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## **Emulation as an Approach for Rapid Estuarine Modeling**

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## 14 **Abstract:**

15 Probabilistic flood hazard assessment is a promising methodology for estuarine risk assessment  
16 but currently remains limited by prohibitively long simulation times. This study addresses this  
17 problem through the development of an emulator, or surrogate model, which replaces the  
18 simulator (in this case the coupled ADCIRC+SWAN model) with a statistical representation that  
19 is able to rapidly predict estuarine variables relevant to flooding. Emulation of water levels  
20 (WLs), non-tidal residual, and significant wave height, is explored at Grays Harbor, Washington  
21 (WA) USA using Gaussian process regression. The effectiveness of the methodology is validated  
22 at various model simplification levels to determine where error is being sourced. Emulated WLs  
23 are found to be skillful when compared to over a decade of tide gauge observations (root mean  
24 square error, RMSE, <15 cm). The largest loss of skill in the method originates with  
25 ADCIRC+SWAN attempting to reproduce observations, even when the majority of relevant  
26 physics are included. Subsequent simplifications to the simulator (input reduction techniques)  
27 and the emulator itself are found to introduce a trivial amount of error (average increase in  
28 RMSE of 1 cm). Emulated WLs are also compared to spatially varying observations and found to  
29 be equally skillful throughout the estuary. An example emulation application is explored by  
30 decomposing the relative forcing contributions to extreme WLs across the study site. Results  
31 show a compound nature of extreme estuarine WLs in that all forcing dimensions contribute to  
32 extremes, with streamflow having the least influence and tides the largest. Overall the approach  
33 is shown to be both skillful and efficient at reproducing critical hydrodynamic variables,  
34 suggesting that emulation may play a key role in improving our ability to probabilistically assess  
35 flood risk in complex environments as well as being promising in a range of other applications.

36 **Keywords:** Emulation; Gaussian Process Regression; ADCIRC+SWAN; Estuary; Probabilistic  
37 Modeling; Water Levels

## 38 **1. Introduction**

39 Modeling estuarine hydrodynamics remains both a challenge and a goal for the scientific  
40 community. Estuaries and bays are often densely populated with significant economic and  
41 cultural investment [Pendleton, 2010]. They are also subject to a unique flood hazard  
42 environment, with high water levels (WLs) driven by numerous contributing processes including  
43 both offshore and local waves, storm surge, and river inflows, among others. Over the past

44 several decades, research efforts have led to improved computational models and increased  
45 physical understanding of estuarine flood dynamics [Bode and Hardy, 1997; Kantha and  
46 Clayson, 2000; Ganju et al., 2015]. However, increasing hydrodynamic model predictive skill is  
47 generally coupled to increasing complexity within numerical models and a correspondingly  
48 larger computational load. This has led to computational time, rather than a physical  
49 understanding of the problem, being a limiting control on our ability to answer questions about  
50 estuarine flooding.

51         Increasing computer processing power and code parallelization has pushed the boundary  
52 for what can be explored with complex computer codes. However, even with these advances,  
53 many questions still cannot be comprehensively addressed due to computational limitations. One  
54 example is the recent focus by the scientific community on uncertainty in model results  
55 [Mastrandrea et al., 2010; Green et al., 2011]. In the field of flood hazards, a major thrust area  
56 has been probabilistic assessments, which brings the benefits of uncertainty quantification, utility  
57 as a stakeholder-centered decision making tool, better handling of extreme events, and more  
58 skillful flooding estimates [Cloke and Pappenberger, 2009; Di Baldassarre et al., 2010; Dale et  
59 al., 2014]. However, the combination of multiple model iterations (required for probabilistic  
60 modeling) and large per-run computational costs has remained a barrier for moving forward.

61         Often the solution to long simulation times is a compromise, such as simplifying or  
62 eliminating various forcing components [Purvis et al., 2008; Lin et al., 2010]; using smaller  
63 ensemble sizes [Davis et al., 2010]; or simplifying model physics [Dawson et al., 2005; Moel et  
64 al., 2012]. A promising recent development has been to implement variable model complexity,  
65 with a fast model determining relevant or extreme events and a more highly-resolved, accurate  
66 model being used to simulate the extremes [Lin et al., 2010, 2012; Orton et al., 2016]. This  
67 technique has been successfully demonstrated for hurricane-induced flooding but is potentially  
68 problematic for other regions. For example, environments not dominated by tropical cyclones  
69 often are defined by compound events where combinations of non-extreme forcings can combine  
70 to create extremes [Leonard et al., 2014; Wahl et al., 2015; Moftakhari et al., 2017; Zscheischler  
71 et al., 2018]. In addition, event based techniques can still be considered computationally limited  
72 as the full parameter space cannot usually be explored. There remains a need for a modeling  
73 technique that can bridge the gap between time-intensive, complex models and fast simulation  
74 times.

75           This paper investigates emulation as a technique for the efficient prediction of estuarine  
76 hydrodynamic variables in Grays Harbor, Washington (WA) USA. The foundational idea of  
77 emulation (also referred to as surrogate modeling, response surface modeling, and meta-  
78 modeling, among others) is the replacement of a slower processes-based model (a simulator)  
79 with a fast, statistical model (an emulator) [O’Hagan, 2006; Razavi et al., 2012]. In the standard  
80 modeling paradigm, the map between simulator inputs and outputs is based on the laws of  
81 physics as implemented within a process-based model [Castelletti et al., 2012]. In emulation, this  
82 map is approximated using a statistical model. The benefit is that, following an upfront  
83 computational expense to create a training dataset and train the emulator, applying the emulator  
84 is nearly instantaneous. Thus, emulation represents a tradeoff between short simulation times and  
85 errors associated with the approximation. This tradeoff suggests that emulation may be ideal for  
86 probabilistic flood modeling along with many other potential applications including assessments  
87 of model uncertainty, model optimization, sensitivity analysis, real time forecasting, and extreme  
88 event analysis [Oakley, 1999; Kennedy et al., 2006; Levy and Steinberg, 2010].

89           The general concept of emulation originated in the 1980s through the idea of computer  
90 experiments [Sacks et al., 1989]. Since then, emulation ideas have spread widely resulting in a  
91 rich literature of applications, emulator formulations, and theories from numerous fields. Razavi  
92 et al. [2012] reviews emulation in the field of water resources, with over 30 studies revealing a  
93 wide range of applications and emulation approaches. As a brief overview of coastal  
94 applications, Gouldby et al [2014], Malde et al. [2016b] and Rueda et al. [2016] successfully  
95 implemented emulators for wave prediction problems using SWAN [Booij et al., 1997] as a  
96 simulator. The pairing of SWAN and emulation was extended to delineating offshore conditions  
97 causing wave induced coastal flooding by Rohmer and Idier, [2012] by using kriging and an  
98 adaptive sampling technique. Timmermans [2015] used emulation to explore how tuning  
99 parameters affect uncertainty in results from the Wave Watch III [Tolman, 2009] wave model.  
100 Liu and Guillas [2017] investigated the effect of uncertainty in bathymetry on tsunami height  
101 predictions using a novel merging of Gaussian process regression (GPR) emulation with  
102 dimensional reduction techniques.

103           In the context of flooding, emulation has been applied to river channel flooding [Apel et  
104 al., 2008] and coastal dyke systems [Moel et al., 2012], although from the relatively simplistic  
105 perspective of lookup tables. Surge response functions [SRF; Resio et al., 2009; Song et al.,

106 2012] can be considered a specific case of emulation through regression of dimensionless  
107 cyclone scaling terms. However, SRFs are limited in application to tropical cyclones, and have  
108 been shown to perform poorly in complex environments [Taylor et al., 2015]. As an alternative  
109 to SRFs, Kim et al. [2015] used an artificial neural network to emulate coupled  
110 ADCIRC+STWAVE calculated surge from tropical cyclones. This approach was enhanced by  
111 Bass and Bedient [2018] who used a similar strategy but with the addition of a coupled  
112 hydrologic model and GPR as the emulator formulation. Jia et al. [2013; 2016] used GPR  
113 emulation for predicting tropical cyclone surges.

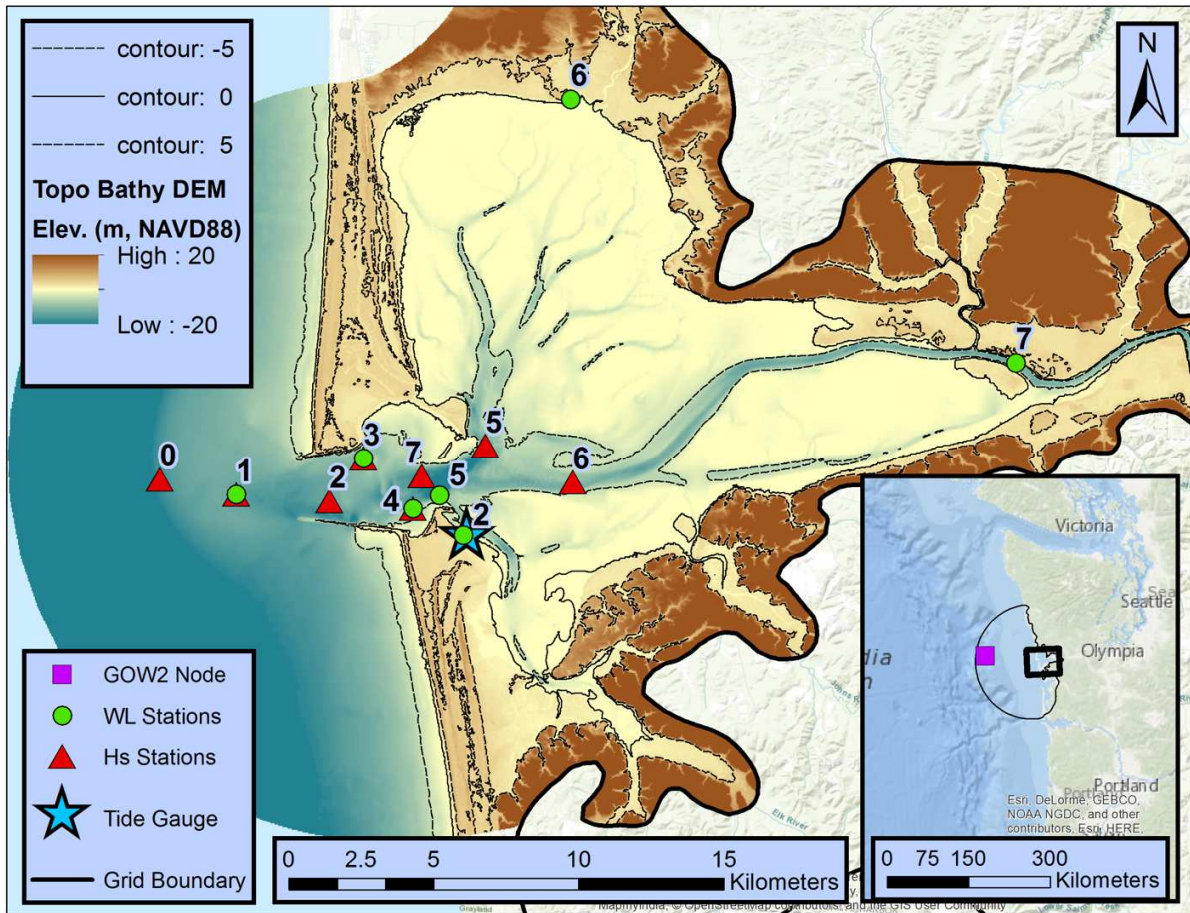
114 Overall, multiple studies have demonstrated the potential of emulation in a coastal hazard  
115 setting. Surge from tropical cyclones has, in particular, seen a variety of successful emulator  
116 implementations. This study builds on these recent efforts but explores an estuary in the USA  
117 Pacific Northwest (PNW) that does not experience tropical cyclone forcing. This results in a  
118 unique challenge in terms of handling diverse forcings and a potentially larger input parameter  
119 space, since there is no dominant forcing dimension. Other studies focused on predicting WLs,  
120 such as those by Jia et al. [2013; 2016], Kim et al. [2015], and Bass and Bedient [2018], reduce  
121 input dimensionality through considering only cyclones and using discrete cyclone  
122 characteristics as input dimensions. This study, however, considers a general application of  
123 emulating the coupled ADCIRC+SWAN [ADCSWAN; Dietrich et al., 2011] simulator in which  
124 any combination of forcings can be used to calculate WLs. This paper is intended as a rigorous  
125 investigation into the applicability of emulation in this new context. Therefore, the focus here is  
126 primarily on describing the methodology and validation and only a single application,  
127 decomposing extreme estuarine water levels, is presented.

## 128 **2. Study Sites and Observations**

### 129 **2.1 Study Site**

130 Grays Harbor, WA (Figure 1) is an excellent candidate for testing emulation as it exhibits  
131 many of the complexities that make estuarine modeling difficult. Grays Harbor is predominantly  
132 shallow, dominated by depths averaging less than 5 meters, but also contains a maintained  
133 (United States Army Corps of Engineers; USACE) deep-water navigation channel giving it  
134 significant depth variability (Figure 1). The bay exhibits spatial variability in WLs [Cialone et

135 al., 2001] as a result of its size [approximately 235.3 km<sup>2</sup>, Engle et al., 2007], shape, and  
136 gradients in forcing. Grays Harbor is located in the PNW (Figure 1) and is therefore subject to an  
137 energetic storm and wave climate. A Global Ocean Wave 2 (GOW2) reanalysis [Perez et al.,  
138 2017] near the study site (see Figure 1) reveals a mean offshore significant wave height (H<sub>s</sub>) of  
139 2.5 meters with events exceeding 7.5 meters annually. Extreme storm events are generally  
140 associated with extratropical cyclones that can produce strong winds, pressure differentials, and  
141 precipitation [Allan and Komar, 2002b; Mass and Dotson, 2010]. These events are often  
142 associated with significant non-tidal residuals (NTR) [Allan and Komar, 2002b, 2006; Allan et  
143 al., 2011; Serafin et al., 2017], although of a smaller magnitude than locations impacted by  
144 tropical cyclones or with broader continental shelves [Zhang et al., 1999]. Within this study,  
145 NTR is defined as an observed or modeled WL with tides removed (with the specifics of how  
146 NTR is calculated detailed in section 4.3). Grays Harbor has significant hydrological input from  
147 the Chehalis, Humptulips, Hoquiam, Elk, and Johns Rivers which collectively drain a watershed  
148 of over 7,000 km<sup>2</sup> for an average monthly runoff volume of 22 million m<sup>3</sup>/month [Engle et al.,  
149 2007].



150

151 Figure 1: Grays Harbor, WA study site and locations of observational datasets. Circles and triangles  
 152 represent USACE deployments with co-located instruments labeled with a single number representing  
 153 both WL and Hs stations. The main panel shows the bathymetry and topography of the estuary in the  
 154 NAVD88 Datum. The inset panel shows the larger geographical context of the estuary with the thin black  
 155 line delineating the domain of the hydrodynamic model. The purple square within the inset is the location  
 156 of the utilized GOW2 node (located at 47° N, 125° W).

157 2.2 Observational Data

158 This study utilizes a variety of observational datasets ranging from instrument  
 159 deployments to reanalysis products. Forcing and model development datasets are explained in  
 160 the following section (2.2.1), while section 2.2.2 details observations specifically used for model  
 161 validation.

### 162 2.2.1 Forcing and Model Development Datasets

163 Wave forcing for the model was obtained from the GOW2 reanalysis of Perez et al.  
164 [2017] with output selected from a node located at (Lat: 47° N, Lon: 125° W; Figure 1).  
165 Atmospheric forcing was provided by the North American Regional Reanalysis (NARR)  
166 [Mesinger et al., 2006]. NARR provides a wide range of gridded atmospheric variables from  
167 which the 3-hourly 10m wind fields and 3-hourly surface pressure fields were utilized.  
168 Streamflow was obtained from USGS river gauges with total estuary inflow constructed as the  
169 sum of three gauged rivers, the Chehalis, Satsop, and Wynoochee (USGS stations 12031000,  
170 12035000, and 12037400 respectively). The Satsop and Wynoochee rivers are tributaries to the  
171 Chehalis river which join the Chehalis below the Chehalis gauge. Therefore, the sum of these  
172 three gauges reproduces the majority of the Chehalis flow into the Grays Harbor estuary. While  
173 Grays Harbor has other river inlets, the majority of the input flow is concentrated at the Chehalis  
174 River which captures around 80% of the watershed area. For simplicity, as well as due to  
175 temporal availability of gauge data, only the Chehalis input (as constructed from the three  
176 gauged rivers) is included in the study with all other streamflow inputs assumed to be minimal  
177 with only local influences on variables of interest.

