12.30 12.13 12.21	no an (223 1218 12(7 1217 1212 115 1137 1146 1134 1142 1144
10-		PH 1100 1103 1103 110 110 110 110 110 110
	<1000> E3915>	74 1196 1200 1193 1188 1180 1180 1161 1161 1161 1181 1136 1165 1161
+1-	-01 12.37 10.99 19.74	

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Deviation from IGLD <sup>§</sup> meters	IGLD feet	USGS* feet	Area 10 <sup>3</sup> m <sup>2</sup>	Volume 10 <sup>3</sup> m <sup>3</sup>	Deviation from IGLD <sup>5</sup> meters	IGLD feet	USGS* feet	Area 10 <sup>3</sup> m <sup>2</sup>	Volume 10 <sup>3</sup> m <sup>3</sup>
2.0	575.16	576.76	650	777			574.05	530	285
1.9	574.84	576.44	646	712			574.00	529	277
1.8	574.51	576.11	642	648			573.95	528	269
	J/4.JI	576.10	641	646			573.90	527	261
		576.05	637	636			573.85	526	253
		576.00	634	627					
					1.1	572.21	573.81	526	246
		575.95	630	618			573.80	525	245
		575.90	626	608			573.75	524	237
		575.85	623	599			573.70	524	229
		575.80	619	590			573.65	523	221
1.7	574.18	575.78	617	586			573.60	522	213
		575.75	615	580			573.55	521	205
		575.70	612	571			573.50	520	197
		575.65	608	562					
					1.0	571.88	573.48	519	194
		575.60	604	552			573.45	515	190
		575.55	601	543			573.40	507	182
1.6	#33.0F	575.50	597	533			573.35	498	175
	573.85	575.45	593	524			573.30	490	168
		575.40	591	515			573.25	482	161
		575.35	589	507			573.20	474	154
		575.30	587	498	0.9	571.55	573.15	466	147
		575.25	584	489					
							573.10	458	139
		\$75.20	582	480			573.05	450	132
	673.63	575.15	580	472			573.00	442	125
1.5	573.52	575.12	578	466			572.95	434	118
		575.10	577	463					
		575.05	575	454			572.90	426	111
		575.00	573	445			572.85	418	104
		\$74.95	570	437	0.8	571.23	572.83	414	101
		574.90	568	428			572.80	405	98
		574.85	566	419			572.75	390	93
		574.80	564	410			572.70	374	89
1.4	573.19	574.79	563	408			572.65	359	84
	010129	574.75	561	402			572.60	343	79
							673 55		76
		574.70	559	393	0.7	570.90	572.55	328	75
		574.65	556	385	0,7	570.90	572.50 572.45	313	70
		574.60	554	376			572.45	297	65
		574.55	551	368			572.40	282	61
							572.35	266	56
		574.50	549	360			572.30	251	51
1.3	572.86	574.47	547	354			572.25	236	47
		574.45	547	351			572.20	220	42
		574.40	544	343	0.6	570.57	573 17		20
		574.35	542	334	<i></i>	210.31	572.17 572.15	211 206	<b>39</b> 39
		574.30	539	326					
		574.25	537	318			572.10	194	37
		574.20	535	309			572.05	181	35
1.2	572.54	574.15	532	301					
	312.34	574.14	532	<b>299</b>					
		574.10	531	<u>293</u>					

Appendix D Expanded hypsographic and depth-volume table for Old Woman Creek wetland  $^{\dagger}$ 

Appendix D Continued

Deviation from IGLD <sup>§</sup> meters	IGLD feet	USGS* feet	Area 10 <sup>3</sup> m <sup>2</sup>	Valume 10 <sup>3</sup> m <sup>3</sup>
		572.00	169	33
		571.95	156	31
		571.90	144	30
		571.85	131	28
0.5	570.24	571.84	129	27
		571.80	119	26
		571.75	107	24
		571.70	94	22
		571.65	82	21
		571.60	69	19
		571.55	57	17
0.4	569.91	571.51	47	16
	•••	571.50	46	15
		571.45	45	15
		571.40	43	14
		571.35	41	14
		571,30	39	13
		571.25	38	13
		571.20	36	12
0.3	569.59	571.19	36	12
		571,15	34	12
		571.10	32	11
		571.05	31	11
		571.00	29	10
		570.95	27	10
		570.90	25	9
0.2	569,26	570.86	24	9
0.0	568.60	570.20	18	- 4
-8.2	567.94	569.54	7	2
-0.4	567.29	568.89	2	1
-0.6	566.63	568.23	1	8.0
-0.8	566.34	567.94	1	0.5
-1.0	565.32	566.92	1	0.3
-1.2	564.66	566.26	0.7	0.1
-14	564.01	565.61	0.3	9.93

International Great Lakes Datum

\* USGS feet = IGLD feet + 1.60

<sup>†</sup> Bolded values were reported by Herdendorf and Hume (1991); other values were interpolated assuming linear change between the bolded values. Areas and volumes are rounded to 3 or fewer significant figures.

#### The Heidelberg College Water Quality Laboratory

The Water Quality Laboratory (WQL), founded in 1969, is an environmental research and education organization associated with the science departments of Heidelberg College. The WQL operates large scale, long term programs that (1) monitor impacts of agriculture on surface and ground water quality and (2) track progress associated with agricultural pollution abatement programs. The laboratory employs eight full time staff and several student technicians.

In its Lake Erie Tributary Monitoring Program, The WQL measures the concentration of pollutants in the major rivers draining into Lake Erie, including the Raisin, Maumee, Sandusky, Cuyahoga, and Grand Rivers. This program continues to provide the most detailed and longest term records available in the United State regarding the concentration of sediments, fertilizers and pesticides in rivers draining intensive row crop agriculture.

Through its Cooperative Private Well Testing Program, the WQL works with local county organizations to provide a low cost well testing program to residents who will allow data from their wells to be added to a local data base. Since most ground water pollution problems result from local causes, local data are essential to develop appropriate and effective ground water protection programs. Since 1987 the program has included 340 counties in 15 states, and 37,000 private wells.

The data collected by the WQL are used extensively by public and private organizations. In addition to the publication of reports and articles, the WQL operates an Environmental Extension Program, providing about 60 presentations per year to agricultural, environmental and general audiences throughout Ohio and the Midwest.

Financial support for WQL programs comes from state and federal agencies, industries, private foundations, and participants in the Cooperative Private Well Testing Program.