178 The bathymetry data for the simulator grid were developed by blending two National  
179 Oceanic and Atmospheric Administration (NOAA) digital elevation models (DEMs): the  
180 Astoria, OR tsunami DEM (1/3 arc second) and the coastal relief model (3 arc seconds) [NOAA  
181 National Centers for Environmental Information, 2003; Love et al., 2012]. Bay topography was  
182 sourced from Oregon Department of Geology and Mineral Industries (DOGAMI) LiDAR  
183 [DOGAMI, 2010].

### 184 2.2.2 Validation Datasets

185 In addition to forcing, a series of observational datasets were used to validate simulated  
186 and emulated variables within the study site. The first dataset is the Westport, WA tide gauge  
187 (NOAA station ID # 9441102) which provides continuous hourly WL data beginning in 2006.  
188 WL observations were decomposed into constituent components (e.g., deterministic tide,  
189 monthly mean sea level anomalies (MMSLA), storm surge etc.) using the approach described in



190 Serafin and Ruggiero [2014]. The five largest NTR events on record were extracted for testing  
 191 model skill. A brief summary of these storm events is provided in Table 1.

192 Table 1: Summary of forcing for the five largest NTR events at Grays Harbor, WA. Forcing values are  
 193 reported at the occurrence of maximum NTR.

	<b>Storm 1</b>	<b>Storm 2</b>	<b>Storm 3</b>	<b>Storm 4</b>	<b>Storm 5</b>
Date	12/15/06	12/3/07	01/12/14	12/12/14	12/11/15
Non-Tidal Residual (m)	0.73	0.93	0.66	0.58	1.11
Significant Wave Height (m)	8.0	11.1	8.5	6.2	11.1
Peak Wave Period (sec)	12.7	15.4	14.3	12.2	18.2
Wave Direction (deg.)	243	195	259	229	248
Surface Pressure (hPa.)	977	989	989	986	984
Wind Speed (m/s)	17.5	19.2	14.9	9.1	12.3
Wind Direction (deg.)	217	201	238	218	198
Streamflow (m <sup>3</sup> /s)	1270	2020	860	670	1510

194

195 Water level observations at the tide gauge were supplemented by a field campaign carried  
 196 out by the USACE from September-December 1999 [Figure 1, Cialone et al., 2001; 2002]. This  
 197 dataset includes seven locations near the inlet with bottom mounted tripods measuring wave  
 198 characteristics, WLs, tidal currents, and suspended sediment concentrations. Additionally, five  
 199 surface stations were distributed throughout the bay measuring WLs, conductivity, and  
 200 temperature. The USACE field campaign was broken up into two deployments (with a small  
 201 maintenance/data collection break between the two). Instruments were replaced in approximately  
 202 the same location except for Hs station 0 which was moved to location 7 for the second  
 203 deployment [Cialone et al., 2002]. Figure 1 illustrates the spatial distribution of the various  
 204 observation stations which have been renamed in this paper for clarity.

### 205 **3. Methods**

#### 206 **3.1 Simulator Configuration**

207 This study utilizes the coupled Advanced Circulation [ADCIRC; Luetlich and Westerink,  
 208 1992] and unstructured Simulating Waves Nearshore [SWAN, Zijlema, 2010] simulator

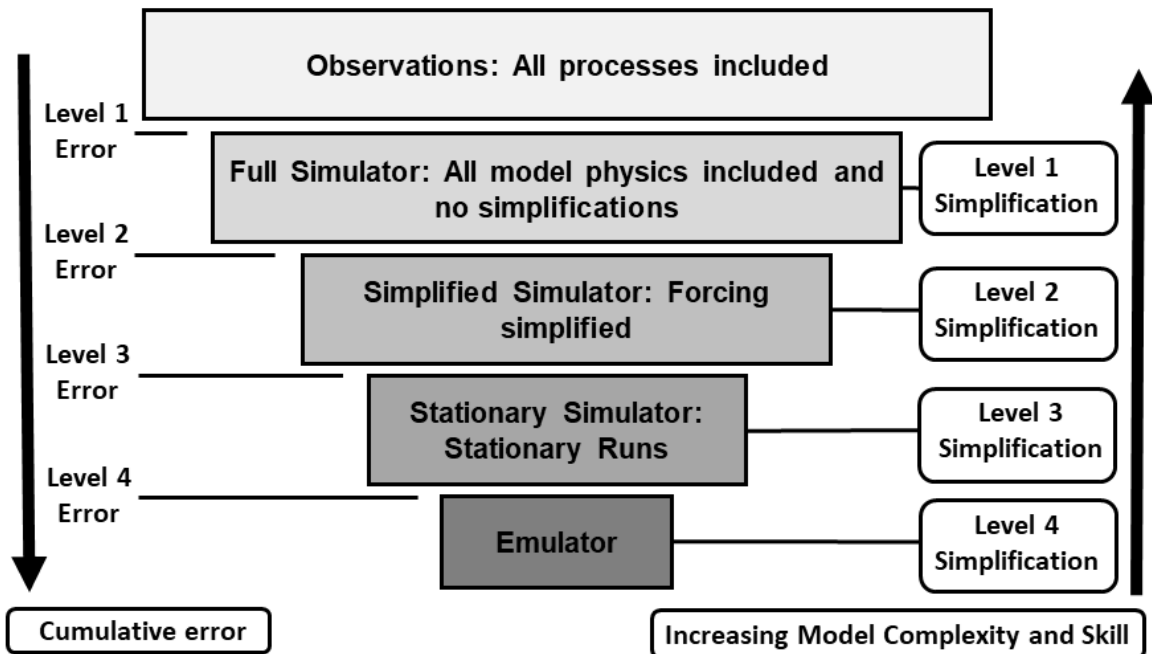
209 [ADCSWAN; Dietrich et al., 2011]. ADCSWAN has seen extensive validation and success in  
210 predicting WLs and NTR at various estuaries around the world [Dietrich et al., 2012; Bhaskaran  
211 et al., 2013; Krien et al., 2015]. Recently, ADCSWAN has been successfully implemented in the  
212 PNW with good agreement between simulator output and observations of WLs, NTR, and  
213 currents [Cialone et al., 2002; Cheng et al., 2015b]. ADCSWAN is implemented in the 2D depth-  
214 integrated barotropic mode which has been shown to perform with acceptable error for  
215 computing WLs and depth integrated currents in estuaries [Resio and Westerink, 2008; Weaver  
216 and Luettich, 2010]. ADCIRC is run in the fully 2-way coupled implementation with SWAN,  
217 which has been shown to be critical for resolving interactions between waves and nearshore  
218 hydrodynamics [Cialone et. al., 2002; Funakoshi et al., 2008; Dietrich et al., 2010, 2011;].  
219 ADCSWAN is run on an unstructured mesh that extends beyond the continental shelf  
220 (approximately 115 kilometers offshore; Figure 1). Unstructured meshes provide flexibility in  
221 simulator resolution with the utilized model grid having element sizes ranging from around 7,000  
222 meters offshore to under 20 meters within the inner Grays Harbor channel.

### 223 3.2 Dimensional Reduction and Levels of Simplification

224 Emulator construction requires sampling the full input parameter space. This constraint  
225 dictates that the number of times the simulator must be run to create the training dataset is  
226 proportional to the number of dimensions included as inputs. In general, process-based  
227 hydrodynamic simulators are based on many inputs making some form of dimensional reduction  
228 necessary. Emulator construction thus requires finding a balance between minimizing the  
229 number of inputs and maintaining sufficient complexity to acceptably resolve output variables of  
230 interest.

231 Figure 2 provides a conceptual model of the dimensional reduction approach taken in this  
232 study (through simplifications), transforming the full process-based simulator (ADCSWAN) into  
233 an emulator. Each of the simplifications, noted on the right side of Figure 2, theoretically  
234 introduces some level of error into the output, noted on the left side of Figure 2. These errors are  
235 discussed in this paper both as individual contributions, and in the cumulative (sum of all errors  
236 up to a given level) sense. When discussed explicitly in this paper, simplification levels will be  
237 capitalized. For example, a comparison of model output from the level 3 simplification  
238 (Stationary Simulator) to Observations (no simplification) quantifies the cumulative level 3 error.

239 The following sections (3.2.1 – 3.2.3) explain each simplification in this hierarchy while  
 240 corresponding error is quantified in the Results section.



241  
 242 Figure 2: Hierarchy of model simplifications between observations (top) and emulator output (bottom).  
 243 Each simplification is associated with a level (right side of the figure) and some amount of error (defined  
 244 on the left side of the figure).

245

### 246 3.2.1 Simulator Simplifications

247 The first level of simplification is simply that of using a process-based simulator.  
 248 Simulators are unable to exactly reproduce observations for a variety of reasons ranging from  
 249 incorrect or unresolved physics (e.g., assumptions, parameterizations, etc.) to numerical  
 250 approximations (truncation errors, etc.) to incorrect or biased input forcing. The x induced by  
 251 this simplification is primarily a function of the chosen model, model tuning, and the quality of  
 252 forcing/bathymetric information. Research has shown that errors in model inputs such as  
 253 bathymetry and mesh resolution [Bunya et al., 2010; Weaver and Slinn, 2010] and forcing fields  
 254 [Madsen and Jakobsen, 2004; Lewis et al., 2013; Lakshmi et al., 2017] are significant sources of  
 255 model error. Therefore, the specific configuration and choice of ADCSWAN (section 3.1) and

256 the quality of observational data (section 2.2.1) are the primary controls on the impact of this  
257 simplification.

258 This study considers emulation of a specific implementation of the ADCSWAN model  
259 and therefore the model grid (bathymetry, resolution, etc.) is held constant. Additionally,  
260 ADCSWAN contains a large number of input switches, tuning parameters, forcing options,  
261 numerical configurations, and other choices [Westerink et al., 1992]. This study holds all general  
262 model configuration parameters constant leaving the various forcing components of WL  
263 variability as the sole driver of input dimensionality within the emulator.

### 264 3.2.2 Forcing Simplifications

265 Even with the simplification of holding the model configuration fixed, the input  
266 dimensionality remains high, due to the numerous physical forcing mechanisms. Below we  
267 describe simplifications that reduce the model dimensionality to 16. This reduction is desirable  
268 since it requires a smaller training dataset and therefore produces a more efficient emulator  
269 construction.

#### 270 3.2.2.1 Wave Simplification

271 It is well known that offshore wave energy can impact water levels within bays such as  
272 Grays Harbor [Olabarrieta et al., 2011; Cheng et al., 2015b]. Wave forcing is implemented in the  
273 simulator using a JONSWAP spectrum fitted to peak wave period ( $T_p$ ),  $H_s$ , mean wave direction  
274 (MWD), and directional spread parameters. While research has shown the importance of forcing  
275 with full directional spectra for reproducing wave observations [Rogers et al., 2007; Montoya et  
276 al. 2013], most studies accounting for wave influence on WLs use simpler bulk parameter-based  
277 formulations. Therefore, a fitted JONSWAP spectrum is used for both the Full (level 1) and  
278 Simplified Simulator (level 2) comparisons. Based on previous research in the PNW [Cheng et  
279 al., 2015a], directional spread is held constant at 20 degrees, and wave forcing is applied  
280 uniformly along the Full Simulator open boundary (Figure 1). With these simplifications, wave  
281 forcing is included in the emulator as three dimensions:  $H_s$ ,  $T_p$ , and MWD.

### 282 3.2.2.2 *Atmospheric Simplification*

283 Atmospheric forcing represents a unique challenge for emulation due to the spatial  
284 variability of wind and pressure fields. Gridded inputs represent a high degree of dimensionality,  
285 with every node potentially representing an input dimension. For this reason, a sensitivity study  
286 was undertaken to see if spatially constant atmospheric forcing could be used as an  
287 approximation of the full forcing fields. WL output from simulator runs with full gridded forcing  
288 were compared to runs with spatially constant forcing. Results indicated (not shown) that the  
289 error introduced in predicted WLs by the spatially constant assumption was acceptable in  
290 comparison to the corresponding reduction in dimensionality. This error is quantified in the  
291 Results section (along with other simulator simplifications) as level 2 error. Adopting the  
292 spatially constant assumption, atmospheric forcing is reduced in the emulator framework to three  
293 dimensions: wind speed, wind direction, and sea level atmospheric pressure.

### 294 3.2.2.3 *Tidal Simplification*

295 Tidal forcing is generally represented in hydrodynamic models through harmonic  
296 constituents. Many studies using ADCIRC are forced with eight or fewer constituents, mainly  
297 because global databases of tidal constituents (e.g., TPXO [Dushaw et al., 1997], or LeProvost  
298 [Le Provost et al., 1994]) are typically limited to that number. Despite this, simulations using this  
299 small number of constituents are typically found to agree well with both harmonic analysis  
300 derived and observed tidal elevations [Westerink et al., 1992; Blain and Rogers, 1998; Blain et  
301 al., 2001]. ADCIRC simulates tidal forcing as a boundary elevation time series [Luettich et al.,  
302 1992] determined by a spatially variable, temporally constant phase and amplitude and a  
303 temporally variable, spatially constant equilibrium argument and nodal factor. Amplitudes and  
304 phases are determined by the simulator boundary location and are therefore not an emulator input  
305 dimension when considering a fixed study site. The nodal factor represents adjustments of the  
306 amplitude/phase of each constituent that results from the nodal tide cycle. The equilibrium  
307 argument (deterministic based on date and time) controls the timing of the harmonic.

308 While tides are deterministic, they are included within the emulator as forcing for a  
309 variety of reasons which will be described in section 3.2.3. In approaching simplifications, a  
310 sensitivity test was performed to determine the tidal dimensionality required for accurately

311 reproducing maximum WLs during storm events (Table 1). It was found that removing the nodal  
312 factor did not significantly change simulated WLs. After this simplification, results showed that  
313 eight harmonics (without nodal factors) were sufficient for accurately producing WLs. This  
314 allows tides to be included in the emulator as eight input dimensions: 8 harmonic equilibrium  
315 arguments, each ranging from 0 to 360 degrees.

#### 316 *3.2.2.4 Streamflow Simplification*

317 Streamflow is represented in ADCIRC as a flux of water into the domain (specified as a  
318 normal flow per unit width of boundary). This allows the simulation of large rivers that have  
319 significant cross-channel velocity profiles and for calibration where data on these cross-channel  
320 profiles are available. For this study, we instead specify a laterally constant velocity profile  
321 across each river boundary. This simplification is common [Bunya et al., 2010; McKay and  
322 Blain, 2010], especially if the boundary is far enough away from the area of interest that a  
323 natural flow profile can develop. This allows streamflow to be represented as a single input  
324 dimension (the total volumetric flow rate) for each river inlet.

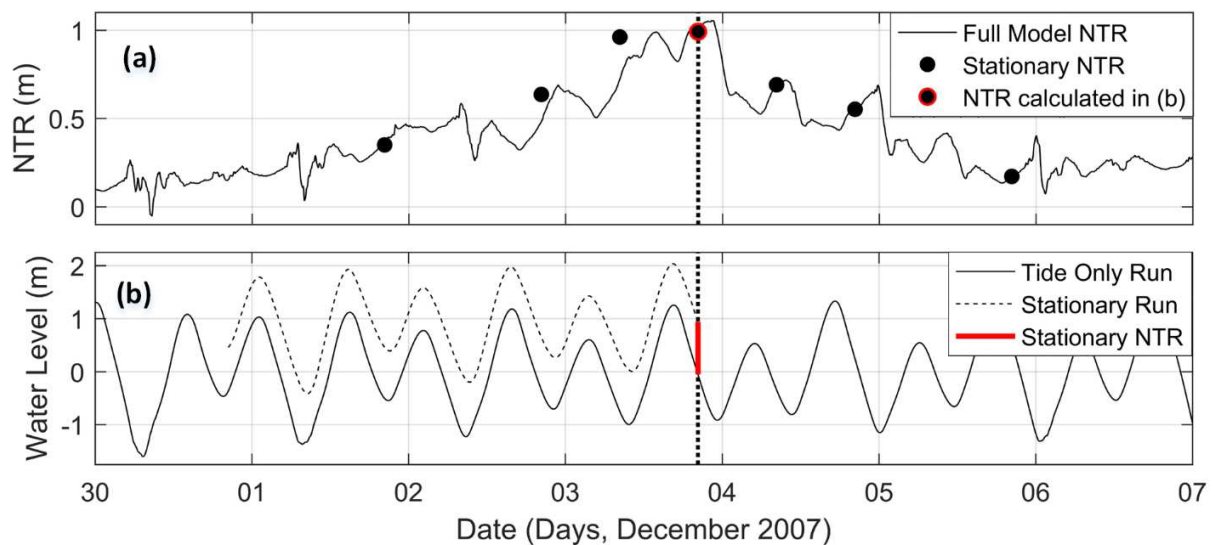
#### 325 *3.2.2.5 Base Water Level Simplifications*

326 A final input dimension is considered within the emulator framework as a “Base WL”  
327 parameter. This is included to account for large scale changes to estuary sea level, as is  
328 experienced through MMSLAs, seasonal variability, and sea level rise (SLR) [Serafin and  
329 Ruggiero, 2014]. These forcing dimensions are defined in the simulator simply as a static change  
330 to mean sea level and are therefore included in the emulator as a single input dimension.

#### 331 *3.2.3 Simulator Stationarity Simplification*

332 ADCSWAN and other process-based hydrodynamic simulators are dynamic in that both  
333 inputs and outputs are functions of time and the simulator state is determined, in part, by  
334 previous states. Seeking simplicity, this study makes the assumption that the dynamic system can  
335 be approximated using a series of stationary simulations. Precedents for such an assumption exist  
336 for coastal systems, including spectral evolution in wave modeling (SWAN) approximated using  
337 a series of steady-state simulations [Rogers et al., 2007; Rusu and Pilar, 2008].

338 Simplifying tidal forcing with stationary simulations is difficult since there is no tidal  
 339 equilibrium in WLs. One approach would be to consider tides as a series of horizontal water  
 340 surfaces of different elevations (corresponding to tidal phases). This would reduce tidal forcing  
 341 dimensionality to a single value (tidal WL), but at the cost of losing spatial variability. Testing  
 342 showed that, for the Grays Harbor study site, tidal wave evolution and propagation across the  
 343 estuary results in significant spatial variability in tidally forced WLs. A second approach would  
 344 be to decouple NTR and tidal WLs and add the two as a linear summation. However, further  
 345 testing confirmed that this simplification results in significant error. Therefore, a hybrid solution  
 346 was developed in which all non-tidal forcing is stationary, but tides are computed dynamically  
 347 with model output recorded only at the specific moment of interest. This approach is appropriate  
 348 since tides are deterministic and, for a specific set of equilibrium arguments, the previous state of  
 349 tide induced WLs will always be the same. This approach allows tidal forcing to be simplified  
 350 but retains the spatial variability in tidal WLs and the nonlinear interactions with other processes.  
 351



352  
 353 Figure 3: Panel (a): Comparison of NTR during storm 2 from a fully dynamic simulation (black line) and  
 354 simplified stationary simulations (black dots). Panel (b): Example stationary run (at the peak of the storm)  
 355 showing how the stationary NTR is calculated. The horizontal bold dotted line represents the time of the  
 356 stationary run. At this time, NTR is calculated by subtracting the value of a tide only run from the value  
 357 of the stationary run (Bold red line). This NTR value is plotted as a red outlined dot in panel (a).

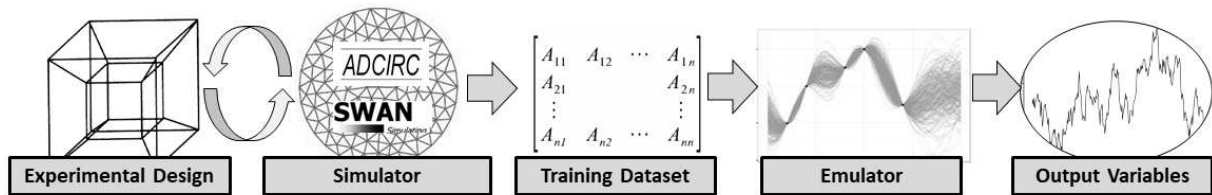
358

359 Figure 3 illustrates how stationary runs compare to the Full Simulator (dynamic). Figure  
 360 3a compares NTR from the fully forced ADCSWAN (simplification level 1; black line) and  
 361 seven stationary ADCSWAN runs (simplification level 3; black dots) during storm 2 (Table 1).  
 362 NTRs are computed for both cases by subtracting a ‘tides only’ simulation from the fully forced  
 363 model. Figure 3b demonstrates how the stationary NTR is computed for the peak of storm 2.  
 364 This NTR value is plotted in Figure 3a as a red outlined dot. The agreement between the fully  
 365 dynamic run and the seven stationary runs was found to be sufficient, with an RMSE error for  
 366 storm 2 of 11 cm.

### 367 3.3 Experimental Design

368 A conceptual overview of the process used for constructing an emulator, in the context of  
 369 this study, is provided in Figure 4.

370



371

372 Figure 4: Conceptual framework for developing an estuarine hydrodynamic emulator.

373

374 The first step in building an emulator is the selection of design points (experimental  
 375 design) to create the training dataset. This study implements a design from the commonly  
 376 utilized Latin Hypercube sampling (LHS) family of schemes first explored by McKay et al.  
 377 [1979]. LHS is one of the oldest and most popular experimental designs and has been found to  
 378 perform well for complex simulators [Jones and Johnson, 2009]. The specific experimental  
 379 design for this study was created using a “maximin” LHS design [Johnson, 1990; Morris and  
 380 Mitchell, 1995] from the LHS package in R [Carnell, 2017].

381 Parameters required for a LHS design are the number of dimensions to be included, the  
 382 range of each dimension, and the number of design points. As detailed in section 3.2, this study  
 383 used an input parameter dimensionality of 16, including wind speed and direction, sea surface



384 pressure, Hs, Tp, MWD, streamflow, base WL, and eight tidal equilibrium arguments. LHS  
385 considers only the maximum and minimum values of each dimension with design points spaced  
386 approximately uniformly across dimensions. Ranges were chosen for each parameter in an  
387 attempt to span all plausible forcing scenarios. This was determined by looking at 100-year  
388 return level events as calculated from the observational records. The size of the training dataset is  
389 typically controlled by the cost of running the simulator, but Loeppky et al. [2009] provide the  
390 general guidance that the training dataset should be approximately 10 times the number of  
391 dimensions of the input space. Given the 16 input dimensions of this study, this suggests a  
392 theoretical training dataset size of 160 runs. To explore the relationship between training dataset  
393 size and emulator skill and to validate the emulator's overall effectiveness, this study  
394 conservatively developed a larger training dataset consisting of 480 ADCSWAN runs.

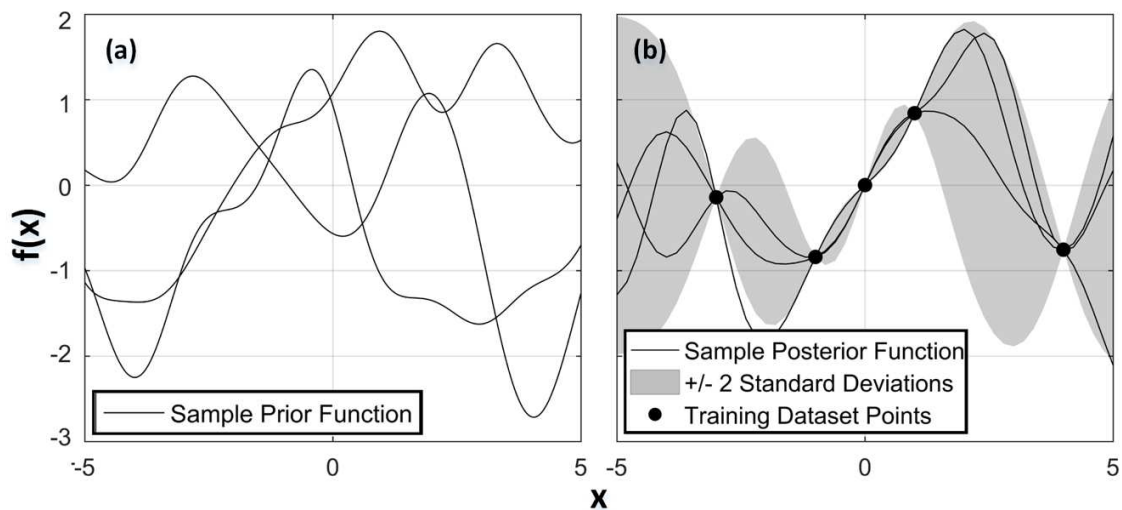
### 395 3.4 Emulator Configuration

396 A variety of formulations have previously been used in an emulation context, including  
397 support vector machines, artificial neural networks, radial basis functions, and many others [Jin  
398 et al., 2001; Gano et al., 2006; Razavi et al., 2012]. This study uses GPR, (also referred to as  
399 Kriging), a Bayesian statistical non-parametric regression model well suited to this particular  
400 application as it scales well to high-dimensional input and intrinsically considers model  
401 uncertainty [O'Hagan, 2006; Levy and Steinberg, 2010]. Furthermore, GPR is a general and  
402 flexible framework that can be optimized for a variety of modeling problems [Rasmussen and  
403 Williams, 2006]. For example, many other common emulator formulations, such as neural  
404 networks [Rasmussen and Williams, 2006] and radial basis functions [Anjyo and Lewis, 2011],  
405 can be shown to be equivalent to GPR under specific conditions.

406 The foundational definition of a Gaussian process is that of an infinite collection of  
407 variables for which any finite subset is described by a multivariate Gaussian distribution. Every  
408 point in the input space can be modeled as a random variable (due to uncertainty about the  
409 functional response to inputs). A Gaussian process governs how these variables are related. A  
410 common way of thinking about GPR is as a distribution over functions [Rasmussen and  
411 Williams, 2006]. This is mathematically tractable as a GPR can be completely defined by a mean  
412 and covariance function (due to being modeled as a multivariate Gaussian distribution). From a  
413 Bayesian perspective, this means a GPR is specified using a prior mean and covariance function.

414 The data then updates this prior, using Bayesian inference, with information about the true form  
415 of the function to develop the posterior. The mean posterior function is then the most probable  
416 function (considering all possible functions) given the data that has been observed.

417 This process is conceptualized for a one-dimensional case in Figure 5. The effect of the  
418 Bayesian conditioning on the emulator can be seen as “anchoring” the posterior sample functions  
419 (and uncertainty) at locations of observations. This limits the possible functions to those that go  
420 through these observed points. Uncertainty is quantified by considering the possible functions  
421 that pass through these training points.



422

423 Figure 5: Example 1-D application of GPR for determining  $f(x)$  from observations. Panel (a) shows three  
424 random sample functions drawn from the prior distribution. A non-informative prior is specified so the  
425 average over functions has a zero mean. Panel (b) shows 3 random sample functions drawn from the  
426 posterior distribution after 4 training observations (dark black points). The effect of training is to  
427 constrain possible functions to only those that go through observation points. In panel (b), the shaded  
428 region represents plus and minus 2 standard deviations from the mean posterior prediction. Figure after  
429 Rasmussen and Williams [2006].

430

431 The first component of a Gaussian process is the mean function, which defines the mean  
432 of the infinite set of functions that are being considered. A common choice is to set the prior  
433 mean to zero, which can be thought of as a non-informative prior where the form of the function  
434 between inputs and outputs is unknown. This is demonstrated in Figure 5, as shown by the  
435 approximate mean of the sample prior functions being zero. As an alternative, this study follows  
436 the methodology of Timmermans [2015] who used a simple linear regression to obtain

437 information about the mean function's form. Residual analysis of our data showed a cubic  
438 relationship for the tidal equilibrium argument terms, a somewhat expected result due to the  
439 cyclic nature of tides. Based on this result, the prior mean function was defined with a cubic term  
440 for all tidal equilibrium argument inputs and a linear term for all other inputs. A k-fold cross-  
441 validation (see section 4.2) was performed to evaluate emulator skill with and without the  
442 modified mean function. Results showed a significant gain in skill (both in terms of RMSE and  
443 Determination,  $R^2$ ) by using the modified mean function.

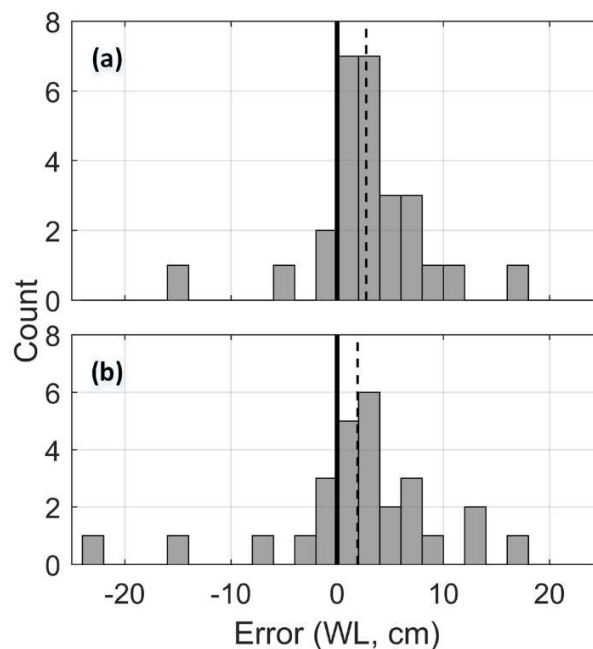
444 The covariance function of a Gaussian process is the second necessary component for  
445 defining the emulator. The covariance function (often called the kernel) can be thought of as  
446 describing the relationship between points in the process. Practically this describes the  
447 smoothness of the resulting GPR. In general, the covariance function contains hyper-parameters  
448 describing the details of the relationship between points (e.g., parameters such as length-scale,  
449 signal variance, etc.). These parameters can be inferred from the data, which is commonly done  
450 through maximizing the marginal likelihood rather than full Bayesian inference [Schulz et al.,  
451 2018]. This was the approach used for this study.

452 A comparison of model performance was performed using 3 commonly used covariance  
453 functions: the Gaussian, squared exponential, and Matern [Rasmussen and Williams, 2006]. The  
454 Matern covariance function was tested with  $\nu$  (a parameter controlling smoothing) equal to 1.5  
455 and 2.5. The best performing model was evaluated using k-fold cross-validation and comparing  
456 model RMSE values [Kohavi, 1995; Arlot and Celisse, 2010]. K-fold cross-validation breaks the  
457 total training dataset into k segments and cycles through every possible combination of  
458 withholding one segment for validation and training the emulator with the remaining segments.  
459 This results in an ensemble of skill metrics for which the mean is less biased and more robust to  
460 the training period than a standard validation methodology [Arlot and Celisse, 2010]. It was  
461 found that the Matern ( $\nu = 2.5$ ) performed the best and therefore was utilized for all results found  
462 in the following section. The training of the emulator was performed using the Managing  
463 Uncertainty in Complex Models (MUCM) package in R [Malde et al., 2016a].

464 **4. Results**

465 **4.1 Error Introduced by Model Simplifications**

466 With the construction of the emulator being hierarchical (Figure 2), it becomes important  
467 to assess the skill of the emulator at multiple simplification levels to determine where errors are  
468 being introduced. This was investigated for simulator simplifications by looking at the 5 largest  
469 storms, in terms of NTR, on record (Table 1). Storm events were chosen for this analysis as it is  
470 expected that the strong forcing gradients and rapidly changing dynamics of these events would  
471 provide the most robust test of simulator simplifications. For each storm, WLs were calculated  
472 using the Full Simulator (dynamic, non-simplified) and the Stationary Simulator (all  
473 simplifications except for emulation) to quantify the sum of level 2 and level 3 errors. This  
474 comparison was performed at six or seven (seven except for storm 2) temporally random points  
475 distributed across each storm. Figure 6 shows the difference between calculated WLs (level 1  
476 simplification minus level 3 simplification) at two locations: the tide gauge (Figure 6a) and WL  
477 station 7 (Figure 6b). This difference is denoted here as the “error” resulting from simplifying the  
478 simulator. Two locations are plotted to visually sample how error is affected by location within  
479 the estuary.



480

481 Figure 6: Error between Full and Stationary (Level 1 and Level 3) Simulator calculated WLs. The  
482 histogram includes comparisons for storms 1-5 (Table 1). The bold line represents zero error while the

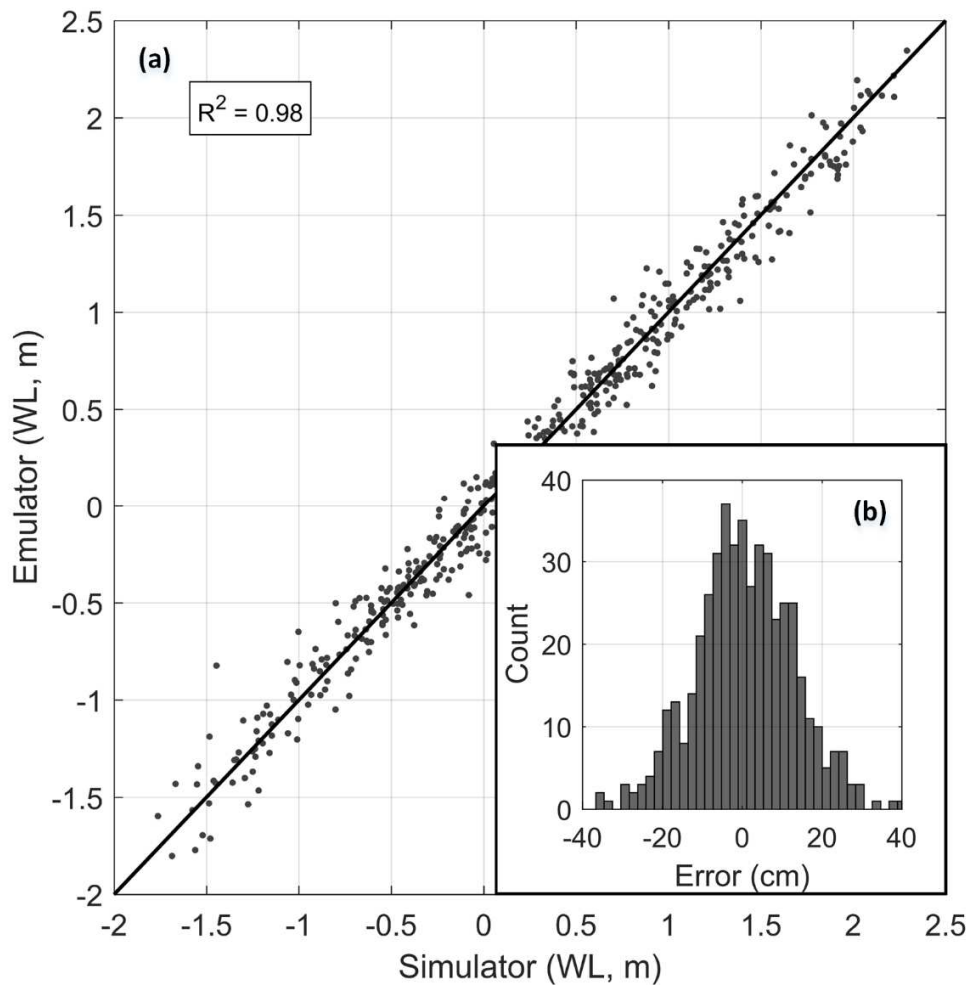
483 dotted line is the mean error of the ensemble. Subplot (a) is computed at the tide gauge location while  
484 subplot (b) is at WL station 7 (located deeper within in the estuary; Figure 1).

485

486 The error computed via this test was found to have a max of 25 cm with a RMSE of 6 cm  
487 at the gauge location. The maximum RMSE for approximately 100 test stations randomly  
488 scattered across the estuary domain was found to be 9 cm. The Simplified Simulator was found  
489 to be only slightly biased with a mean approximately 2 cm lower than the Full Simulator  
490 (represented as a positive mean error in Figure 6). Additional snapshot runs were performed to  
491 examine the model's error for non-storm conditions (not shown). Results confirmed that the  
492 Simplified Simulator, on average, performs better for non-storm conditions, suggesting that the  
493 results in Figure 6 are likely conservative.

#### 494 4.2 Emulator Validation

495 The ability of the emulator to replicate the Stationary Simulator (level 4 error) was  
496 quantified using a k-fold cross-validation. Figure 7 shows the results of the cross-validation with  
497 5 segments comparing emulated WLs and simplified simulator WLs.



498

499 Figure 7: Panel (a): Simplified Simulator vs. emulator WLs for the full training dataset. The comparison  
 500 was performed using a 5-fold validation procedure. Panel (b): histogram of the error between Simplified  
 501 Simulator and emulator WLs.

502

503 Overall the emulator was found to perform well at this comparison level with a high level  
 504 of skill. The emulator shows little bias (mean of the residuals is less than 1 cm), and relatively  
 505 even variance in residuals across WL magnitude (Figure 7a). However, the width of the  
 506 histogram in Figure 7b suggests that this step introduces more error than simulator  
 507 simplifications. The RMSE was found to be around 13 cm (level 4 error) which is significantly  
 508 larger than the calculated simulator simplification error (Figure 6, sum of level 2 and level 3  
 509 error) of approximately 6 cm. However, this comparison of RMSEs is imperfect as the level 4

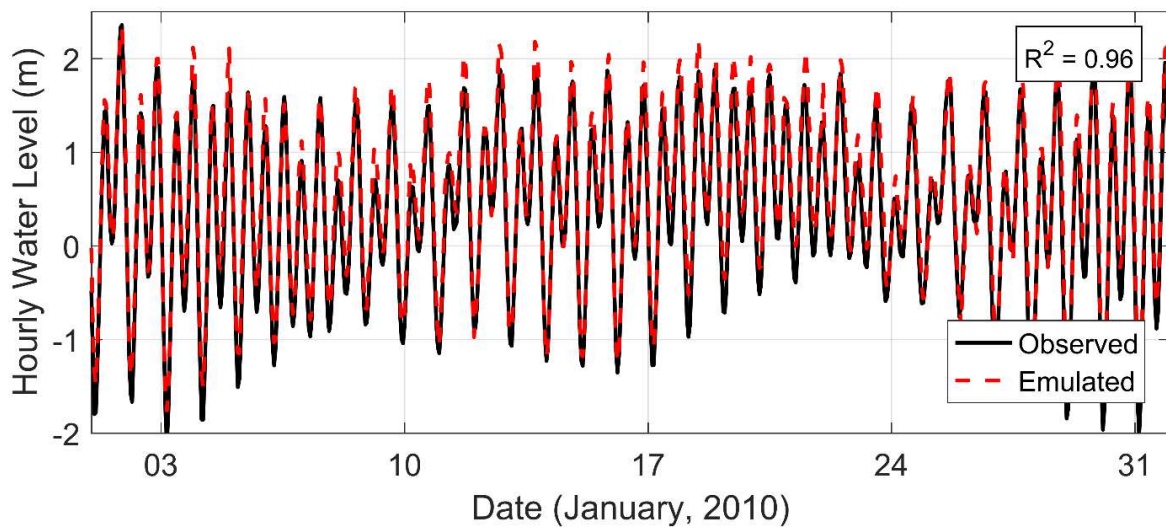
510 error assessment is based on a larger sample size and more rigorous k-fold validation and the  
511 level 2 and level 3 error assessment only examined performance during storm events.

### 512 4.3 Emulator Performance: Westport, WA Tide Gauge

513 The next stage of quantifying the skill of the emulator is to compare emulated WLs to  
514 observations at the tide gauge. This test provides a measure of the cumulative level 4 error, or the  
515 total integrated error from predicting observed WLs using emulation. This analysis was  
516 performed by using the emulator to hindcast hourly WLs at the location of the tide gauge for the  
517 entire period of record (2006-2016). Comparison between tide gauge observations and hourly  
518 emulated WLs for a randomly chosen month long segment are shown in Figure 8. Overall,  
519 hourly emulated WLs (for the over 10-year long record) compare favorably to the tide gauge  
520 with an  $R^2$  value of greater than 0.96, RMSE of approximately 15 cm, and a bias of less than 1  
521 cm.

522

523



524

525 Figure 8: Comparison of emulated (red dashed line) and observed (black line) hourly WLs at the Grays  
526 Harbor tide gauge for January 2010. Coefficient of determination is calculated using the entire tidal  
527 record (2006 - 2016). WLs are plotted in reference to mean sea level.

528

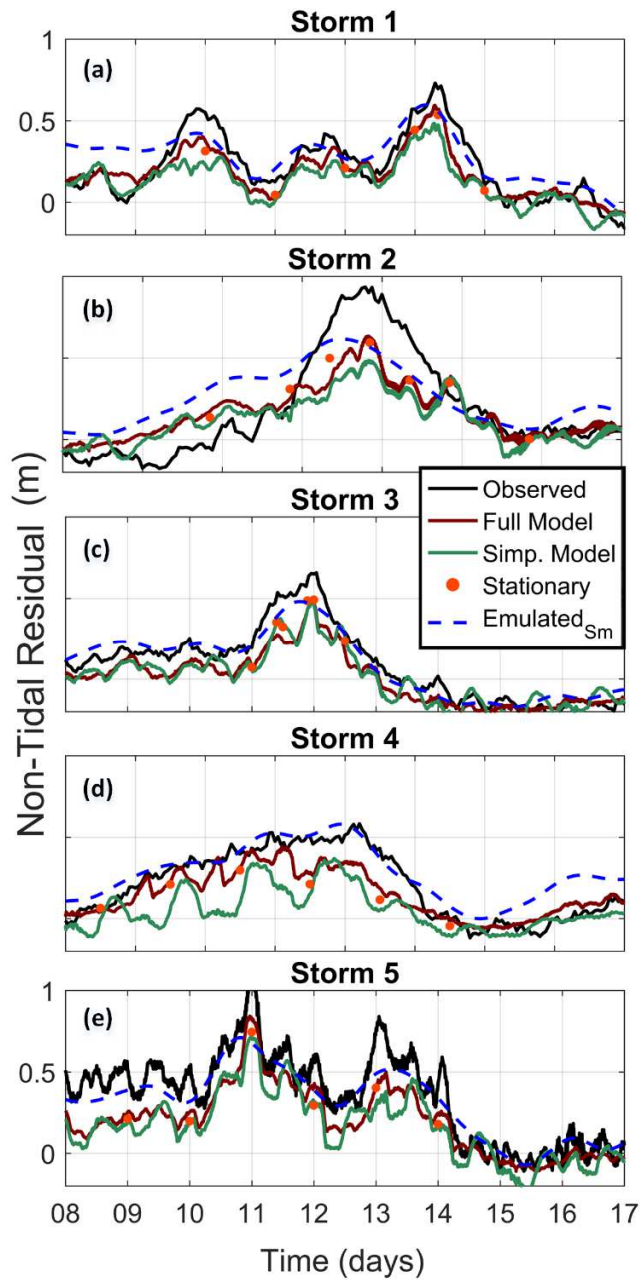
529 As with tide gauge records, WL output from an emulator can be considered as the sum  
530 between two components: tides and NTRs. In the PNW, tides are the dominant source of WL  
531 variability [Allan and Komar, 2002a] and so the skill of the emulator in predicting WLs is  
532 primarily controlled by its ability to reproduce the deterministic tides. However, coastal hazard  
533 research often considers NTR individually as the driver of extreme WLs on top of regular, and  
534 well predicted, tidal cycles. Therefore, it is additionally important to test the emulator's skill at  
535 reproducing NTR signals. Furthermore, this provides a more robust test of emulator performance  
536 as NTR is not explicitly modeled as an output by the emulator.

537 NTRs are often calculated at tide gauges by subtracting the predicted tide (determined  
538 through harmonic analysis) from the measured WL. This procedure can be problematic since  
539 NTR may affect tidal phase, resulting in a false NTR signal from out of phase tidal signals  
540 [Pugh, 1996; Haigh et al., 2014; Serafin and Ruggiero; 2014]. Therefore, for this study NTRs  
541 were calculated from tide gauge data using the procedure of Serafin and Ruggiero [2014]  
542 (modeled after Bromirski et al., 2003), which uses spectral filtering to remove energy from tidal  
543 bands.

544 The Bromirski et al. [2003] methodology is not used to determine NTRs from the  
545 ADCSWAN simulations. Storm simulations are on the scale of weeks which is too short  
546 temporally to recover energy across all tide bands of interest in the frequency domain.  
547 Instead, NTRs from the ADCSWAN simulations (at all simplification levels) and emulator  
548 simulations were calculated as a full forcing run minus a tidal only run. Emulated NTRs were  
549 then subsequently smoothed with a loess filter to reduce noise associated with tidal phase  
550 mismatches between the tide-only and full forcing emulated time series.

551 Comparison of observed and hindcasted NTRs for storms 1-5 (Figure 9) show a good  
552 overall performance of the emulator. To contextualize this comparison, the Full, Simplified, and  
553 Stationary Simulator calculated NTR time series are all plotted. A quantitative comparison of  
554 error between observed and modeled NTRs found that all simplification levels (from full  
555 ADCSWAN to emulator) have an RMSE of approximately 14 cm plus or minus 1 cm. The  
556 similar error across all simplification levels suggest that the largest source of error for NTR is in  
557 Full Simulator itself (level 1). For example, in Figure 9b it is clear that the Full Simulator (red  
558 line) is unable to reproduce the peak NTR signal in storm 2.





559

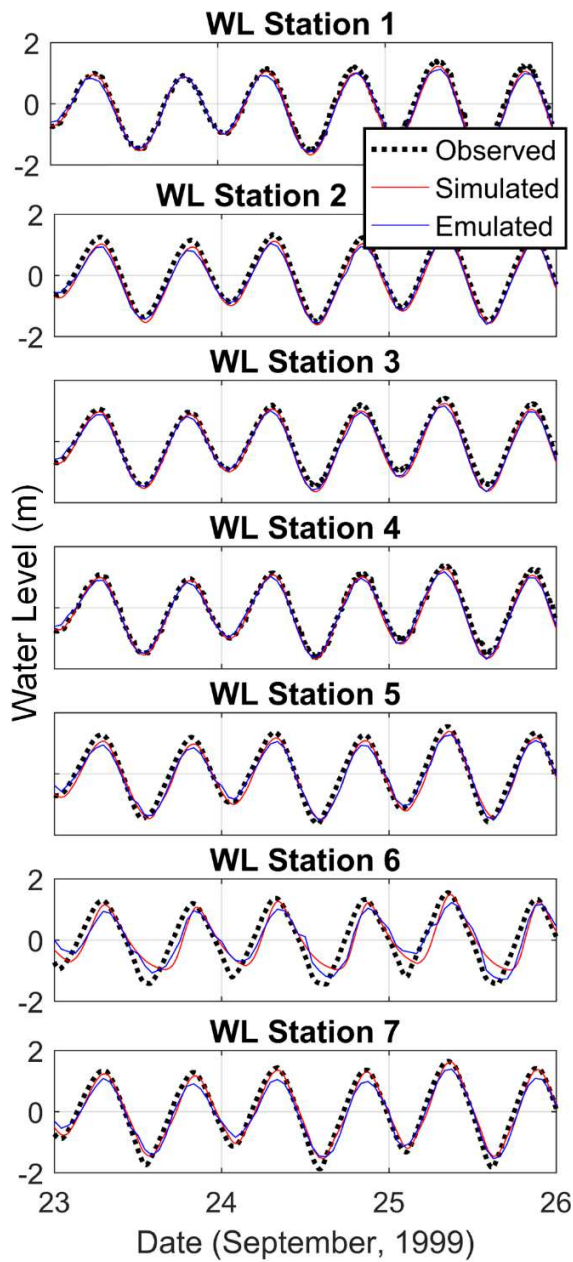
560 Figure 9: Tide Gauge comparison of observed and modeled NTR at various simplification levels. Each  
 561 subpanel (a through e) is one of the top 5 storms of record (see Table 1). Due to windowing for spectral  
 562 filtering, storm 5's observed NTR is calculated using the subtraction method rather than the Bromirski  
 563 method. All panels have the same y-axis scaling. The specific dates on the x axes vary by storm, but each  
 564 tick represents a day.

565

#### 566 4.4 Emulator Performance: USACE Field Campaign

567           The tide gauge provides a rich dataset for validating the emulator due to its record length  
568 but is spatially limited to a single comparison point within the study area. One key strength of  
569 emulation, in comparison to a fully data driven methodology, is the ability to provide WL  
570 information across study sites where observational information may not be available. An  
571 emulator can be constructed at any location within the ADCSWAN model domain where output  
572 is provided. Figure 10 evaluates the spatial performance of emulation through a comparison of  
573 emulated and observed WL time series for a 1999 field campaign led by the USACE in Grays  
574 Harbor [Figure 1, Cialone et al., 2001; 2002].

575           Figure 10 shows good performance between observed and modeled WLs across most  
576 locations. The main exception to this is WL station 6 which displays comparatively poor  
577 agreement between the observed and emulated WLs. This lack of skill is equally shown by the  
578 Full Simulator and is therefore not a result of the emulation procedure. Table 2 gives RMSE  
579 values for a comparison between observations and modeled output at various levels of  
580 simplification (level 1 simulations and level 4 emulations are shown in Figure 10). The levels  
581 described in the column headers are cumulative error, or a comparison of the model at that  
582 simplification level (Figure 2) to observations. No level 3 skill estimate was developed due to the  
583 computational constraints of simulating sufficient stationary point runs to accurately quantify  
584 model skill.



585

586 Figure 10: Comparison of observations (USACE deployments, Figure 1), Full Simulator and emulated  
 587 WLs. All panels have the same y-axis and x-axis scaling.

588

589 Table 2 indicates that the largest drop in skill is at the level 1 simulator simplification.  
 590 This corresponds to the full ADCSWAN model's inability to perfectly reproduce observations.  
 591 Level 2 simplifications are found to only nominally impact modeled WLs with a small (1 cm)

592 increase in RMSE for some stations. Level 4 simplifications additionally produce very little loss  
 593 of skill.

594 Table 2: RMSE values comparing WL model output to observations at various simplification levels.  
 595 Rows are locations / variables while the two column groupings represent instrument deployments. The  
 596 length of the time series comparison varies depending on station deployment length.

Station	Deployment 1: RMSE (cm)			Deployment 2: RMSE (cm)		
	Level 1	Level 2	Level 4	Level 1	Level 2	Level 4
WL 1	24	24	25	21	21	22
WL 2	19	20	22	16	17	19
WL 3	26	26	27	37	37	29
WL 4	19	19	21	22	23	20
WL 5	21	22	22	19	19	20
WL 6	44	44	41	36	36	39
WL 7	21	22	28	23	21	27

597

## 598 **5. Discussion**

### 599 **5.1 Effect of Simulator Simplifications**

600 The hierarchical validation used in this study provides a unique approach to quantifying  
 601 the error budget as sourced from multiple simplification levels. A comparison of model  
 602 performance at various simplification levels (Table 2) found that the primary source of lost skill  
 603 is from the Full Simulator rather than from simulator simplifications or from emulation.  
 604 Averaging across stations and deployment periods, all simplifications and emulation only  
 605 increased RMSE by 1 cm relative to the level 1 error.

606 This result is of particular interest when compared to the quantification of discrete error  
 607 from only emulation (see the histogram in Figure 7) which shows that emulation introduces  
 608 comparatively significant error into WL estimates. A cumulative level 4 comparison at the tide  
 609 gauge (Figure 8) found a RMSE of 15 cm while Level 4 itself (Figure 7) had a RMSE of 13 cm.  
 610 This result suggests that the error from each simplification during emulator construction is not  
 611 independent. In other words, the cumulative error variance is not the sum of the discrete error  
 612 variances. Practically, the dominance of level 1 error is found to mask that of the other levels.

613 This may not be true if level 1 error was able to be significantly reduced by improvements in  
614 process-based modeling at which point other simplifications may become relevant to the error  
615 budget.

616 In terms of quantifying model skill, the RMSE for comparisons to the 1999 USACE field  
617 data is overall larger than the RMSE in comparison to the tide gauge. A close examination of the  
618 USACE WL time series shows significant high frequency noise that is likely the cause of the  
619 overall larger RMSE values. The USACE data exhibits more high frequency variability due to a  
620 shorter averaging period (6 minutes for the tide gauge and 3 minutes for the USACE data).

621 This study suggests that the most effective action to improve emulated WL predictions is  
622 to reduce level 1 error. One option would be optimized tuning, a process which can be  
623 accomplished by including tuning parameters within the emulator framework [Kennedy et al.,  
624 2006; Hall et al., 2011;]. ADCSWAN could also be replaced with a different simulator or  
625 physics implementation, for example ADCSWAN in 3D baroclinic mode. This would come at  
626 the cost of drastically increasing computation time and requiring additional input dimensionality  
627 in the form of density, temperature, and salinity fields.

628 A level 1 error reduction could also be accomplished through improving the quality of  
629 model input data, both in terms of bathymetry and forcing. Incorrect bathymetry is likely the  
630 source of errors for WL station 6. Figure 10 shows WLs at this station having an asymmetric  
631 tidal signal indicative of shallow water while the observations have less asymmetry. This  
632 suggests that the water depth at the time of deployment was greater than the depth in the  
633 compiled bathymetric dataset used to generate the ADCSWAN grid. Therefore, investment in  
634 more accurate or more recent bathymetry is another viable step for decreasing level 1 error.  
635 Similarly, level 1 error integrates error as a function of poor-quality forcing, making  
636 improvements to forcing another promising avenue for error reduction.

637 Level 2 error could be reduced by making less aggressive simplifications of forcing  
638 inputs. It is conceptually straight forward to include other input dimensionality such as spatially  
639 variable atmospheric forcing or full spectral wave forcing. A promising strategy for including  
640 field variables as input dimensions is through decomposing the field into principal components  
641 [Higdon et al., 2008; Liu and Guillas, 2017].

642           There are additionally a range of options for avoiding the stationarity assumption made in  
643 this study, which would eliminate or reduce level 3 error. The incorporation of temporal  
644 variability in emulators is reviewed by Reichert et al. [2011] who suggest the following  
645 strategies:

- 646       1) Apply a standard emulation methodology but with time as an additional degree of  
647       dimensionality. [Conti et al., 2005]
- 648       2) Describe the time series using basis functions and then apply emulation to the basis  
649       function coefficients. [Bayarri et al., 2007; Higdon et al., 2008]
- 650       3) Emulate the difference from one time point to the next. [Bhattacharya, 2007; Conti et al.,  
651       2009]
- 652       4) Use a Gaussian stochastic process as a Bayesian prior. [Liu and West, 2009]
- 653       5) Develop a hybrid dynamic/emulated model, or a “Mechanistic dynamic emulator.”  
654       [Reichert et al., 2011; Albert C., 2012]

655 In the context of this study, strategy 1 is conceptually the simplest but it is not clear a priori how  
656 far into the past the system’s memory extends and each included time step multiplies the  
657 dimensionality of the input space. Strategy 2 is complicated by identifying basis functions that  
658 adequately capture the various contributing signals. For example, a Fourier transformation is a  
659 natural solution except that storm surge is non-periodic and an important contributor to estuarine  
660 WLs. Strategies 3-5 all have potential advantages but bring additional complexity to an already  
661 complex methodology so were not explored further.

662           For reducing level 4 error, GPR is a flexible framework and there are likely gains to be  
663 made through a more exhaustive approach for emulator specification. In particular, handling of  
664 the periodic nature of tides within the covariance function [Roberts et al., 2013] is a promising  
665 research direction.

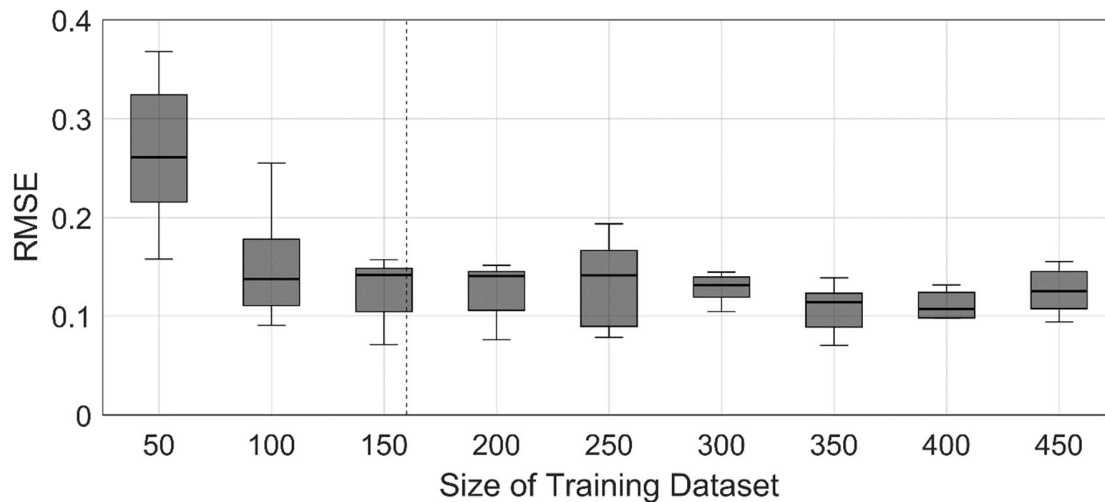
## 666 5.2 Computational Cost Considerations

667           Emulation is an approach to dramatically reduce simulation times and is therefore most  
668 valuable in situations where the simulator must be run for very long periods or for multiple  
669 iterations (e.g., probabilistic risk assessment). Emulation requires an upfront cost, through the  
670 running of multiple simulations to construct a training dataset, but is comparatively  
671 instantaneous after this initial investment. As the nature of the trade-off is computation time, it is  
672 useful to review the costs of building the training dataset.

673           The first control on computational cost for the training set is the number of input  
674 dimensions. The simplifications implemented in this study managed to reduce the input space to  
675 16 dimensions. Each design point took approximately 66 core hours to run in parallel on a server  
676 with Intel Xeon E52450 CPUs (2.1 GHz). With this setup, a full experimental design of 160  
677 points would require over 10.5 thousand core hours (although with parallelization the actual time  
678 was much less). This study developed a larger experimental design (over 400 points) but this was  
679 primarily for validation rather than emulator skill (see discussion below). Full ADCSWAN  
680 required approximately 18 core hours per day of simulation time. Based off these computational  
681 costs, emulation becomes an efficient option if approximately one and a half years of simulation  
682 are required. This limit is highly situationally dependent and is controlled by processor speed,  
683 simulator, emulator, etc. and is intended only as an order of magnitude reference. Furthermore,  
684 emulation is primarily targeted at probabilistic methodologies, rather than hindcasting, for which  
685 multiple iterations of time series quickly sum to very large total simulation times.

686           The above analysis is based on a LHS design and the Loeppky et al. [2009] guideline that  
687 a training dataset should be around 10 times the number of input dimensions. However, LHS is  
688 one of many possible experimental designs [Levy and Steinberg, 2010]. Significant research has  
689 focused on optimizing experimental designs beyond LHS and it is possible that a more complex  
690 design could reduce the size of the training dataset. For example, LHS does not consider the  
691 probability that a particular combination of input parameters may occur. Therefore, some design  
692 points are likely poorly utilized exploring space that is physically impossible or highly  
693 improbable (for example, high wave heights associated with low wave periods).

694           Finally, the above analysis did not consider the effect of training dataset size on skill.  
695 This relationship was tested by quantifying emulator performance at a variety of training dataset  
696 sizes. For this analysis, the total body of simulations (480) was partitioned into smaller dataset  
697 sizes ranging from 50 to 450 simulations for testing. For each smaller dataset, a k-fold validation  
698 with 5 segments was performed (Figure 11) to quantify emulator skill at this smaller training  
699 dataset size. This analysis is identical to that described in section 4.2 but with an artificially  
700 decreased training dataset size.



701

702 Figure 11: Impact of training dataset size on emulator skill. RMSE is calculated with a 5-fold cross-  
 703 validation and represented by a standard boxplot. The dotted vertical line represents the theoretical  
 704 training dataset size from Loeppky et al. [2009].

705

706 Results from this analysis are in good agreement with the guidance of Loeppky et al.  
 707 [2009] in that ten times the number of input dimensions is sufficient for building a skillful  
 708 emulator (Figure 11). Beyond this limit, only very small gains in skill are realized, suggesting  
 709 that it is not efficient to over build the training dataset.

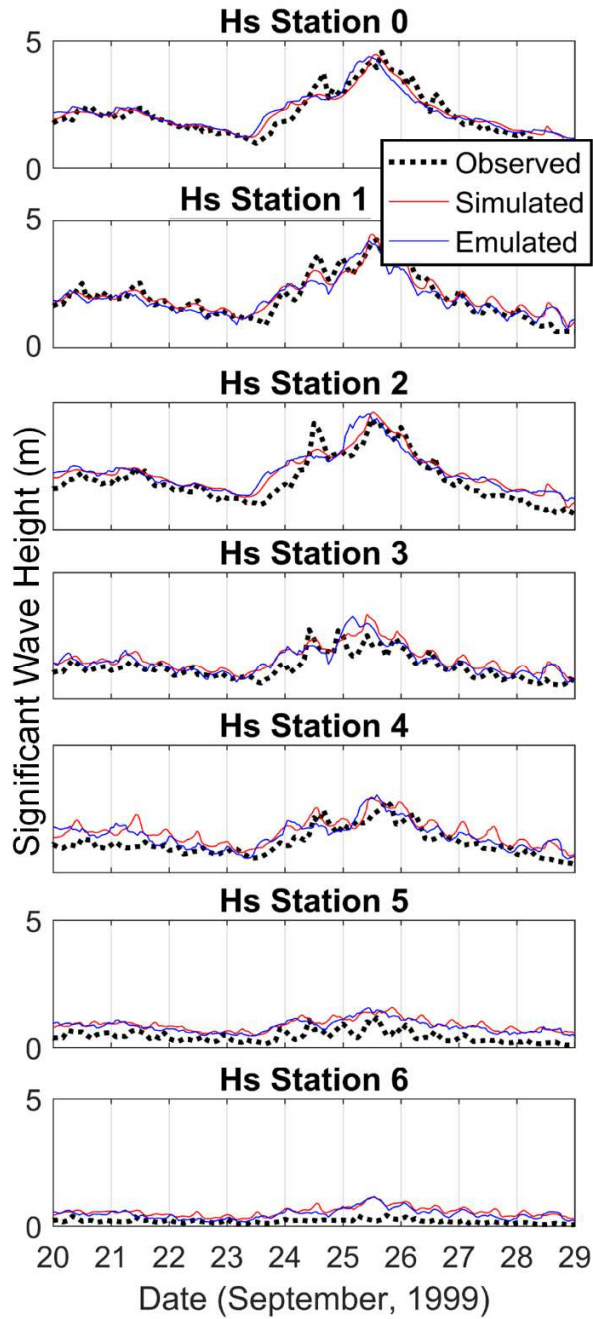
710 It is worth considering the cumulative computational cost of developing multiple  
 711 emulators. This study takes the approach of building individual emulators at each location of  
 712 interest. While emulator training and simulation is rapid for individual emulators, the sum  
 713 computational cost of constructing many emulators can be significant. This is especially true  
 714 considering that large estuarine hydrodynamic models can be of very high output dimensionality  
 715 (the utilized Grays Harbor ADCSWAN grid has over 29,000 nodes). A common solution is to  
 716 dimensionally reduce model outputs via approaches such as principal component analysis [Chen  
 717 et al., 2011; Jia and Taflanidis, 2013; Jia et al., 2016; Bass and Bedient, 2017]. An alternative  
 718 option uses the multivariate Gaussian process to generalize the standard GPR case to a “multi-  
 719 output emulator” [Conti and O’Hagan, 2010; Fricker et al., 2010]. While not considered in this  
 720 study, which is primarily concerned with point assessments, these approaches could result in  
 721 significant computational savings for a larger output dimensionality. Further, considering



722 emulators individually implicitly assumes independence of output variables and ignores the  
723 inherent correlation between output variables [Rasmussen and Williams, 2006].

### 724 5.3 Emulation Beyond Water Levels

725         While this study has focused primarily on emulating WLS, emulation can easily be  
726 extended to other variables in a coastal hazards framework. To explore this possibility, Hs was  
727 emulated at the observational Hs stations from the 1999 USACE field campaign (Figure 1)  
728 [Cialone et al., 2001; 2002]. Hs emulators were developed using an identical approach to that of  
729 WLS except that Hs emulation was found to not need cubic terms for the prior mean function. A  
730 comparison to observations (Figure 12) shows that GPR emulators perform well for Hs with the  
731 highest Hs (around September 26, 1999) being well reproduced by the emulator at stations 0, 1,  
732 2, 3, and 4 (Figure 12). Performance is comparatively poor at stations 5 and 6, which are further  
733 within the estuary and less influenced by offshore waves. These results are further quantified in  
734 Table 3 which shows poor skill for interior Hs stations. It should be noted, however, that the Hs  
735 signals at these two stations have low variance and are barely above the noise floor.



736

737 Figure 12: Comparison of observations (USACE deployments), Full Simulator and emulated Hs time  
 738 series. Symbols are used for the observations for the sake of visual clarity. All panels have the same y-  
 739 axis scaling.

740

741 Results show that, similarly to WLS, the largest loss of skill is at simplification level 1.

742 Simplified Simulator and emulator results are found to closely track the Full Simulator. This is

743 most evident at the bay interior stations where the Full Simulator and emulator are both found to  
 744 over-predict Hs. Table 3 reveals that level 2, 3 and 4 simplifications produce little loss of skill  
 745 for calculated Hs (average increase in RMSE of 2 cm). An exception to this is Hs station 5 which  
 746 shows a significant increase in RMSE at the emulation level. The cause of poor emulator  
 747 performance at this one location is unclear but it is likely due to a poor emulator model fit.

748

749 Table 3: RMSE values comparing Hs model output to observations at various simplification levels. Rows  
 750 represent station locations while the two column groupings represent instrument deployments. Hs station  
 751 0/7 was relocated so deployment 1 values represent the Hs station 0 location and deployment 2 values  
 752 representing the station 7 location.

Station	Deployment 1: RMSE (cm)			Deployment 2: RMSE (cm)		
	Level 1	Level 2	Level 4	Level 1	Level 2	Level 4
Hs 0/7	32	32	37	73	73	75
Hs 1	38	39	37	61	61	69
Hs 2	45	46	51	48	49	61
Hs 3	45	45	35	52	52	50
Hs 4	49	50	40	148	148	42
Hs 5	47	47	104	136	136	209
Hs 6	35	35	28	40	40	38

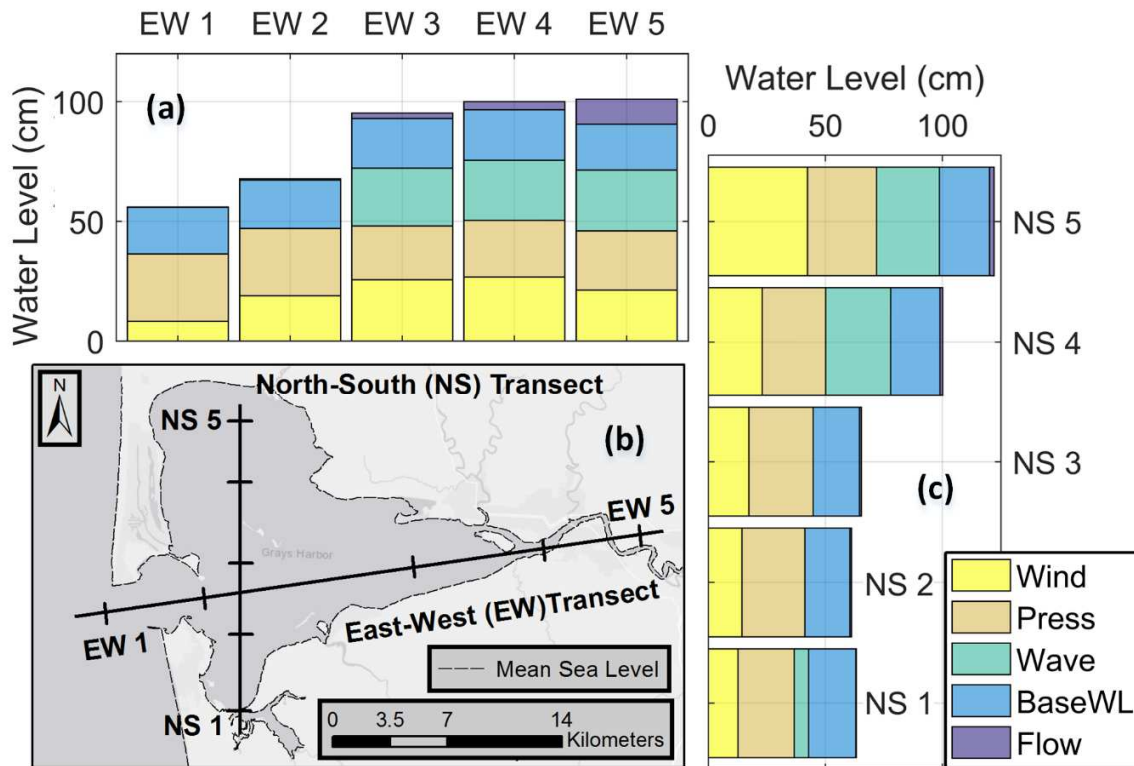
753

754 Overall these results suggest that emulation could be integrated into many parts of an  
 755 estuary modeling system. However, a key assumption of emulation with GPR is smoothness in  
 756 response characteristics, suggesting that GPR may be sub-optimal for “jumpy” variables. Not  
 757 shown are similar results for Tp which can exhibit discontinuities within estuaries as Tp switches  
 758 from one wave spectrum component to another. The emulator is qualitatively able to capture Tp  
 759 characteristics but cannot resolve these instantaneous jumps. For this reason, it is important to  
 760 carefully consider the form of the output variable being emulated and its relation to the input  
 761 parameters.

#### 762 5.4 Example Emulator Application: Extreme Water Level Decomposition

763           Outside of validation, it is illustrative to explore an example application of emulation. For  
764 this purpose, a decomposition of the relative forcing contributions responsible for extreme WLs  
765 within Grays Harbor was performed. Seven versions of a 31-year time series (1984-2015) were  
766 emulated under different forcing scenarios. As a baseline, a “full forcing” case time series was  
767 emulated with the observed forcing at Grays Harbor (comparable to the hindcasts in Figure 8 and  
768 Figure 10). Each additional forcing scenario was emulated with one forcing contribution  
769 excluded (waves, wind, pressure, base WL, streamflow, and tides) to isolate the relative  
770 contribution of individual forcings to WLs. A particular forcing contribution was calculated as  
771 the emulator output WL with full forcing minus the emulator output WL with all forcing except  
772 the component of interest. The exception to this is tides (which cannot be turned off due to how  
773 they are included in the emulator) which were calculated simply as emulator output with no other  
774 forcing but tides. WL contributions were calculated at the time of the 31 annual maximum WL  
775 events as determined by the full forcing time series. The average relative contribution of each  
776 forcing component over the 31 annual maxima are plotted along East-West and North-South  
777 transects in Figure 13.

778



779

780 Figure 13: Average WL contribution from forcing components during extreme events (maximum annual  
 781 WLs). Two transects are plotted with subplot (b) showing plotted transects, (East-West, EW) and (North-  
 782 South, NS), with station locations marked as ticks. Tick locations are approximate (within 1 km) to  
 783 scattered station locations. Subplot (a) is the East-West transect and subplot (c) is the North-South  
 784 transect.

785 The diverse mix of contributions for each bar in Figure 13 shows that extreme WL events  
 786 are compound in nature. This conclusion is further supported by the variance in emulated  
 787 extreme WL contributions (not shown) which reveals that the composition of each individual  
 788 annual maximum event varies widely across the timeseries. The mean contribution of each  
 789 forcing is found to be significant providing evidence that all included forcing processes are  
 790 important for properly quantifying extreme water levels. The only exception is streamflow which  
 791 is found to be nominally important except near the streamflow boundary. This result is likely  
 792 specific to the Grays Harbor estuary and would be different for a more hydrologically dominated  
 793 estuary system [Svensson and Jones, 2004; Lavery and Donovan, 2005; Chen et al., 2014].

794 The mix of contributions is found to be spatially variable across the estuary domain,  
 795 leading to both an East-West and North-South gradient in contributions to WLs. For example,  
 796 the streamflow contribution is found to increase moving west towards the estuary's streamflow

797 inlet. Wave influence is found to have a significant contribution to the annual maxima but only at  
798 stations in the northern and eastern reaches of the bay. This result is likely due to breaking  
799 induced setup not occurring at the bay's entrance channel. The influence of wind increases to the  
800 north, due to the mean wind direction emanating from the south during storm events. The  
801 influence of pressure anomalies on extreme WLs is found to be uniform but this result is likely  
802 from the spatial simplification of sea level pressure fields.

803 Not shown in Figure 13 is the contribution from tidal forcing. This is primarily for scale  
804 reasons as the tidal component is an order of magnitude larger than that from other forcing  
805 (average of 140 cm). Tides also show a gradient across the estuary although with the opposite  
806 pattern as that shown in Figure 13. The tidal component of annual maxima WLs decreases by  
807 about 30 cm moving from the center of the estuary moving North or East. As WLs are the sum of  
808 these two components (tides and forcing driven NTR), the calculated gradient in total WLs is  
809 less than that shown in Figure 13 (under 20 cm across the two transects).

## 810 **6. Conclusions**

811 This paper has presented an application of emulation, or surrogate modeling, to the  
812 problem of rapidly simulating hydrodynamic variables within the Grays Harbor, WA estuary.  
813 This methodology is targeted towards a variety of computationally constrained problems  
814 including probabilistic modeling, uncertainty quantification, model optimization, and non-  
815 parametric extreme event analysis. To facilitate efficiently achieving these goals, this study has  
816 focused on validating and quantifying the error induced by emulation. Additionally, a variety of  
817 simplifications to the simulator have been suggested for reducing input dimensionality, and  
818 therefore the size of the emulator training dataset.

819 The results from this study suggest that the Gaussian Process regression (GPR) derived  
820 emulator is skillful for calculating a variety of model output variables (WL, NTR, and Hs). A  
821 decadal-scale comparison of emulated WLs to tide gauge data showed the emulator having a  
822 RMSE of 15 cm. Emulator performance is evaluated at multiple observation points across the  
823 estuary domain providing confidence that emulation is skillful across spatial extents.  
824 Decomposing the error from different emulator construction simplification levels shows that the  
825 largest source of unexplained variance in emulator hindcasts is from ADCSWAN itself. Of  
826 particular interest, strong simulator simplifications (including that of stationarity) are a relatively

827 low contributor to losses in emulator performance (average increase in RMSE of 1 cm).  
828 Therefore, future efforts to improve emulator performance should focus on improving the Full  
829 Simulator before reducing simplifications or optimizing the emulator.

830 Emulation is additionally found to be very efficient after the construction of the training  
831 dataset. Using an LHS experimental design, analysis shows that the training dataset size  
832 guidance of 10 times the number of input dimensions [Loeppky et al., 2009] is optimal in the  
833 case examined here. Overall emulation is found to have the same order of magnitude skill as  
834 process-based models as well as showing significant gains in computational efficiency.  
835 Therefore, emulation is shown to be a viable path for exploring estuarine hydrodynamic  
836 modeling problems.

837 Finally, the emulator was applied to investigate the relative contributions of different  
838 forcing variables to annual maxima WLs and NTR at the study site. Results show a diverse mix  
839 of forcing contributing to annual extreme WLs, indicating the importance of considering  
840 compound events for flood hazard assessments in the PNW. All forcing components, along with  
841 WL itself, were found to exhibit significant spatial variability hinting at important information  
842 for flood vulnerability assessment. Tides were found to be the largest contributor to extreme  
843 WLs with other components being of the same order of magnitude. The exception to this is  
844 streamflow which was found to be, on average, a relatively minor contributor to extremes except  
845 near the river's mouth. Additionally, waves were found to only contribute to WLs at stations  
846 near the edge of the estuary domain, a result that is likely tied to wave penetration into the  
847 estuary. While only a single example application of emulation to estuary hydrodynamics  
848 questions was explored, results signal the significant potential of emulation to a broad range of  
849 applications.

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864

## 865 **References**

866 Albert, C., 2012. A mechanistic dynamic emulator. *Nonlinear Anal. Real World Appl.* 13, 2747–  
867 2754. <https://doi.org/10.1016/j.nonrwa.2012.04.003>

868 Allan, J.C., Komar, P.D., 2006. Climate Controls on US West Coast Erosion Processes. *J. Coast.*  
869 *Res.* 223, 511–529. <https://doi.org/10.2112/03-0108.1>

870 Allan, J.C., Komar, P.D., 2002. Extreme Storms on the Pacific Northwest Coast during the 1997–  
871 98 El Niño and 1998–99 La Niña. *J. Coast. Res.* <https://doi.org/10.2307/4299063>

872 Allan, J.C., Komar, P.D., 2002. Wave Climate Change and Coastal Erosion in the US Pacific  
873 Northwest, in: *Ocean Wave Measurement and Analysis*. American Society of Civil  
874 Engineers, Reston, VA, pp. 680–689. [https://doi.org/10.1061/40604\(273\)70](https://doi.org/10.1061/40604(273)70)

875 Allan, J.C., Komar, P.D., Ruggiero, P., 2011. Storm Surge Magnitudes and Frequency on the  
876 Central Oregon Coast, in: *Solutions to Coastal Disasters 2011*. American Society of Civil  
877 Engineers, Reston, VA, pp. 53–64. [https://doi.org/10.1061/41185\(417\)6](https://doi.org/10.1061/41185(417)6)

878 Anjyo, K., Lewis, J.P., 2011. RBF interpolation and Gaussian process regression through an  
879 RKHS formulation. *J. Math-for-Industry* 3, 63–71.

880 Apel, H., Merz, B., Thielen, A.H., 2008. Quantification of uncertainties in flood risk  
881 assessments. *Int. J. River Basin Manag.* 6, 149–162.  
882 <https://doi.org/10.1080/15715124.2008.9635344>

883 Arlot, S., Celisse, A., 2010. A survey of cross-validation procedures for model selection. *Stat.*  
884 *Surv.* 4, 40–79. <https://doi.org/10.1214/09-SS054>

885 Bass, B., Bedient, P., 2018. Surrogate modeling of joint flood risk across coastal watersheds. *J.*  
886 *Hydrol.* 558, 159–173. <https://doi.org/10.1016/j.jhydrol.2018.01.014>



- 887 Bayarri, M.J., Berger, J.O., Cafeo, J., Garcia-Donato, G., Liu, F., Palomo, J., Parthasarathy, R.J.,  
888 Paulo, R., Sacks, J., Walsh, D., 2007. Computer Model Validation with Functional Output.  
889 *Ann. Stat.* 35, 1874–1906. <https://doi.org/10.1214/009053607000000163>
- 890 Bhaskaran, P.K., Nayak, S., Bonthu, S.R., Murty, P.L.N., Sen, D., 2013. Performance and  
891 validation of a coupled parallel ADCIRC–SWAN model for THANE cyclone in the Bay of  
892 Bengal. *Environ. Fluid Mech.* 13, 601–623. <https://doi.org/10.1007/s10652-013-9284-5>
- 893 Bhattacharya, S., 2007. A simulation approach to Bayesian emulation of complex dynamic  
894 computer models. *Bayesian Anal.* 2, 783–815. <https://doi.org/10.1214/07-BA232>
- 895 Blain, C.A., Preller, R.H., Rivera, A.P., 2001. Tidal Prediction Using the Advanced Circulation  
896 Model (ADCIRC) and a Relocatable PC-based System. *Oceanography* 15.
- 897 Blain, C.A., Rogers, W.E., 1998. Coastal Tide Prediction Using the ADCIRC-2DDI  
898 Hydrodynamic Finite Element Model: Model Validation and Sensitivity Analyses in the  
899 Southern North Sea/English Channel.
- 900 Bode, L., Hardy, T.A., 1997. Progress and Recent Developments in Storm Surge Modeling. *J.*  
901 *Hydraul. Eng.* 123, 315–331. [https://doi.org/10.1061/\(ASCE\)0733-9429\(1997\)123:4\(315\)](https://doi.org/10.1061/(ASCE)0733-9429(1997)123:4(315))
- 902 Booij, N., Holthuijsen, L.H., Ris, R.C., 1997. The “SWAN” Wave Model for Shallow Water, in:  
903 *Coastal Engineering 1996*. American Society of Civil Engineers, New York, NY, pp. 668–  
904 676. <https://doi.org/10.1061/9780784402429.053>
- 905 Bromirski, P.D., Flick, R.E., Cayan, D.R., 2003. Storminess Variability along the California  
906 Coast: 1858–2000. *J. Clim.* 16, 982–993. [https://doi.org/10.1175/1520-0442\(2003\)016<0982:SVATCC>2.0.CO;2](https://doi.org/10.1175/1520-0442(2003)016<0982:SVATCC>2.0.CO;2)
- 908 Bunya, S., Dietrich, J.C., Westerink, J.J., Ebersole, B.A., Smith, J.M., Atkinson, J.H., Jensen, R.,  
909 Resio, D.T., Luettich, R.A., Dawson, C., Cardone, V.J., Cox, A.T., Powell, M.D.,  
910 Westerink, H.J., Roberts, H.J., 2010. A High-Resolution Coupled Riverine Flow, Tide,  
911 Wind, Wind Wave, and Storm Surge Model for Southern Louisiana and Mississippi. Part I:  
912 Model Development and Validation. *Mon. Weather Rev.* 138, 345–377.  
913 <https://doi.org/10.1175/2009MWR2906.1>
- 914 Carnell, R., 2017. LHS: Latin Hypercube Samples.
- 915 Castelletti, A., Galelli, S., Restelli, M., Soncini-Sessa, R., 2012. Data-driven dynamic emulation  
916 modelling for the optimal management of environmental systems. *Environ. Model. Softw.*  
917 34, 30–43. <https://doi.org/10.1016/j.envsoft.2011.09.003>
- 918 Chen, T., Hadinoto, K., Yan, W., Ma, Y., 2011. Efficient meta-modelling of complex process  
919 simulations with time–space-dependent outputs. *Comput. Chem. Eng.* 35, 502–509.  
920 <https://doi.org/10.1016/J.COMPHEMENG.2010.05.013>

- 921 Chen, W.-B., Liu, W.-C., Chen, W.-B., Liu, W.-C., 2014. Modeling Flood Inundation Induced  
922 by River Flow and Storm Surges over a River Basin. *Water* 6, 3182–3199.  
923 <https://doi.org/10.3390/w6103182>
- 924 Cheng, T.K., Hill, D.F., Beamer, J., García-Medina, G., 2015. Climate change impacts on wave  
925 and surge processes in a Pacific Northwest (USA) estuary. *J. Geophys. Res. Ocean.* 120,  
926 182–200. <https://doi.org/10.1002/2014JC010268>
- 927 Cheng, T.K., Hill, D.F., Read, W., 2015. The Contributions to Storm Tides in Pacific Northwest  
928 Estuaries: Tillamook Bay, Oregon, and the December 2007 Storm. *J. Coast. Res.* 313, 723–  
929 734. <https://doi.org/10.2112/JCOASTRES-D-14-00120.1>
- 930 Cialone, M.A., Kraus, N.C., 2001. Engineering Study of Inlet Entrance Hydrodynamics: Grays  
931 Harbor, Washington, USA, in: *Coastal Dynamics '01*. American Society of Civil Engineers,  
932 Reston, VA, pp. 413–422. [https://doi.org/10.1061/40566\(260\)42](https://doi.org/10.1061/40566(260)42)
- 933 Cialone, M.A., Militello, A., Brown, M.E., Kraus, N.C., 2002. Coupling of Wave and  
934 Circulation Numerical Models at Grays Harbor Entrance, Washington, USA, in:  
935 *Proceedings 28th Coastal Engineering Conference*. World Scientific Publishing Company,  
936 pp. 1279–1291. [https://doi.org/10.1142/9789812791306\\_0108](https://doi.org/10.1142/9789812791306_0108)
- 937 Cloke, H.L., Pappenberger, F., 2009. Ensemble flood forecasting: A review. *J. Hydrol.* 375, 613–  
938 626. <https://doi.org/10.1016/J.JHYDROL.2009.06.005>
- 939 Conti, S., Gosling, J.P., Oakley, J.E., O'Hagan, A., 2009. Gaussian process emulation of  
940 dynamic computer codes. *Biometrika* 96, 663–676. <https://doi.org/10.1093/biomet/asp028>
- 941 Conti, S., Anderson, C.W., Kennedy, M.C., O'Hagan, A., 2005. A Bayesian Analysis of  
942 Complex Dynamic Computer Models. *Sensit. Anal. Model Output*.
- 943 Conti, S., O'Hagan, A., 2010. Bayesian emulation of complex multi-output and dynamic  
944 computer models. *J. Stat. Plan. Inference* 140, 640–651.  
945 <https://doi.org/10.1016/j.jspi.2009.08.006>
- 946 Dale, M., Wicks, J., Mylne, K., Pappenberger, F., Laeger, S., Taylor, S., 2014. Probabilistic  
947 flood forecasting and decision-making: an innovative risk-based approach. *Nat. Hazards* 70,  
948 159–172. <https://doi.org/10.1007/s11069-012-0483-z>
- 949 Davis, J.R., Paramygin, V. a., Forrest, D., Sheng, Y.P., 2010. Toward the Probabilistic  
950 Simulation of Storm Surge and Inundation in a Limited-Resource Environment. *Mon.*  
951 *Weather Rev.* 138, 2953–2974. <https://doi.org/10.1175/2010MWR3136.1>
- 952 Dawson, R.J., Hall, J.W., Bates, P.D., Nicholls, R.J., 2005. Quantified Analysis of the  
953 Probability of Flooding in the Thames Estuary under Imaginable Worst-case Sea Level Rise  
954 Scenarios Quantified Analysis of the Probability of Flooding in the Thames Estuary under

- 955 Imaginable Worst-case Sea Level Rise Scenarios. *Int. J. Water Resour. Dev.* 21, 577–591.  
956 <https://doi.org/10.1080/07900620500258380>
- 957 Di Baldassarre, G., Schumann, G., Bates, P.D., Freer, J.E., Beven, K.J., 2010. Flood-plain  
958 mapping: a critical discussion of deterministic and probabilistic approaches. *Hydrol. Sci. J.*  
959 55, 364–376. <https://doi.org/10.1080/02626661003683389>
- 960 Dietrich, J.C., Bunya, S., Westerink, J.J., Ebersole, B.A., Smith, J.M., Atkinson, J.H., Jensen, R.,  
961 Resio, D.T., Luettich, R.A., Dawson, C., Cardone, V.J., Cox, A.T., Powell, M.D.,  
962 Westerink, H.J., Roberts, H.J., Dietrich, J.C., Bunya, S., Westerink, J.J., Ebersole, B.A.,  
963 Smith, J.M., Atkinson, J.H., Jensen, R., Resio, D.T., Luettich, R.A., Dawson, C., Cardone,  
964 V.J., Cox, A.T., Powell, M.D., Westerink, H.J., Roberts, H.J., 2010. A High-Resolution  
965 Coupled Riverine Flow, Tide, Wind, Wind Wave, and Storm Surge Model for Southern  
966 Louisiana and Mississippi. Part II: Synoptic Description and Analysis of Hurricanes Katrina  
967 and Rita. *Mon. Weather Rev.* 138, 378–404. <https://doi.org/10.1175/2009MWR2907.1>
- 968 Dietrich, J.C., Tanaka, S., Westerink, J.J., Dawson, C.N., Luettich, R.A., Zijlema, M.,  
969 Holthuijsen, L.H., Smith, J.M., Westerink, L.G., Westerink, H.J., 2012. Performance of the  
970 Unstructured-Mesh, SWAN+ADCIRC Model in Computing Hurricane Waves and Surge. *J.*  
971 *Sci. Comput.* 52, 468–497. <https://doi.org/10.1007/s10915-011-9555-6>
- 972 Dietrich, J.C., Zijlema, M., Westerink, J.J., Holthuijsen, L.H., Dawson, C., Luettich, R.A.,  
973 Jensen, R.E., Smith, J.M., Stelling, G.S., Stone, G.W., 2011. Modeling hurricane waves and  
974 storm surge using integrally-coupled, scalable computations. *Coast. Eng.* 58, 45–65.  
975 <https://doi.org/10.1016/J.COASTALENG.2010.08.001>
- 976 DOGAMI, 2010. Lidar Remote Sensing Data Collection: Southwest Washington.
- 977 Dushaw, B.D., Egbert, G.D., Worcester, P.F., Cornuelle, B.D., Howe, B.M., Metzger, K., 1997.  
978 A TOPEX/POSEIDON global tidal model (TPXO.2) and barotropic tidal currents  
979 determined from long-range acoustic transmissions. *Prog. Oceanogr.* 40, 337–367.  
980 [https://doi.org/10.1016/S0079-6611\(98\)00008-1](https://doi.org/10.1016/S0079-6611(98)00008-1)
- 981 Engle, V.D., Kurtz, J.C., Smith, L.M., Chancy, C., Bourgeois, P., 2007. A Classification of U.S.  
982 Estuaries Based on Physical and Hydrologic Attributes. *Environ. Monit. Assess.* 129, 397–  
983 412. <https://doi.org/10.1007/s10661-006-9372-9>
- 984 Funakoshi, Y., Hagen, S.C., Bacopoulos, P., 2008. Coupling of Hydrodynamic and Wave  
985 Models: Case Study for Hurricane Floyd (1999) Hindcast. *J. Waterw. Port, Coastal, Ocean*  
986 *Eng.* 134, 321–335. [https://doi.org/10.1061/\(ASCE\)0733-950X\(2008\)134:6\(321\)](https://doi.org/10.1061/(ASCE)0733-950X(2008)134:6(321))
- 987 Ganju, N.K., Brush, M.J., Rashleigh, B., Aretxabaleta, A.L., Del Barrio, P., Gear, J.S., Harris,  
988 L.A., Lake, S.J., Mccardell, G., O'donnell, J., Ralston, D.K., Signell, R.P., Testa, J.M.,  
989 Vaudrey, J.M.P., 2015. Progress and Challenges in Coupled Hydrodynamic-Ecological  
990 Estuarine Modeling. *Estuaries and Coasts.* <https://doi.org/10.1007/s12237-015-0011-y>

- 991 Gano, S., Kim, H., Brown, D., 2006. Comparison of Three Surrogate Modeling Techniques:  
 992 Datascape, Kriging, and Second Order Regression, in: 11th AIAA/ISSMO Multidisciplinary  
 993 Analysis and Optimization Conference. American Institute of Aeronautics and Astronautics,  
 994 Reston, Virginia. <https://doi.org/10.2514/6.2006-7048>
- 995 Gouldby, B., Méndez, F.J., Guancho, Y., Rueda, A., Mínguez, R., 2014. A methodology for  
 996 deriving extreme nearshore sea conditions for structural design and flood risk analysis.  
 997 *Coast. Eng.* 88, 15–26. <https://doi.org/10.1016/J.COASTALENG.2014.01.012>
- 998 Green, C., Viavattene, C., Thompson, P., 2011. Guidance for assessing flood losses CONHAZ  
 999 Report.
- 1000 Haigh, I.D., Wijeratne, E.M.S., MacPherson, L.R., Pattiaratchi, C.B., Mason, M.S., Crompton,  
 1001 R.P., George, S., 2014. Estimating present day extreme water level exceedance probabilities  
 1002 around the coastline of Australia: tides, extra-tropical storm surges and mean sea level.  
 1003 *Clim. Dyn.* 42, 121–138. <https://doi.org/10.1007/s00382-012-1652-1>
- 1004 Hall, J.W., Manning, L.J., Hankin, R.K.S., 2011. Bayesian calibration of a flood inundation  
 1005 model using spatial data. *Water Resour. Res.* 47, W05529.  
 1006 <https://doi.org/10.1029/2009WR008541>
- 1007 Higdon, D., Gattiker, J., Williams, B., Rightley, M., 2008. Computer Model Calibration Using  
 1008 High-Dimensional Output. *J. Am. Stat. Assoc.* 103, 570–583.  
 1009 <https://doi.org/10.1198/016214507000000888>
- 1010 Jia, G., Taflanidis, A.A., 2013. Kriging metamodeling for approximation of high-dimensional  
 1011 wave and surge responses in real-time storm/hurricane risk assessment. *Comput. Methods*  
 1012 *Appl. Mech. Eng.* 261–262, 24–38. <https://doi.org/10.1016/J.CMA.2013.03.012>
- 1013 Jia, G., Taflanidis, A.A., Nadal-Caraballo, N.C., Melby, J.A., Kennedy, A.B., Smith, J.M., 2016.  
 1014 Surrogate modeling for peak or time-dependent storm surge prediction over an extended  
 1015 coastal region using an existing database of synthetic storms. *Nat. Hazards* 81, 909–938.  
 1016 <https://doi.org/10.1007/s11069-015-2111-1>
- 1017 Jin, R., Chen, W., Simpson, T.W., 2001. Comparative studies of metamodeling techniques  
 1018 under multiple modelling criteria. *Struct. Multidiscip. Optim.* 23, 1–13.  
 1019 <https://doi.org/10.1007/s00158-001-0160-4>
- 1020 Johnson, M.E., Moore, L.M., Ylvisaker, D., 1990. Minimax and maximin distance designs. *J.*  
 1021 *Stat. Plan. Inference* 26, 131–148. [https://doi.org/10.1016/0378-3758\(90\)90122-B](https://doi.org/10.1016/0378-3758(90)90122-B)
- 1022 Jones, B., Johnson, R.T., 2009. Design and analysis for the Gaussian process model. *Qual.*  
 1023 *Reliab. Eng. Int.* 25, 515–524. <https://doi.org/10.1002/qre.1044>
- 1024 Kantha, L.H., Clayson, C.A., 2000. Numerical models of oceans and oceanic processes., Vol. 66.  
 1025 ed. Elsevier.

- 1026 Kennedy, M.C., Anderson, C.W., Conti, S., O'Hagan, A., 2006. Case studies in Gaussian process  
1027 modelling of computer codes. *Reliab. Eng. Syst. Saf.* 91, 1301–1309.  
1028 <https://doi.org/10.1016/J.RESS.2005.11.028>
- 1029 Kim, S.W., Melby, J. a., Nadal-Caraballo, N.C., Ratcliff, J., 2015. A time-dependent surrogate  
1030 model for storm surge prediction based on an artificial neural network using high-fidelity  
1031 synthetic hurricane modeling. *Nat. Hazards* 76, 565–585. <https://doi.org/10.1007/s11069-014-1508-6>  
1032
- 1033 Kohavi, R., 1995. A Study of Cross-Validation and Bootstrap for Accuracy Estimation and  
1034 Model Selection, in: *International Joint Conference on Artificial Intelligence*.
- 1035 Krien, Y., Dudon, B., Roger, J., Zahibo, N., 2015. Probabilistic hurricane-induced storm surge  
1036 hazard assessment in Guadeloupe, Lesser Antilles. *Nat. Hazards Earth Syst. Sci.* 15, 1711–  
1037 1720. <https://doi.org/10.5194/nhess-15-1711-2015>
- 1038 Lakshmi, D.D., Murty, P.L.N., Bhaskaran, P.K., Sahoo, B., Kumar, T.S., Shenoi, S.S.C.,  
1039 Srikanth, A.S., 2017. Performance of WRF-ARW winds on computed storm surge using  
1040 hydrodynamic model for Phailin and Hudhud cyclones. *Ocean Eng.* 131, 135–148.  
1041 <https://doi.org/10.1016/J.OCEANENG.2017.01.005>
- 1042 Lavery, S., Donovan, B., 2005. Flood risk management in the Thames Estuary looking ahead  
1043 100 years. *Philos. Trans. A. Math. Phys. Eng. Sci.* 363, 1455–74.  
1044 <https://doi.org/10.1098/rsta.2005.1579>
- 1045 Le Provost, C., Genco, M.L., Lyard, F., Vincent, P., Canceil, P., 1994. Spectroscopy of the world  
1046 ocean tides from a finite element hydrodynamic model. *J. Geophys. Res.* 99, 24777.  
1047 <https://doi.org/10.1029/94JC01381>
- 1048 Leonard, M., Westra, S., Phatak, A., Lambert, M., van den Hurk, B., McInnes, K., Risbey, J.,  
1049 Schuster, S., Jakob, D., Stafford-Smith, M., 2014. A compound event framework for  
1050 understanding extreme impacts. *Wiley Interdiscip. Rev. Clim. Chang.* 5, 113–128.  
1051 <https://doi.org/10.1002/wcc.252>
- 1052 Levy, S., Steinberg, D.M., 2010. Computer experiments: a review. *Adv. Stat. Anal.* 94, 311–324.  
1053 <https://doi.org/10.1007/s10182-010-0147-9>
- 1054 Lewis, M., Bates, P., Horsburgh, K., Neal, J., Schumann, G., 2013. A storm surge inundation  
1055 model of the northern Bay of Bengal using publicly available data. *Q. J. R. Meteorol. Soc.*  
1056 139, 358–369. <https://doi.org/10.1002/qj.2040>
- 1057 Lin, N., Emanuel, K.A., Smith, J.A., Vanmarcke, E., 2010. Risk assessment of hurricane storm  
1058 surge for New York City. *J. Geophys. Res.* 115, D18121.  
1059 <https://doi.org/10.1029/2009JD013630>

- 1060 Lin, N., Emanuel, K.A., Oppenheimer, M., Vanmarcke, E., 2012. Physically-based Assessment  
1061 of Hurricane Surge Threat under Climate Change. *Nat. Clim. Chang.* 2.6, 462–467.
- 1062 Liu, F., West, M., 2009. A dynamic modelling strategy for Bayesian computer model emulation.  
1063 *Bayesian Anal.* 4, 393–411. <https://doi.org/10.1214/09-BA415>
- 1064 Liu, X., Guillas, S., 2017. Dimension Reduction for Gaussian Process Emulation: An  
1065 Application to the Influence of Bathymetry on Tsunami Heights. *SIAM/ASA J. Uncertain.*  
1066 *Quantif.* 5, 787–812. <https://doi.org/10.1137/16M1090648>
- 1067 Loepky, J.L., Sacks, J., Welch, W.J., 2009. Choosing the Sample Size of a Computer  
1068 Experiment: A Practical Guide. *Technometrics* 51, 366–376.  
1069 <https://doi.org/10.1198/TECH.2009.08040>
- 1070 Love, M.R., Friday, D.Z., Grothe, P.R., Carignan, K.S., Eakins, B.W., Taylor, L.A., 2012.  
1071 Digital Elevation Model of Astoria, Oregon: Procedures, Data Sources and Analysis.
- 1072 Luettich, R A, J., Westerink, J.J., Scheffner, N.W., 1992. ADCIRC: An Advanced Three-  
1073 Dimensional Circulation Model for Shelves, Coasts, and Estuaries. Report 1. Theory and  
1074 Methodology of ADCIRC-2DDI and ADCIRC-3DL.
- 1075 Madsen, H., Jakobsen, F., 2004. Cyclone induced storm surge and flood forecasting in the  
1076 northern Bay of Bengal. *Coast. Eng.* 51, 277–296.  
1077 <https://doi.org/10.1016/J.COASTALENG.2004.03.001>
- 1078 Malde, S., Oakley, J., Wyncott, D., 2016. MUCM: Gaussian Process Emulator.
- 1079 Malde, S., Wyncoll, D., Oakley, J., Tozer, N., Gouldby, B., 2016. Applying emulators for  
1080 improved flood risk analysis. *E3S Web Conf.* 7, 04002.  
1081 <https://doi.org/10.1051/e3sconf/20160704002>
- 1082 Mass, C., Dotson, B., 2010. Major Extratropical Cyclones of the Northwest United States:  
1083 Historical Review, Climatology, and Synoptic Environment. *Mon. Weather Rev.* 138,  
1084 2499–2527. <https://doi.org/10.1175/2010MWR3213.1>
- 1085 Mastrandrea, M.D., Field, C.B., Stocker, T.F., Edenhofer, O., Ebi, K.L., Frame, D.J., Held, H.,  
1086 Kriegler, E., Mach, K.J., Matschoss, P.R., Plattner, G.-K., Yohe, G.W., Zwiers, F.W., 2010.  
1087 Guidance Note for Lead Authors of the IPCC Fifth Assessment Report on Consistent  
1088 Treatment of Uncertainties IPCC Cross-Working Group Meeting on Consistent Treatment  
1089 of Uncertainties.
- 1090 McKay, M.D., Beckman, R.J., Conover, W.J., 1979. Comparison of Three Methods for Selecting  
1091 Values of Input Variables in the Analysis of Output from a Computer Code. *Technometrics*  
1092 21, 239–245. <https://doi.org/10.1080/00401706.1979.10489755>

- 1093 Mckay, P., Blain, C.A., 2010. Toward Developing a Hydrodynamic Flow and Inundation Model  
1094 of the Lower Pearl River.
- 1095 Mesinger, F., DiMego, G., Kalnay, E., Mitchell, K., Shafran, P.C., Ebisuzaki, W., Jović, D.,  
1096 Woollen, J., Rogers, E., Berbery, E.H., Ek, M.B., Fan, Y., Grumbine, R., Higgins, W., Li,  
1097 H., Lin, Y., Manikin, G., Parrish, D., Shi, W., Mesinger, F., DiMego, G., Kalnay, E.,  
1098 Mitchell, K., Shafran, P.C., Ebisuzaki, W., Jović, D., Woollen, J., Rogers, E., Berbery, E.H.,  
1099 Ek, M.B., Fan, Y., Grumbine, R., Higgins, W., Li, H., Lin, Y., Manikin, G., Parrish, D., Shi,  
1100 W., 2006. North American Regional Reanalysis. *Bull. Am. Meteorol. Soc.* 87, 343–360.  
1101 <https://doi.org/10.1175/BAMS-87-3-343>
- 1102 Moel, H., Asselman, N., Aerts, J., 2012. Uncertainty and sensitivity analysis of coastal flood  
1103 damage estimates in the west of the Netherlands. *Nat. Hazards Earth Syst. Sci* 12, 1045–  
1104 1058. <https://doi.org/10.5194/nhess-12-1045-2012>
- 1105 Mofstakhari, H.R., AghaKouchak, A., Sanders, B.F., Matthew, R.A., 2017. Cumulative hazard:  
1106 The case of nuisance flooding. *Earth's Futur.* 5, 214–223.  
1107 <https://doi.org/10.1002/2016EF000494>
- 1108 Montoya, R.D., Osorio Arias, A., Ortiz Royero, J.C., Ocampo-Torres, F.J., 2013. A wave  
1109 parameters and directional spectrum analysis for extreme winds. *Ocean Eng.* 67, 100–118.  
1110 <https://doi.org/10.1016/J.OCEANENG.2013.04.016>
- 1111 Morris, M.D., Mitchel, T.J., 1995. Exploratory Designs for Computational Experiments. *J. Stat.*  
1112 *Plan. Inference* 43, 381–402. [https://doi.org/10.1016/0378-3758\(94\)00035-T](https://doi.org/10.1016/0378-3758(94)00035-T)
- 1113 NOAA National Centers for Environmental Information, 2003. Coastal Relief Model.
- 1114 O'Hagan, A., 2006. Bayesian analysis of computer code outputs: A tutorial. *Reliab. Eng. Syst.*  
1115 *Saf.* 91, 1290–1300. <https://doi.org/10.1016/j.ress.2005.11.025>
- 1116 Oakley, J., 1999. Bayesian uncertainty analysis for complex computer codes. Thesis.
- 1117 Orton, P.M., Hall, T.M., Talke, S.A., Blumberg, A.F., Georgas, N., Vinogradov, S., 2016. A  
1118 validated tropical-extratropical flood hazard assessment for New York Harbor. *J. Geophys.*  
1119 *Res. Ocean.* 121, 8904–8929. <https://doi.org/10.1002/2016JC011679>
- 1120 Pendleton, L.H., 2010. The economic and market value of coasts and estuaries: what's at stake?  
1121 *Restore America's Estuaries*, Arlington.
- 1122 Perez, J., Menendez, M., Losada, I.J., 2017. GOW2: A global wave hindcast for coastal  
1123 applications. *Coast. Eng.* 124, 1–11. <https://doi.org/10.1016/J.COASTALENG.2017.03.005>
- 1124 Pugh, D.T., 1996. *Tides, Surges and Mean Sea-Level*. John Wiley & Sons.

- 1125 Purvis, M.J., Bates, P.D., Hayes, C.M., 2008. A probabilistic methodology to estimate future  
 1126 coastal flood risk due to sea level rise. *Coast. Eng.* 55, 1062–1073.  
 1127 <https://doi.org/10.1016/j.coastaleng.2008.04.008>
- 1128 Rasmussen, C.E., Williams, C.K.I., 2006. *Gaussian processes for machine learning*. Cambridge:  
 1129 MIT press.
- 1130 Razavi, S., Tolson, B. a., Burn, D.H., 2012. Review of surrogate modeling in water resources.  
 1131 *Water Resour. Res.* 48. <https://doi.org/10.1029/2011WR011527>
- 1132 Reichert, P., White, G., Bayarri, M.J., Pitman, E.B., 2011. Mechanism-based emulation of  
 1133 dynamic simulation models: Concept and application in hydrology. *Comput. Stat. Data*  
 1134 *Anal.* 55, 1638–1655. <https://doi.org/10.1016/j.csda.2010.10.011>
- 1135 Resio, D.T., Irish, J., Cialone, M., 2009. A surge response function approach to coastal hazard  
 1136 assessment – part 1: basic concepts. *Nat. Hazards* 51, 163–182.  
 1137 <https://doi.org/10.1007/s11069-009-9379-y>
- 1138 Resio, D.T., Westerink, J.J., 2008. Modeling the Physics of Storm Surges. *Phys. Today* 61.  
 1139 <https://doi.org/10.1063/1.2982120>
- 1140 Roberts, S., Osborne, M., Ebden, M., Reece, S., Gibson, N., Aigrain, S., 2013. Gaussian  
 1141 processes for time-series modelling. *Philos. Trans. A. Math. Phys. Eng. Sci.* 371, 20110550.  
 1142 <https://doi.org/10.1098/rsta.2011.0550>
- 1143 Rogers, W.E., Kaihatu, J.M., Hsu, L., Jensen, R.E., Dykes, J.D., Holland, K.T., 2007.  
 1144 Forecasting and hindcasting waves with the SWAN model in the Southern California Bight.  
 1145 *Coast. Eng.* 54, 1–15. <https://doi.org/10.1016/J.COASTALENG.2006.06.011>
- 1146 Rohmer, J., Idier, D., 2012. A meta-modelling strategy to identify the critical offshore conditions  
 1147 for coastal flooding. *Nat. Hazards Earth Syst. Sci.* 12, 2943–2955.  
 1148 <https://doi.org/10.5194/nhess-12-2943-2012>
- 1149 Rueda, A., Gouldby, B., Mendez, F.J., Tomas, A., Losada, I.J., Lara, J.L., Diaz-Simal, P., 2016.  
 1150 The use of wave propagation and reduced complexity inundation models and metamodels  
 1151 for coastal flood risk assessment. *J. Flood Risk Manag.* 9, 390–401.  
 1152 <https://doi.org/10.1111/jfr3.12204>
- 1153 Rusu, L., Pilar, P., 2008. Hindcast of the wave conditions along the west Iberian coast. *Coast.*  
 1154 *Eng.* 55, 906–919. <https://doi.org/10.1016/J.COASTALENG.2008.02.029>
- 1155 Sacks, J., Schiller, S.B., Welch, W.J., 1989. Designs for Computer Experiments. *Technometrics*  
 1156 31, 41–47. <https://doi.org/10.1080/00401706.1989.10488474>



- 1157 Schulz, E., Speekenbrink, M., Krause, A., 2018. A tutorial on Gaussian process regression:  
1158 Modelling, exploring, and exploiting functions. *J. Math. Psychol.* 85, 1–16.  
1159 <https://doi.org/10.1016/J.JMP.2018.03.001>
- 1160 Serafin, K.A., Ruggiero, P., 2014. Simulating extreme total water levels using a time-dependent,  
1161 extreme value approach. *J. Geophys. Res. Ocean.* 119, 6305–6329.  
1162 <https://doi.org/10.1002/2014JC010093>
- 1163 Serafin, K.A., Ruggiero, P., Stockdon, H.F., 2017. The relative contribution of waves, tides, and  
1164 non-tidal residuals to extreme total water levels on US West Coast sandy beaches. *Geophys.*  
1165 *Res. Lett.* 44, 1839–1847. <https://doi.org/10.1002/2016GL071020>
- 1166 Song, Y.K., Irish, J.L., Udoh, I.E., 2012. Regional attributes of hurricane surge response  
1167 functions for hazard assessment. *Nat. Hazards* 64, 1475–1490.  
1168 <https://doi.org/10.1007/s11069-012-0309-z>
- 1169 Svensson, C., Jones, D.A., 2004. Dependence between sea surge, river flow and precipitation in  
1170 south and west Britain. *Hydrol. Earth Syst. Sci.* 8, 973–992. [https://doi.org/10.5194/hess-8-](https://doi.org/10.5194/hess-8-973-2004)  
1171 [973-2004](https://doi.org/10.5194/hess-8-973-2004)
- 1172 Taylor, N.R., Irish, J.L., Udoh, I.E., Bilskie, M. V., Hagen, S.C., 2015. Development and  
1173 uncertainty quantification of hurricane surge response functions for hazard assessment in  
1174 coastal bays. *Nat. Hazards* 77, 1103–1123. <https://doi.org/10.1007/s11069-015-1646-5>
- 1175 Timmermans, B., 2015. Uncertainty in Numerical Wind-wave Models. University of  
1176 Southampton.
- 1177 Tolman, H.L., 2009. User manual and system documentation of WAVEWATCH III TM version  
1178 3.14 †.
- 1179 Wahl, T., Jain, S., Bender, J., Meyers, S.D., Luther, M.E., 2015. Increasing risk of compound  
1180 flooding from storm surge and rainfall for major US cities. *Nat. Clim. Chang.* 5, 1093–  
1181 1097. <https://doi.org/10.1038/nclimate2736>
- 1182 Weaver, R.J., Luettich, Jr., R.A., 2010. 2D vs. 3D Storm Surge Sensitivity in ADCIRC: Case  
1183 Study of Hurricane Isabel, in: *Estuarine and Coastal Modeling (2009)*. American Society of  
1184 Civil Engineers, Reston, VA, pp. 762–779. [https://doi.org/10.1061/41121\(388\)44](https://doi.org/10.1061/41121(388)44)
- 1185 Weaver, R.J., Slinn, D.N., 2010. Influence of bathymetric fluctuations on coastal storm surge.  
1186 *Coast. Eng.* 57, 62–70. <https://doi.org/10.1016/J.COASTALENG.2009.09.012>
- 1187 Westerink, J.J., Luettich, R.A., Baptists, A.M., Scheffner, N.W., Farrar, P., 1992. Tide and Storm  
1188 Surge Predictions Using Finite Element Model. *J. Hydraul. Eng.* 118, 1373–1390.  
1189 [https://doi.org/10.1061/\(ASCE\)0733-9429\(1992\)118:10\(1373\)](https://doi.org/10.1061/(ASCE)0733-9429(1992)118:10(1373))

- 1190 Zhang, K., Douglas, B.C., Leatherman, S.P., 1999. Twentieth-Century Storm Activity along the  
1191 U.S. East Coast. *J. Clim.* 13.
- 1192 Zijlema, M., 2010. Computation of wind-wave spectra in coastal waters with SWAN on  
1193 unstructured grids. *Coast. Eng.* 57, 267–277.  
1194 <https://doi.org/10.1016/J.COASTALENG.2009.10.011>
- 1195 Zscheischler, J., Westra, S., van den Hurk, B.J.J.M., Seneviratne, S.I., Ward, P.J., Pitman, A.,  
1196 AghaKouchak, A., Bresch, D.N., Leonard, M., Wahl, T., Zhang, X., 2018. Future climate  
1197 risk from compound events. *Nat. Clim. Chang.* 8, 469–477. [https://doi.org/10.1038/s41558-](https://doi.org/10.1038/s41558-018-0156-3)  
1198 [018-0156-3](https://doi.org/10.1038/s41558-018-0156-3)